DA-FR-118

DA-FR-118 is located in a meadow and forested areas with undulating to rolling slopes and contains agricultural land, existing dirt road and gravel road. No new impervious area is proposed within DA-FR-118. Multiple points of analysis were evaluated within DA-FR-118 to evaluate the effects on each receiving stream/channel following construction. Specifically, DA-FR-118 was divided into five sub-drainage areas (sub-areas A to E).

Pre-construction agricultural areas will be returned to agricultural land use (i.e., returned to crop production, in identical condition) following construction. In non-agricultural areas, land use will be restored following construction as noted in the Stormwater Management (SWM) Narrative and the Annual Standards and Specifications. Agricultural areas within the limits of disturbance (LOD) are included in the SWM quality analysis and the total permanent right-of-way is analyzed via VRRM; in these calculations agricultural areas are considered "Forest/Open Space". The total phosphorus load reduction required for DA-FR-113 is -0.71 lbs.

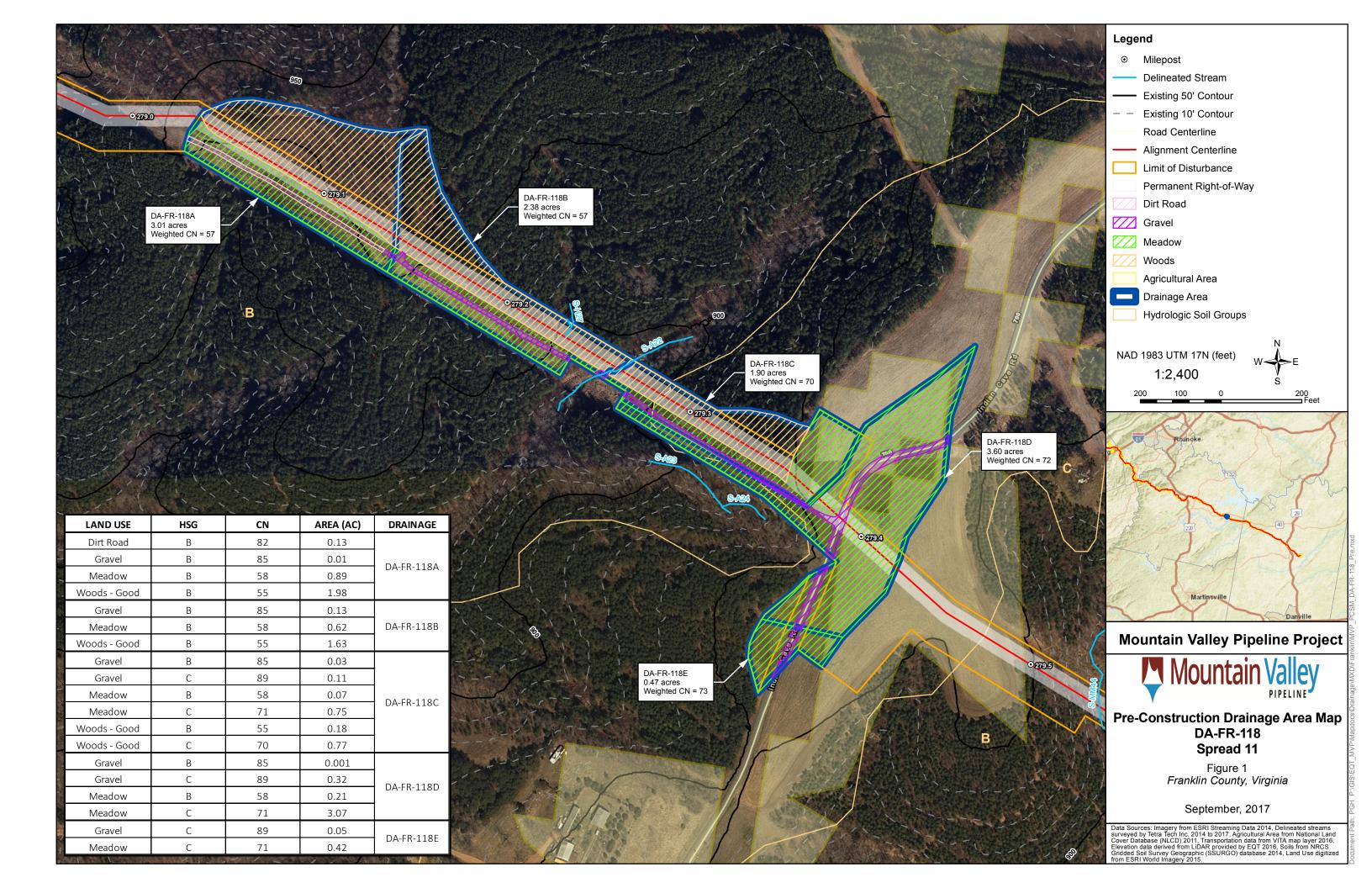
Stormwater quantity is met via the energy balance method sub-areas DA-FR-118D and DA-FR-118E. In addition to the energy balance method, storm water quantity in sub-areas DA-FR-118A, DA-FR-118B and DA-FR-118C is met by seven 2 ft by 30 ft, nine 2 ft by 30 ft and nine 2 ft by 50 ft compost amended water bar areas respectively (See General Detail MVP-ES38 for a full schedule). Sub-areas DA-FR-118A, DA-FR-118B and DA-FR-118C contain entirely non-agricultural areas within the LOD, therefore an Improvement Factor of 0.8 is used when applying the Energy Balance Method per 9VAC25-870-66.B.3.a. Sub-area DA-FR-118D contains both agricultural and non-agricultural areas within the LOD. For SWM quantity, agricultural areas within the study area are considered/included but an Improvement Factor of 1.0 is used when applying the Energy Balance Method to account for the exemption of agricultural areas (§ 62.1-44.15:34 and 9VAC25-870-300) since such areas will be returned to agricultural land use (i.e., returned to crop production, in identical condition) following construction. In addition, sub-area DA-FR-118E within the LOD will be restored to a land use condition that is equivalent to the existing land use condition following construction. For these reasons, an Improvement Factor of 1.0 was assumed when completing the energy balance calculations.

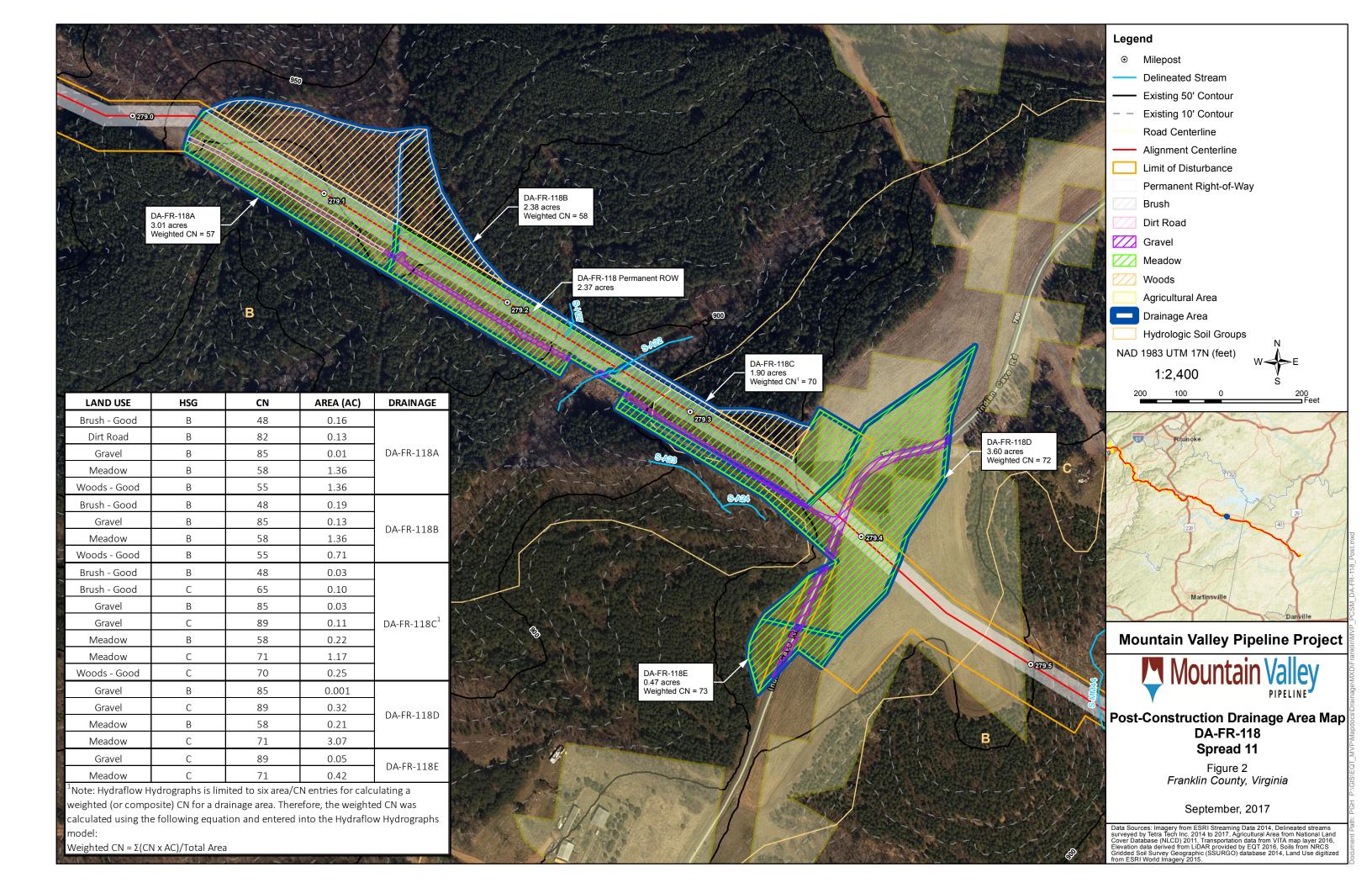
The Hydraflow 10-year 24-hour peak discharge results indicate a reduction in flows ranging from 0 to 0.07 cfs for sub-drainage areas DA-FR-118 A, DA-FR-118C, DA-FR-118D and DA-FR-118E. The Hydraflow 10-year 24-hour peak discharge increased by 0.23 cfs flow for DA-FR-118B (as seen in the following table). The increase in 10-yr peak flow in sub-area DA-FR-118B will have little/no impact on flood stage in the

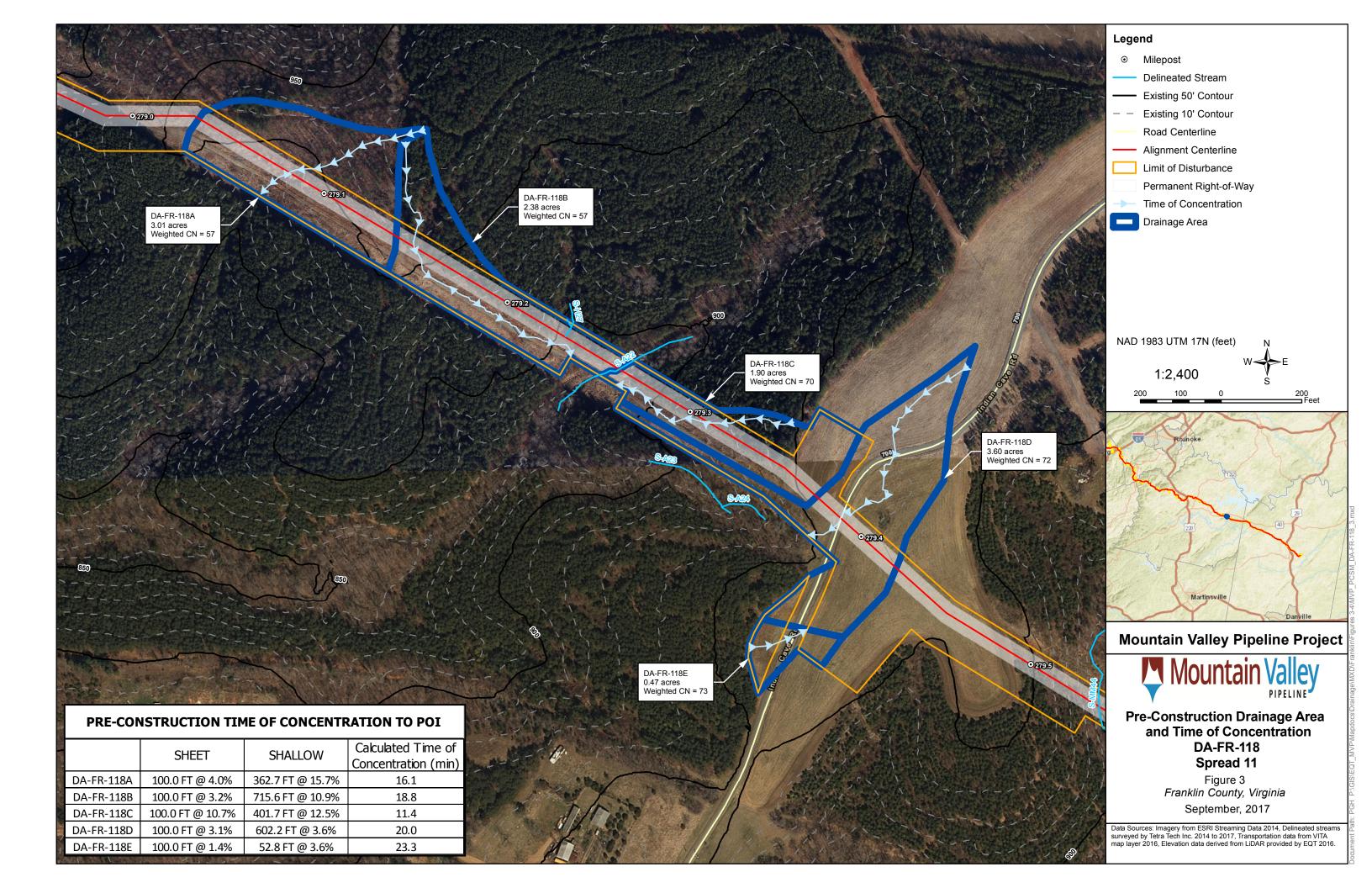
downstream channel. Furthermore, there are no nearby property owners. For these reasons, no further analysis is required.

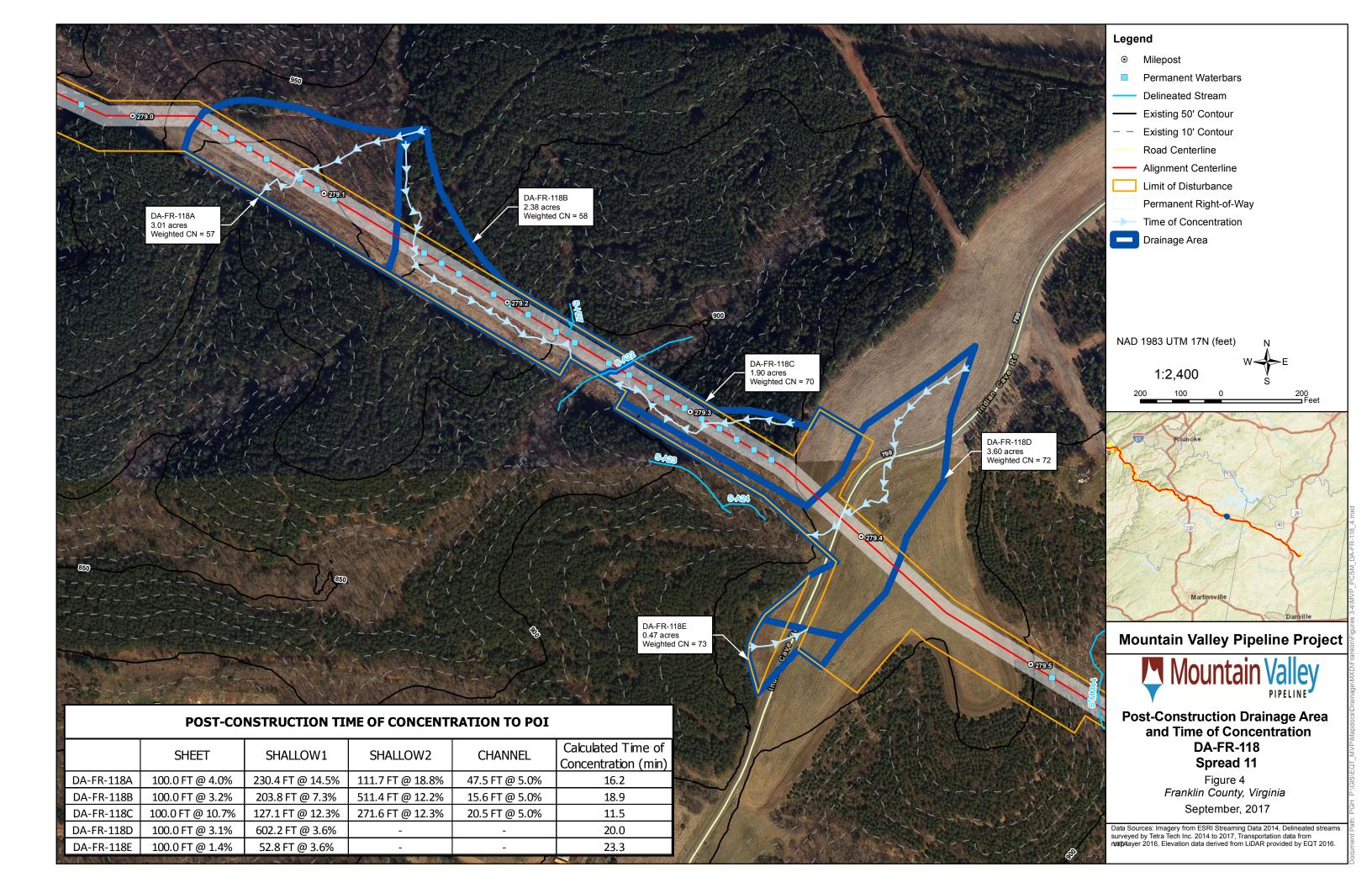
Sub-Area	Pre Peak Flow, 10-yr Q (cfs)	Post Peak Flow, 10-yr Q (cfs)	Flow Differential
DA-FR-118A	5.33	5.30	-0.03
DA-FR-118B	3.75	3.98	0.23
DA-FR-118C	7.44	7.37	-0.07
DA-FR-118D	11.46	11.46	0.00
DA-FR-118E	1.41	1.41	0.00

Figures and calculations for each of the sub-areas for DA-FR-118 follow. See Appendix D of the Annual Standards and Specifications for further detail on stormwater methodology.









DEQ Virginia Runoff Reduction Method Re-Development Compliance Spreadsheet - Version 3.0

BMP Design Specifications List: 2013 Draft Stds & Specs

Site Summary - Linear Development Project***

Total Rainfall (in):	43
Total Disturbed Acreage:	2.37

Site Land Cover Summary

Pre-ReDevelopment Land Cover (acres)

	A soils	B Soils	C Soils	D Soils	Totals	% of Total
Forest/Open (acres)	0.00	1.54	0.79	0.00	2.33	98
Managed Turf (acres)	0.00	0.00	0.00	0.00	0.00	0
Impervious Cover (acres)	0.00	0.00	0.04	0.00	0.04	2
					2.37	100

Post-ReDevelopment Land Cover (acres)

	A soils	B Soils	C Soils	D Soils	Totals	% of Total
Forest/Open (acres)	0.00	1.54	0.79	0.00	2.33	98
Managed Turf (acres)	0.00	0.00	0.00	0.00	0.00	0
Impervious Cover (acres)	0.00	0.00	0.04	0.00	0.04	2
* Forest/Open Space areas must be protected in	2 37	100				

Site Tv and Land Cover Nutrient Loads

	Final Post-Development (Post-ReDevelopment & New Impervious)	Post- ReDevelopment	Post- Development (New Impervious)	Adjusted Pre- ReDevelopment
Site Rv	0.05	0.05		0.05
Treatment Volume (ft³)	420	420		420
TP Load (lb/yr)	0.26	0.26		0.26

Baseline TP Load (lb/yr): 0.9717* *Reduction below new development load limitation not required

Total TP Load Reduction Required (lb/yr)	-0.71	N/A***	N/A***
--	-------	--------	--------

***This is a linear development project

	Final Post-Development Load (Post-ReDevelopment & New Impervious)	Pre- ReDevelopment
TN Load (lb/yr)	1.89	1.89

PreReDevelopment
TP Load per acre
(lb/acre/yr)

0.11

Final Post-Development
TP Load per acre
(lb/acre/yr)

Post-ReDevelopment TP
Load per acre
(lb/acre/yr)

(lb/acre/yr)

0.11

0.11

Site Compliance Summary - ***Linear Development Project

Maximum % Reduction Required Below	20%
Pre-ReDevelopment Load	20%

* Note: % Reduction will reduce post-development TP load to less than or equal to baseline load of 0.97 lb/yr (0.41 lb/ac/yr)
[Post-Dev Reduction Requirement = Post-Dev TP load - baseline load of 0.97 lb/yr], baseline load = site area x 0.41 lb/ac/yr

0	Total Runoff Volume Reduction (ft ³)			
0.00	Total TP Load Reduction Achieved (lb/yr)			
0.00	Total TN Load Reduction Achieved (lb/yr)			
0.26	Remaining Post Development TP Load (lb/yr)			
0.00	Remaining TP Load Reduction (lb/yr) Required			
*0-4				

** TARGET TP REDUCTION EXCEEDED BY 0.71 LB/YEAR **

^{*}Reduction below new development load limitation not required



STORAGE VOLUME OF WATERBAR WITH SOIL COMPOST AMENDMENT AREA

Equations Used:
¹Vgravel storage = L*W*D_{gravel}*(40/100)

²Vsoil storage = L*W*D_{soil}*(20/100)

 3 V surface storage = [W*S*D^2]+[L*S*D^2]+[W*L*D]+[((2*S*D)^2*D)/3]

 $^{1}\text{Equation \#2b under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, assuming that gravel is made up of 40\% voids.}$

²Equation #2b under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, assuming that soil compost amendment is made up of 20% voids.

141.3333333

³Equation #1 under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, but calculation also takes into account surface side slopes.

Inputs:	Depti	h of Gravel Layer, D _{gravel} (ft) =	0	
mpaco.		Amendment Area, D _{soil} (ft) =	1	Refer to Table 4.3 in VA DEQ Stormwater Design Specification No. 4; Note that compost amendment may not be necessary for HSG A/B soils
	Length of Waterbar S	ioil Amendment Area, L (ft) =	30	Assume max. length of 50' for waterbar soil amendment areas (i.e., limited to permanent ROW)
	Width of Waterbar So	il Amendment Area, W (ft) =	2	
	Inside Embar	kment Side Slopes, S (H:V) =	2	Assume 2H:1V surface side slopes for waterbars
	Number of Perm. Wa	terbars in Drainage Area, n =	7	
	Design	Infiltration Rate, IR (in/hr) =	0.2	Min. rate of 0.30 in/hr for HSG A soils and 0.15-0.30 in/hr for HSG B soils (see Chap. 4, p. 4-30 in VA Stormwater Management Handbook Volume II (First Edition, 1999)
	Su	rface Ponding Depth, D (ft) =	0.5	Assume 0.5' CFS height at the end of waterbars
Calandadanan	T-4-14	Channel Daniel DAAD (fe)	1.5	
Calculations:		Total Storage Depth per BMP (ft) =		
Surface Storage Volume per BMP (cf) = Subsurface Storage Volume per BMP (cf) = Total Storage Volume per BMP (cf) = Total BMP Storage Volume in Drainage Area (cf) =		46.6666667		
		12		
		58.6666667		
		Total BMP Storage Volume in Drainage Area (cf) =		
	Calculated Infilt	ration Period per BMP (hr) =	58.6666667	
		Depth-Storage Da		
Depth (ft)	Width (ft)	Length (ft)	Storage Volume per BMP (cf)	Storage Volume in Drainage Area (cf)
0	2	30	0	0
0.5	2	30	6	42
1	2	30	12	84
1.5	4	32	58 6666667	410.6666667

989.3333333

ENERGY BALANCE METHOD

Inputs:

	1-Yr Event		
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)	
Pre-Developed Condition	0.708	3801	
Developed Condition	0.568	3391	
Pre-Developed (Forest) Condition	0.463	3110	

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.8

<u>Calculations:</u>	Check #1:	$Q_{\text{developed}} \leq \text{IF x } \left\{ \left(Q_{\text{gre-developed}} \times \text{RV}_{\text{pre-developed}} \right) / \text{RV}_{\text{developed}} \right>$	Q (cfs) 0.568	≤ OK	Q (cfs) 0.635
	Check #2:	$Q_{developed} \le Q_{pre-developed}$ >	0.568	≤ OK	0.708
	Check #3:	$Q_{\text{developed}} \frac{\text{shall not}}{\text{otherwise}}$ be required to be $\leq (Q_{\text{forest}} \times RV_{\text{forest}}) / RV_{\text{developed}}$ >	0.568	shall not be required to be ≤	0.425

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n								
Grass:									
Average Grass Cover	0.40								
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40								
Light Turf	0.20								
Dense Turf	0.17 – 0.80								
Dense Grass	0.17 – 0.30								
Bermuda Grass	0.30 – 0.48								
Dense Shrubbery and Forest Litter	0.40								
Natural:									
Short Grass Prairie	0.10 – 0.20								
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40								
Sparse Vegetation	0.05 – 0.13								
Oak Grasslands, Open Grasslands	0.60								
Dense Cover of Trees and Bushes	0.80								
Rangeland:									
Typical	0.13								
No Debris Cover	0.09 – 0.34								
20% Debris Cover	0.05 – 0.25								
Woods:									
Light Underbrush	0.40								
Dense Underbrush	0.80								
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40								

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 - Manning's n Values for Open Channel Flow

nan	nel T	уре	Manning n					
			Min.	Normal	Max.			
1.	Exc	cavated or Dredged Channels ¹						
	a.	Earth, Straight, and Uniform:						
		Clean, recently completed	0.016	0.018	0.020			
		Clean, after weathering	0.018	0.022	0.02			
		Gravel, uniform section, clean	0.022	0.025	0.030			
		With short grass, few weeds	0.022	0.027	0.03			
	b.	Earth Winding and Sluggish:						
		No vegetation	0.023	0.025	0.030			
		Grass, some weeds	0.025	0.030	0.03			
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040			
		Earth bottom and rubble sides	0.028	0.030	0.03			
		Stony bottom and weedy banks	0.025	0.035	0.040			
		Cobble bottom and clean sides	0.030	0.040	0.050			
	c.	Dragline-Excavated or Dredged:						
		No vegetation	0.025	0.028	0.03			
		Light brush on banks	0.035	0.050	0.060			
	d.	Rock Cuts:						
		Smooth and uniform	0.025	0.035	0.040			
		Jagged and irregular	0.035	0.040	0.050			
	e.	Channels not Maintained, Weeds and Brush Uncut:						
		Dense weeds, high as flow depth	0.050	0.080	0.120			
		Clean bottom, brush on sides	0.040	0.050	0.080			
		Same as above, highest stage of flow	0.045	0.070	0.110			
		Dense brush, high stage	0.080	0.100	0.140			
2.	Mai	n Channels²						
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.03			
	b.	Same as above, but more stones and weeds	0.030	0.035	0.040			
	C.	Clean, winding, some pools and shoals	0.033	0.040	0.04			
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050			
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.05			
	f.	Same as (d) with more stones	0.045	0.050	0.060			
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080			
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150			

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

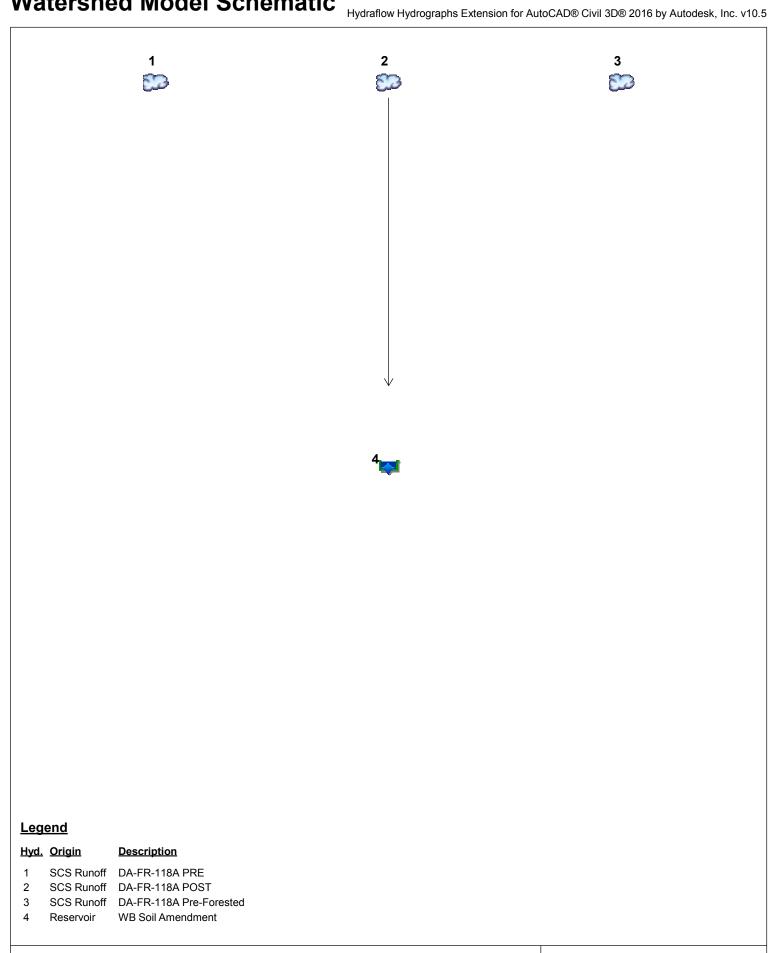
²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5 $\,$

Wednesday, 08 / 16 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year Summary Report	4 5 6 7 8 9 10
2 - Year Summary Report	13 13 14 15
10 - Year Summary Report	18 18 19 20
IDE Panart	າາ

Project: DA-FR-118a.gpw



Wednesday, 08 / 16 / 2017

Hydrograph Return Period Recap Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	Hydrograph	Inflow				Hydrograph					
lo.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		0.708	1.259			5.326				DA-FR-118A PRE
2	SCS Runoff		0.708	1.259			5.326				DA-FR-118A POST
3	SCS Runoff		0.463	0.920			4.652				DA-FR-118A Pre-Forested
4	Reservoir	2	0.568	1.215			5.302				WB Soil Amendment

Proj. file: DA-FR-118a.gpw

Wednesday, 08 / 16 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

		-			Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5					
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	0.708	1	727	3,801				DA-FR-118A PRE	
2	SCS Runoff	0.708	1	727	3,801				DA-FR-118A POST	
3	SCS Runoff	0.463	1	727	3,110				DA-FR-118A Pre-Forested	
4	Reservoir	0.568	1	733	3,391	2	935.55	471	WB Soil Amendment	
DA:	-FR-118a.gpv	\v			Return F	Period: 1 Ye	ear	Wednesday	y, 08 / 16 / 2017	

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

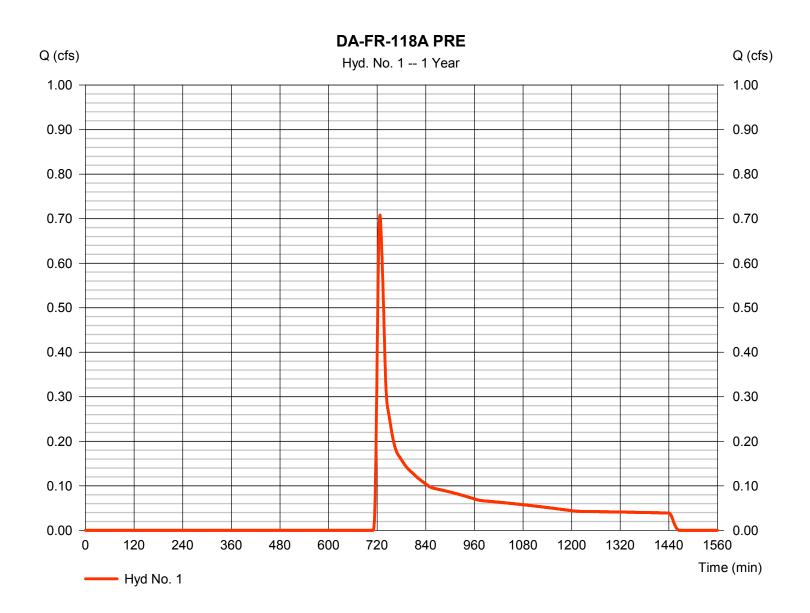
Wednesday, 08 / 16 / 2017

Hyd. No. 1

DA-FR-118A PRE

Hydrograph type = SCS Runoff Peak discharge = 0.708 cfsStorm frequency Time to peak = 727 min = 1 yrsTime interval = 1 min Hyd. volume = 3.801 cuft Curve number Drainage area = 3.010 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 16.10 \, \text{min}$ Total precip. Distribution = Type II = 3.30 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = $[(0.135 \times 82) + (0.006 \times 85) + (0.888 \times 58) + (1.984 \times 55)] / 3.010$



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1DA-FR-118A PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 4.00	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	=	15.13
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 362.70 = 15.70 = Unpaved =6.39	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.95	+	0.00	+	0.00	=	0.95
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.040 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							16.10 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

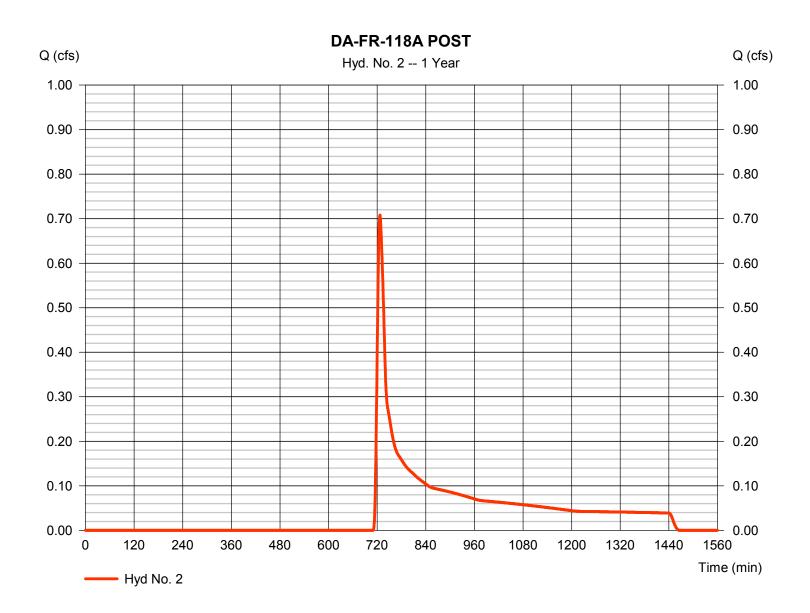
Wednesday, 08 / 16 / 2017

Hyd. No. 2

DA-FR-118A POST

Hydrograph type = SCS Runoff Peak discharge = 0.708 cfsStorm frequency Time to peak = 727 min = 1 yrsTime interval = 1 min Hyd. volume = 3.801 cuft Curve number Drainage area = 3.010 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 16.20 \, \text{min}$ Total precip. Distribution = Type II = 3.30 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.156 x 48) + (0.135 x 82) + (0.006 x 85) + (1.357 x 58) + (1.359 x 55)] / 3.010



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2DA-FR-118A POST

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>				
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 4.00	+	0.800 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	15.13				
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 230.40 = 14.50 = Unpaved =6.14	d	111.70 18.80 Unpave 7.00	d	0.00 0.00 Paved 0.00						
Travel Time (min)	= 0.63	+	0.27	+	0.00	=	0.89				
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015						
Flow length (ft)	({0})47.5		0.0		0.0						
Travel Time (min)	= 0.16	+	0.00	+	0.00	=	0.16				
Total Travel Time, Tc											

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

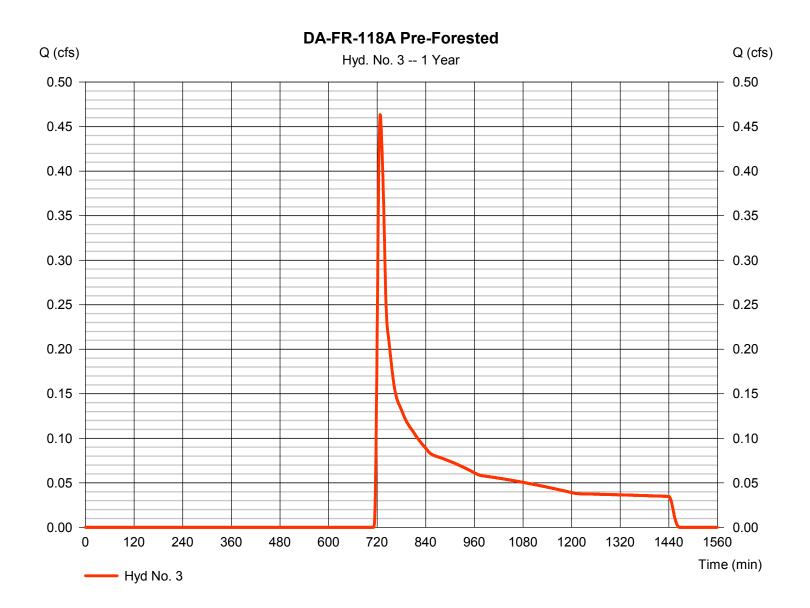
Wednesday, 08 / 16 / 2017

Hyd. No. 3

DA-FR-118A Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 0.463 cfsStorm frequency Time to peak = 727 min = 1 yrsTime interval = 1 min Hyd. volume = 3,110 cuftDrainage area Curve number = 3.010 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 = 16.10 min Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(3.010 x 55)] / 3.010



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3DA-FR-118A Pre-Forested

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 4.00 = 15.13	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	15.13
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 362.70 = 15.70 = Unpaved =6.39	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.95	+	0.00	+	0.00	=	0.95
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.030 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

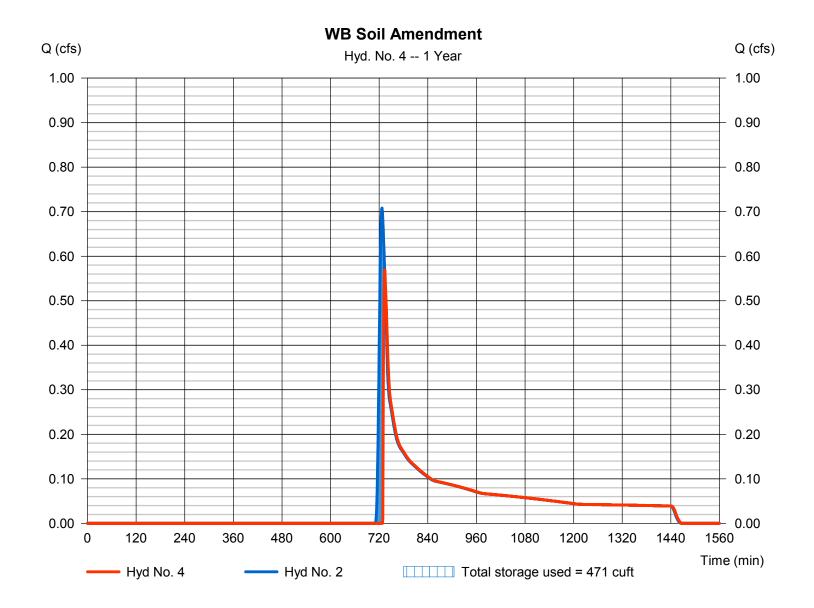
Wednesday, 08 / 16 / 2017

Hyd. No. 4

WB Soil Amendment

Hydrograph type Peak discharge = 0.568 cfs= Reservoir Storm frequency Time to peak = 733 min = 1 yrsTime interval = 1 min Hyd. volume = 3,391 cuftInflow hyd. No. Max. Elevation = 2 - DA-FR-118A POST $= 935.55 \, \text{ft}$ Reservoir name = Waterbar Soil Amendment Max. Storage = 471 cuft

Storage Indication method used.



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 16 / 2017

Pond No. 1 - Waterbar Soil Amendment

Pond Data

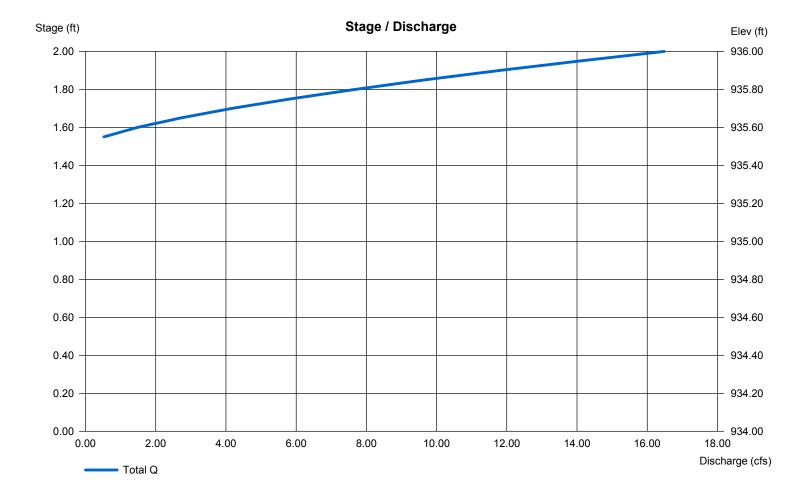
Pond storage is based on user-defined values.

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	934.00	n/a	0	0
0.50	934.50	n/a	42	42
1.00	935.00	n/a	42	84
1.50	935.50	n/a	327	411
2.00	936.00	n/a	579	989

Culvert / Orifice Structures Weir Structures [A] [B] [C] [D] [A] [B] [C] [PrfRsr] Rise (in) = 0.000.00 0.00 0.00 Crest Len (ft) = 14.00 0.00 0.00 0.00 Span (in) = 0.00 0.00 0.00 0.00 Crest El. (ft) = 935.50 0.00 0.00 0.00 No. Barrels = 0 0 Weir Coeff. = 3.333.33 3.33 3.33 = 0.000.00 0.00 0.00 Weir Type = Rect Invert El. (ft) = 0.000.00 0.00 0.00 Multi-Stage Length (ft) = No No No No Slope (%) = 0.000.00 0.00 n/a N-Value = .013 .013 .013 n/a 0.60 0.60 = 0.600.60 Exfil.(in/hr) = 0.000 (by Wet area) Orifice Coeff. Multi-Stage = n/aNo No No TW Elev. (ft) = 0.00

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	Hydrograph	Peak	Time	Time to	Hyd.	Inflow	Maximum	Total	Hydrograph
).	type (origin)	flow (cfs)	interval (min)	Peak (min)	volume (cuft)	hyd(s)	elevation (ft)	strge used (cuft)	Description
1	SCS Runoff	1.259	1	726	5,456				DA-FR-118A PRE
2	SCS Runoff	1.259	1	726	5,456				DA-FR-118A POST
3	SCS Runoff	0.920	1	726	4,598				DA-FR-118A Pre-Forested
4	Reservoir	1.215	1	728	5,045	2	935.59	511	WB Soil Amendment
)A-	FR-118a.gp\	N			Return	Period: 2 Y	ear	Wednesda	ıy, 08 / 16 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

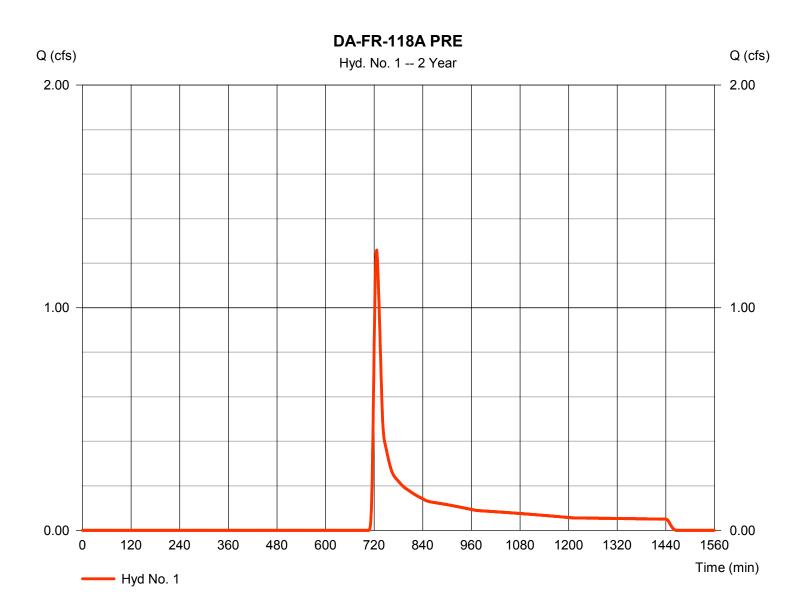
Wednesday, 08 / 16 / 2017

Hyd. No. 1

DA-FR-118A PRE

Hydrograph type = SCS Runoff Peak discharge = 1.259 cfsStorm frequency = 2 yrsTime to peak = 726 min Time interval = 1 min Hyd. volume = 5,456 cuftCurve number Drainage area = 3.010 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 16.10 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = $[(0.135 \times 82) + (0.006 \times 85) + (0.888 \times 58) + (1.984 \times 55)] / 3.010$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

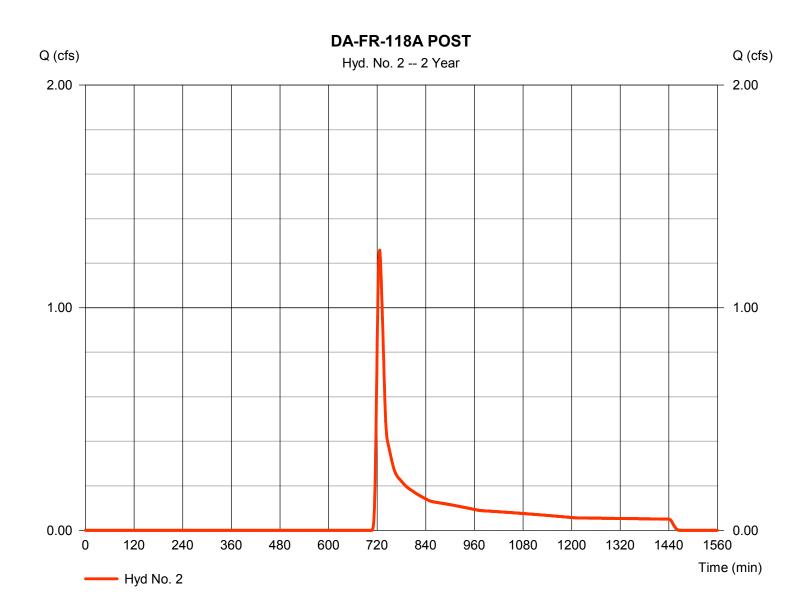
Wednesday, 08 / 16 / 2017

Hyd. No. 2

DA-FR-118A POST

Hydrograph type = SCS Runoff Peak discharge = 1.259 cfsStorm frequency = 2 yrsTime to peak = 726 min Time interval = 1 min Hyd. volume = 5,456 cuftCurve number Drainage area = 3.010 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 16.20 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = [(0.156 x 48) + (0.135 x 82) + (0.006 x 85) + (1.357 x 58) + (1.359 x 55)] / 3.010



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

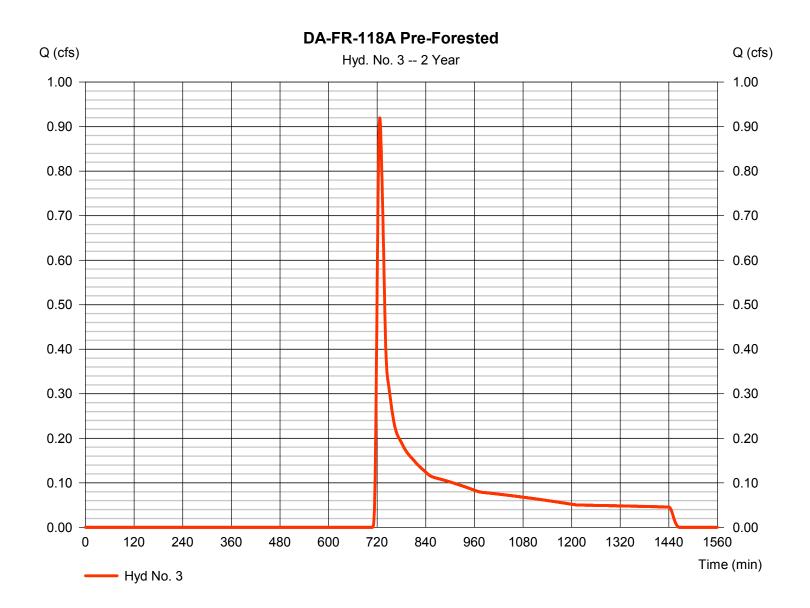
Wednesday, 08 / 16 / 2017

Hyd. No. 3

DA-FR-118A Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 0.920 cfsStorm frequency = 2 yrsTime to peak = 726 min Time interval = 1 min Hyd. volume = 4,598 cuftCurve number Drainage area = 3.010 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 16.10 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = [(3.010 x 55)] / 3.010



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

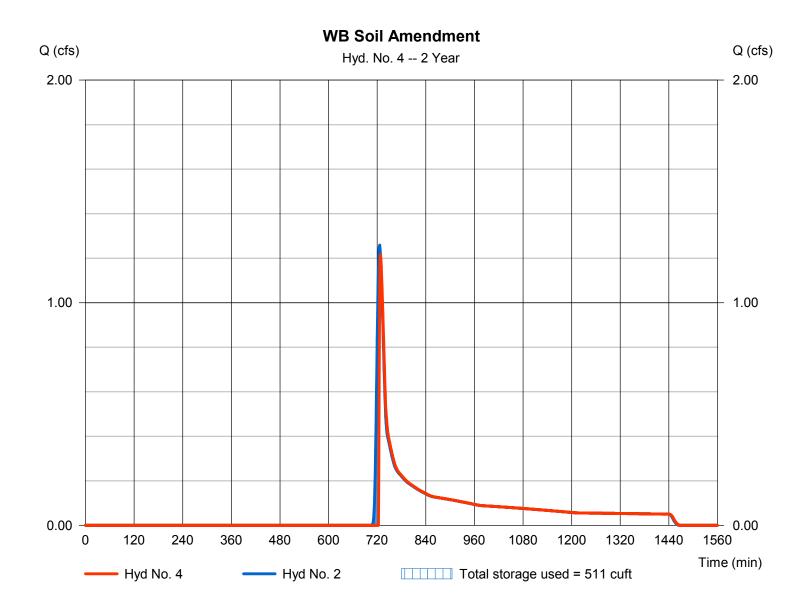
Wednesday, 08 / 16 / 2017

Hyd. No. 4

WB Soil Amendment

Hydrograph type Peak discharge = 1.215 cfs= Reservoir Storm frequency = 2 yrs Time to peak = 728 min Time interval = 1 min Hyd. volume = 5,045 cuftInflow hyd. No. = 2 - DA-FR-118A POST Max. Elevation $= 935.59 \, \text{ft}$ Reservoir name = Waterbar Soil Amendment Max. Storage = 511 cuft

Storage Indication method used.



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	1						, , ,	1	De Civil 3De 2016 by Autodesk, Inc. V10.5
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	5.326	1	724	16,560				DA-FR-118A PRE
2	SCS Runoff	5.326	1	724	16,560				DA-FR-118A POST
3	SCS Runoff	4.652	1	724	14,918				DA-FR-118A Pre-Forested
4	Reservoir	5.302	1	725	16,149	2	935.73	682	WB Soil Amendment
DA-FR-118a.gpw Return Period: 10 Year Wednesday, 08 / 16 / 2017						y, 08 / 16 / 2017			
DA-FR-118a.gpw					Return P	eriod: 10 Y	ear	Wednesday	y, 08 / 16 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

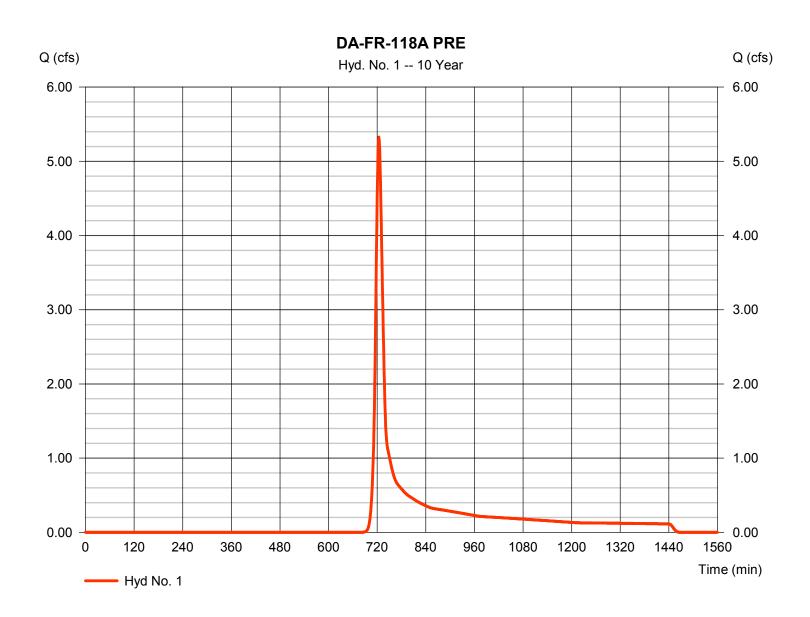
Wednesday, 08 / 16 / 2017

Hyd. No. 1

DA-FR-118A PRE

Hydrograph type = SCS Runoff Peak discharge = 5.326 cfsStorm frequency = 10 yrsTime to peak = 724 min Time interval = 1 min Hyd. volume = 16.560 cuft Curve number Drainage area = 3.010 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 16.10 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inStorm duration = 484 = 24 hrs Shape factor

^{*} Composite (Area/CN) = $[(0.135 \times 82) + (0.006 \times 85) + (0.888 \times 58) + (1.984 \times 55)] / 3.010$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

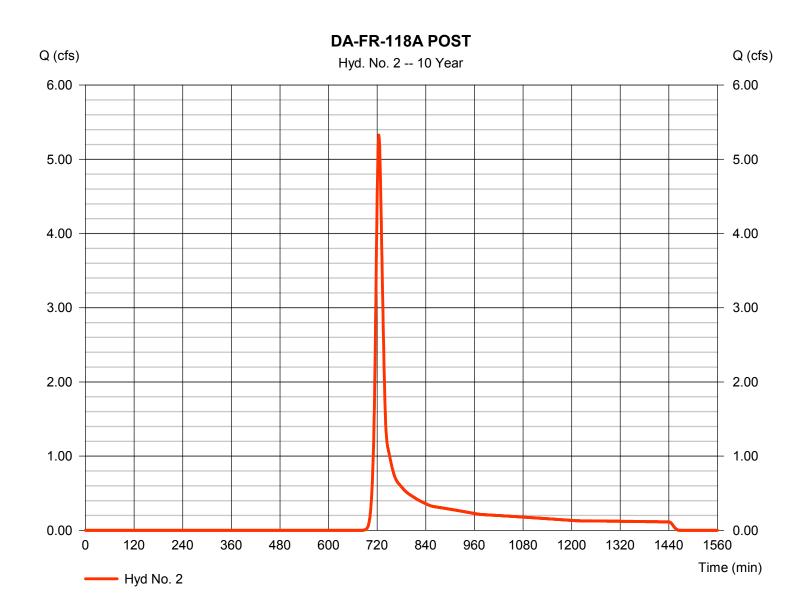
Wednesday, 08 / 16 / 2017

Hyd. No. 2

DA-FR-118A POST

Hydrograph type = SCS Runoff Peak discharge = 5.326 cfsStorm frequency = 10 yrsTime to peak = 724 min Time interval = 1 min Hyd. volume = 16.560 cuft Curve number Drainage area = 3.010 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 16.20 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inStorm duration = 484 = 24 hrs Shape factor

^{*} Composite (Area/CN) = [(0.156 x 48) + (0.135 x 82) + (0.006 x 85) + (1.357 x 58) + (1.359 x 55)] / 3.010



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

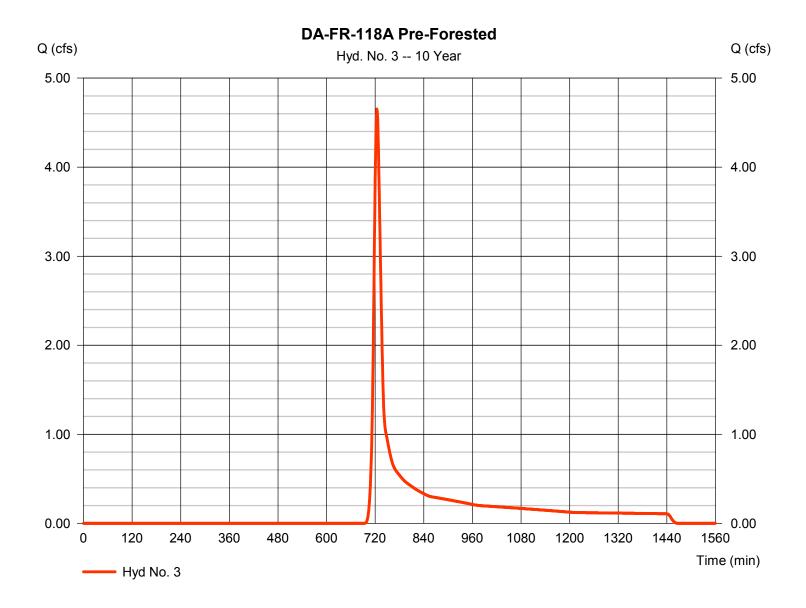
Wednesday, 08 / 16 / 2017

Hyd. No. 3

DA-FR-118A Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 4.652 cfsStorm frequency = 10 yrsTime to peak = 724 min Time interval = 1 min Hyd. volume = 14.918 cuft Curve number Drainage area = 3.010 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 = 16.10 min Total precip. Distribution = Type II = 5.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = [(3.010 x 55)] / 3.010



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

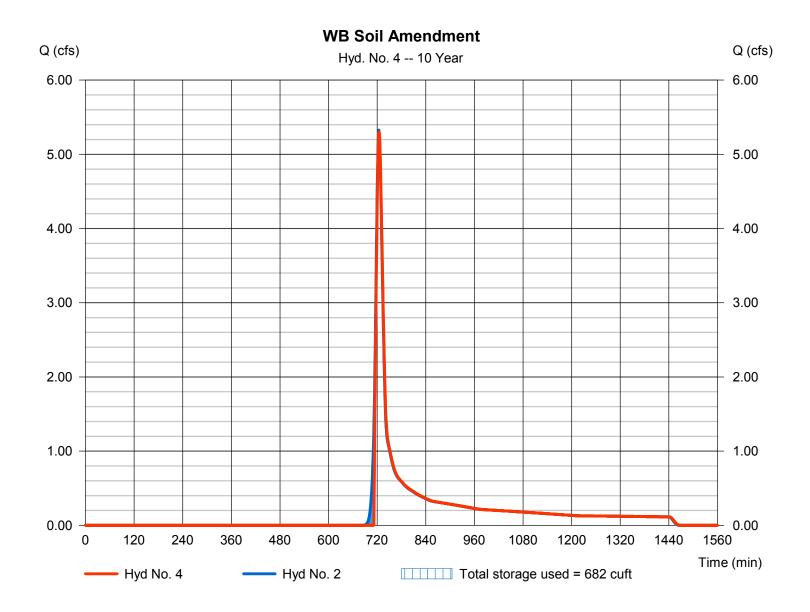
Wednesday, 08 / 16 / 2017

Hyd. No. 4

WB Soil Amendment

Hydrograph type Peak discharge = 5.302 cfs= Reservoir Storm frequency = 10 yrsTime to peak = 725 min Time interval = 1 min Hyd. volume = 16,149 cuft Inflow hyd. No. Max. Elevation = 2 - DA-FR-118A POST = 935.73 ftReservoir name = Waterbar Soil Amendment Max. Storage = 682 cuft

Storage Indication method used.



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 16 / 2017

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)										
(Yrs)	В	D	E	(N/A)							
1	0.0000	0.0000	0.0000								
2	69.8703	13.1000	0.8658								
3	0.0000	0.0000	0.0000								
5	79.2597	14.6000	0.8369								
10	88.2351	15.5000	0.8279								
25	102.6072	16.5000	0.8217								
50	114.8193	17.2000	0.8199								
100	127.1596	17.8000	0.8186								

File name: SampleFHA.idf

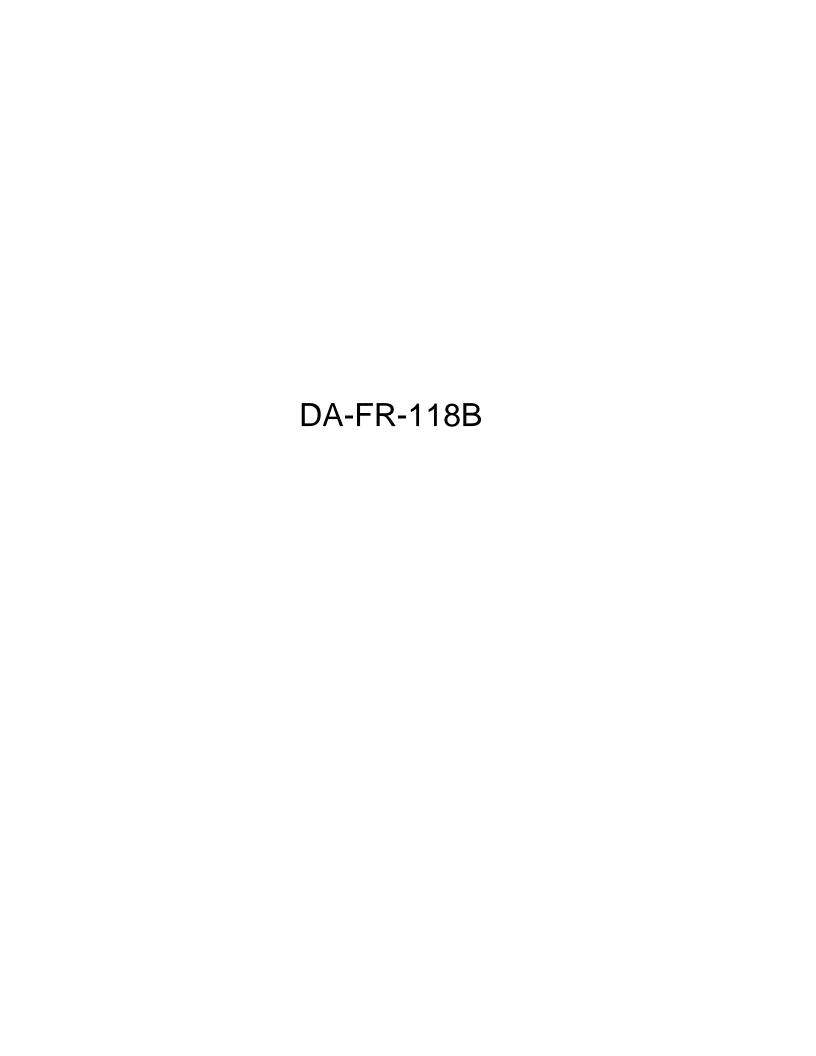
Intensity = B / (Tc + D)^E

Return Period (Yrs)	Intensity Values (in/hr)											
	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

Precip. file name: P:\MVP ESC&SWM Review\Engineering\SW Calculations\Franklin County.pcp

	Rainfall Precipitation Table (in)										
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr			
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00			
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			



STORAGE VOLUME OF WATERBAR WITH SOIL COMPOST AMENDMENT AREA

Equations Used:
¹Vgravel storage = L*W*D_{gravel}*(40/100)

²Vsoil storage = L*W*D_{soil}*(20/100)

 3 V surface storage = [W*S*D^2]+[L*S*D^2]+[W*L*D]+[((2*S*D)^2*D)/3]

¹Equation #2b under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, assuming that gravel is made up of 40% voids.

²Equation #2b under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, assuming that soil compost amendment is made up of 20% voids.

141.3333333

³Equation #1 under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, but calculation also takes into account surface side slopes.

Inputs:	Depti	n of Gravel Layer, D _{gravel} (ft) =	0	
	Depth of Soil	Amendment Area, D _{soil} (ft) =	1	Refer to Table 4.3 in VA DEQ Stormwater Design Specification No. 4; Note that compost amendment may not be necessary for HSG A/B soils
Length of Waterbar Soil Amendment Area, L (ft) =		30	Assume max. length of 50' for waterbar soil amendment areas (i.e., limited to permanent ROW)	
	Width of Waterbar So	il Amendment Area, W (ft) =	2	
	Inside Emban	kment Side Slopes, S (H:V) =	2	Assume 2H:1V surface side slopes for waterbars
	Number of Perm. Wa	terbars in Drainage Area, n =	9	
	Design Infiltration Rate, IR (in/hr) =		0.2	Min. rate of 0.30 in/hr for HSG A soils and 0.15-0.30 in/hr for HSG B soils (see Chap. 4, p. 4-30 in VA Stormwater Management Handbook Volume II (First Edition, 1999)
	Su	rface Ponding Depth, D (ft) =	0.5	Assume 0.5' CFS height at the end of waterbars
Calculations:		Storage Depth per BMP (ft) =	1.5	
	Surface Sto	orage Volume per BMP (cf) =	46.6666667	
	Subsurface Sto	orage Volume per BMP (cf) =	12	
	Total Sto	orage Volume per BMP (cf) =	58.6666667	
	Total BMP Storage Vo	olume in Drainage Area (cf) =	528	
	Calculated Infilt	ration Period per BMP (hr) =	58.6666667	
		Depth-Storage Da		
5 (6)	145 141 (61)	, ,		
Depth (ft)	Width (ft)	Length (ft)	Storage Volume per BMP (cf)	Storage Volume in Drainage Area (cf)
0	2	30	0	0
0.5	2	30	6	54
1	2	30	12	108
1.5	4	32	58.6666667	528

1272

ENERGY BALANCE METHOD

Inputs:

2

	1-Yr Event				
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)			
Pre-Developed Condition	0.493	2969			
Developed Condition	0.401	2729			
Pre-Developed (Forest) Condition	0.324	2428			

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.8

<u>Calculations:</u>	Check #1:	$Q_{\text{developed}} \leq \text{IF } x \left[\left(Q_{\text{pre-developed}} \times RV_{\text{pre-developed}} \right) / RV_{\text{developed}} \right]$	Q (cfs) 0.401	≤ OK	Q (cfs) 0.429
	Check #2:	$Q_{developed} \le Q_{pre-developed}$	0.401	≤ OK	0.493
	Check #3:	$Q_{developed}$ shall not be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed} \longrightarrow$	0.401	shall not be required to be ≤	0.288

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 - 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 - 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 - 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 - 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 - Manning's n Values for Open Channel Flow

hannel Type		Manning n			
			Min.	Normal	Max.
1.	Exc	cavated or Dredged Channels ¹			
	a.	Earth, Straight, and Uniform:			
		Clean, recently completed	0.016	0.018	0.020
		Clean, after weathering	0.018	0.022	0.02
		Gravel, uniform section, clean	0.022	0.025	0.030
		With short grass, few weeds	0.022	0.027	0.03
	b.	Earth Winding and Sluggish:			
		No vegetation	0.023	0.025	0.030
		Grass, some weeds	0.025	0.030	0.03
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040
		Earth bottom and rubble sides	0.028	0.030	0.03
		Stony bottom and weedy banks	0.025	0.035	0.040
		Cobble bottom and clean sides	0.030	0.040	0.050
	c.	Dragline-Excavated or Dredged:			
		No vegetation	0.025	0.028	0.03
		Light brush on banks	0.035	0.050	0.060
	d.	Rock Cuts:			
		Smooth and uniform	0.025	0.035	0.040
		Jagged and irregular	0.035	0.040	0.050
	e.	Channels not Maintained, Weeds and Brush Uncut:			
		Dense weeds, high as flow depth	0.050	0.080	0.120
		Clean bottom, brush on sides	0.040	0.050	0.080
		Same as above, highest stage of flow	0.045	0.070	0.110
		Dense brush, high stage	0.080	0.100	0.140
2.	Mai	n Channels²			
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.03
	b.	Same as above, but more stones and weeds	0.030	0.035	0.040
	C.	Clean, winding, some pools and shoals	0.033	0.040	0.04
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.05
	f.	Same as (d) with more stones	0.045	0.050	0.060
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

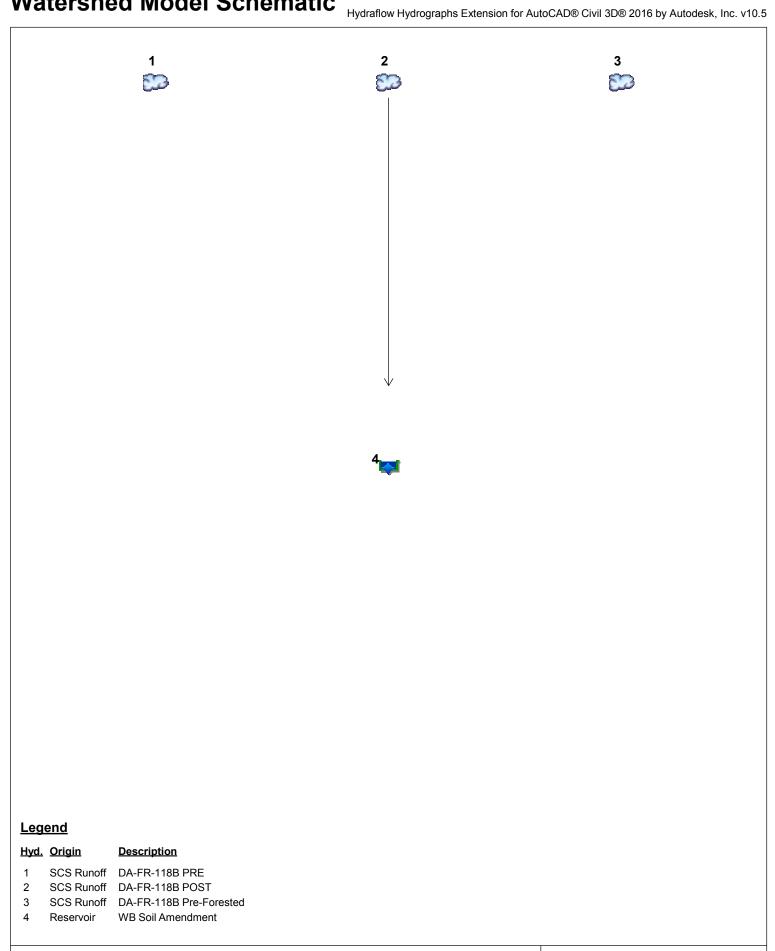
-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Watershed Model Schematic	1
Hydrograph Return Period Recap	. 2
1 - Year	
Summary Report	. 3
Hydrograph Reports	. 4
Hydrograph No. 1, SCS Runoff, DA-FR-118B PRE	. 4
TR-55 Tc Worksheet	. 5
Hydrograph No. 2, SCS Runoff, DA-FR-118B POST	. 6
TR-55 Tc Worksheet	
Hydrograph No. 3, SCS Runoff, DA-FR-118B Pre-Forested	
TR-55 Tc Worksheet	
Hydrograph No. 4, Reservoir, WB Soil Amendment	
Pond Report - Waterbar Soil Amendment	11
2 - Year	
Summary Report	12
Hydrograph ReportsHydrograph No. 1, SCS Runoff, DA-FR-118B PRE	13
Hydrograph No. 1, SCS Runoff, DA-FR-118B PRE	13
Hydrograph No. 2, SCS Runoff, DA-FR-118B POST	
Hydrograph No. 3, SCS Runoff, DA-FR-118B Pre-Forested	
Hydrograph No. 4, Reservoir, WB Soil Amendment	16
40 Vaan	
10 - Year	
Summary Report	
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-118B PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-118B POST	19
Hydrograph No. 3, SCS Runoff, DA-FR-118B Pre-Forested	20
Hydrograph No. 4, Reservoir, WB Soil Amendment	2 1
IDE Panart	22
IDF Report	44

Project: DA-FR-118b.gpw



Hydrograph Return Period Recap Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	Hydrograph	Inflow				Hydrograph					
lo.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		0.493	0.875			3.748				DA-FR-118B PRE
2	SCS Runoff		0.589	1.003			3.993				DA-FR-118B POST
3	SCS Runoff		0.324	0.640			3.264				DA-FR-118B Pre-Forested
4	Reservoir	2	0.401	0.911			3.976				WB Soil Amendment

Proj. file: DA-FR-118b.gpw

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

			_				ision for Autocade Civil 3De 2010 by Autodesk, in			
lyd. Io.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	0.493	1	729	2,969				DA-FR-118B PRE	
2	SCS Runoff	0.589	1	729	3,257				DA-FR-118B POST	
3	SCS Runoff	0.324	1	730	2,428				DA-FR-118B Pre-Forested	
4	Reservoir	0.401	1	740	2,729	2	955.53	573	WB Soil Amendment	
DA	-FR-118b.gp	W			Return	Period: 1 Y	ear	Wednesda	ay, 08 / 16 / 2017	

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

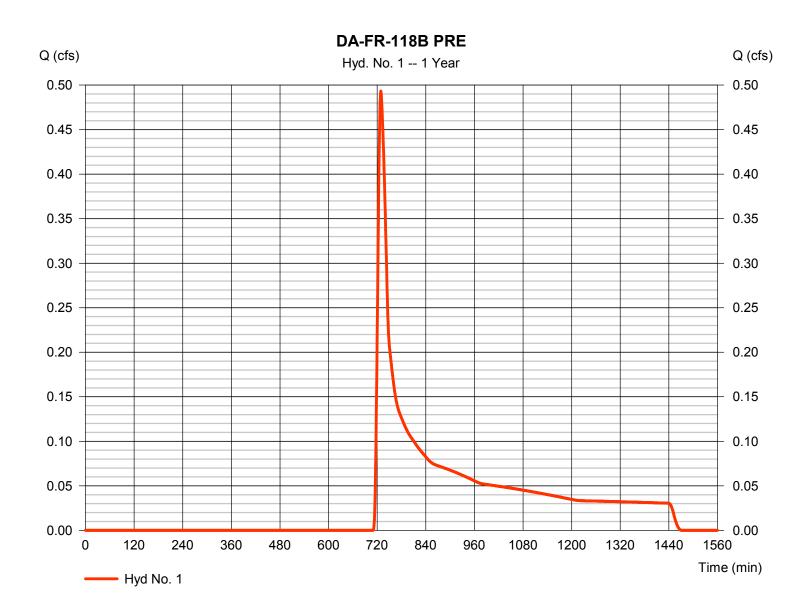
Wednesday, 08 / 16 / 2017

Hyd. No. 1

DA-FR-118B PRE

Hydrograph type = SCS Runoff Peak discharge = 0.493 cfsStorm frequency Time to peak = 729 min = 1 yrsTime interval = 1 min Hyd. volume = 2.969 cuftCurve number Drainage area = 2.380 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 18.80 \, \text{min}$ Total precip. Distribution = Type II = 3.30 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.128 x 85) + (0.620 x 58) + (1.635 x 55)] / 2.380



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1DA-FR-118B PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 3.20 = 16.55	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	16.55
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 715.60 = 10.90 = Unpaved =5.33	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 2.24	+	0.00	+	0.00	=	2.24
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.040 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							18.80 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

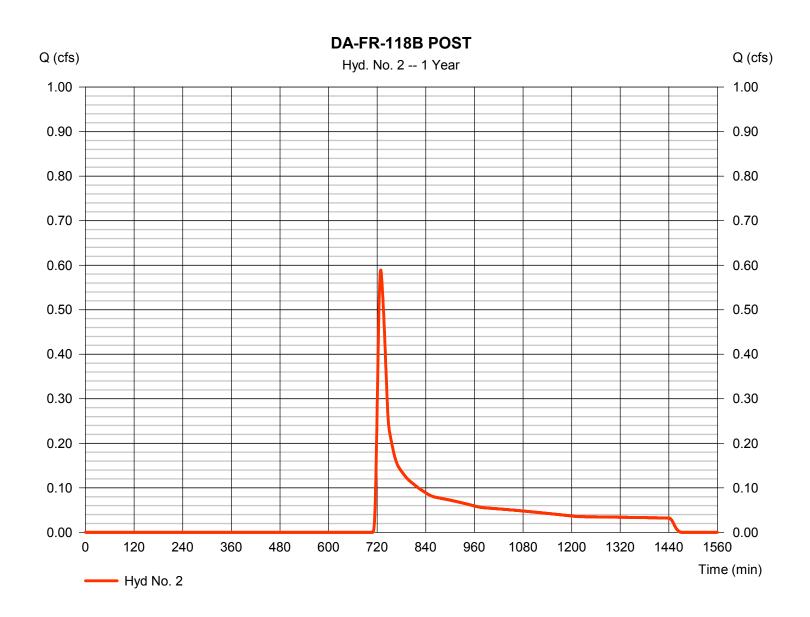
Wednesday, 08 / 16 / 2017

Hyd. No. 2

DA-FR-118B POST

Hydrograph type = SCS Runoff Peak discharge = 0.589 cfsStorm frequency Time to peak = 729 min = 1 yrsTime interval = 1 min Hyd. volume = 3.257 cuft Curve number Drainage area = 2.380 ac= 58* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 18.90 \, \text{min}$ Total precip. Distribution = Type II = 3.30 inStorm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.192 \times 48) + (0.128 \times 85) + (1.355 \times 58) + (0.707 \times 55)] / 2.380$



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2DA-FR-118B POST

<u>Description</u>	<u>A</u>		<u>B</u>		<u>c</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 3.20		0.800 0.0 0.00 0.00		0.011 0.0 0.00 0.00		40.55
Travel Time (min)	= 16.55	+	0.00	+	0.00	=	16.55
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 203.80 = 7.30 = Unpaved =4.36	d	511.40 12.20 Unpave 5.64	d	0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.78	+	1.51	+	0.00	=	2.29
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})15.6		0.0		0.0		
Travel Time (min)	= 0.05	+	0.00	+	0.00	=	0.05
Total Travel Time, Tc							18.90 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

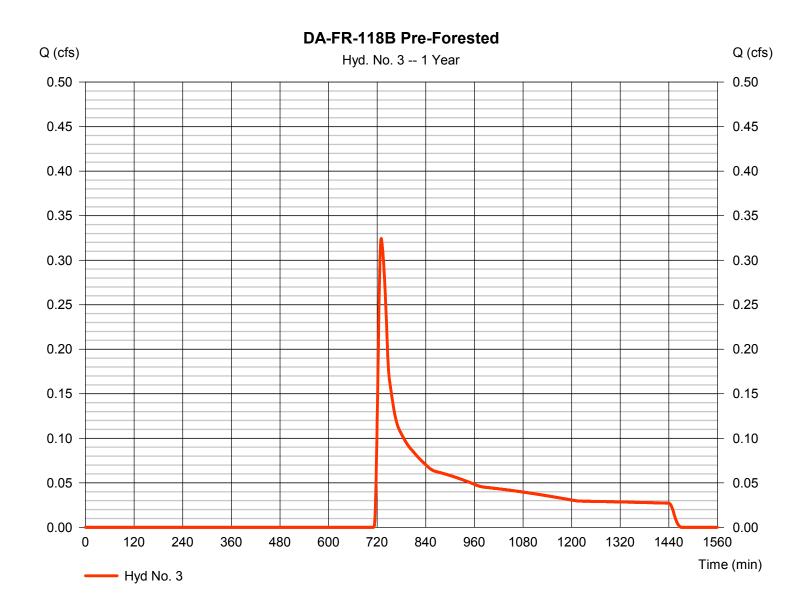
Wednesday, 08 / 16 / 2017

Hyd. No. 3

DA-FR-118B Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 0.324 cfsStorm frequency Time to peak = 730 min = 1 yrsTime interval = 1 min Hyd. volume = 2,428 cuft Curve number Drainage area = 2.380 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 18.80 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(2.380 x 55)] / 2.380



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3DA-FR-118B Pre-Forested

<u>Description</u>	<u>A</u>		<u>B</u>		<u>c</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 3.20		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 16.55	+	0.00	+	0.00	=	16.55
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 715.60 = 10.90 = Unpaved =5.33	t	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 2.24	+	0.00	+	0.00	=	2.24
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value	= 0.00 = 0.00 = 0.00 = 0.030		0.00 0.00 0.00		0.00 0.00 0.00		
Velocity (ft/s)	=0.00		0.015		0.015		
Velocity (ft/s)	=0.00	+	0.00	+	0.00	=	0.00

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

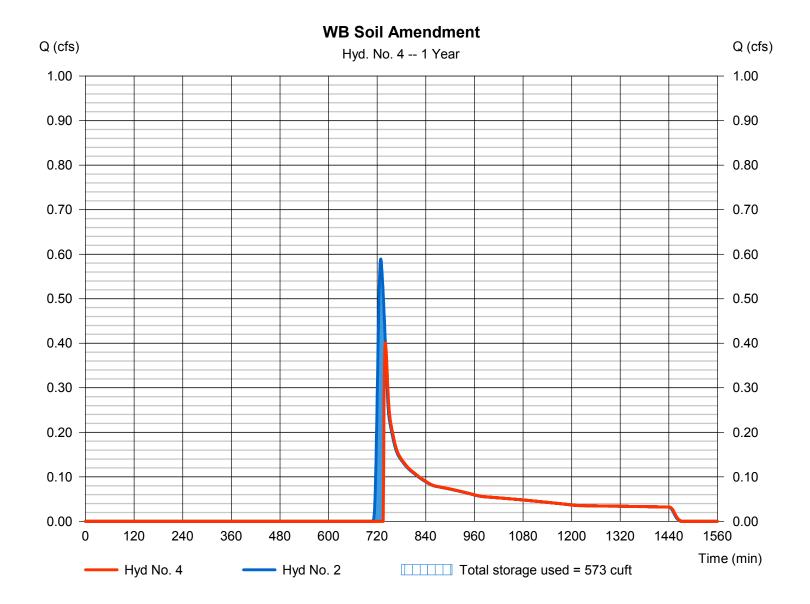
Wednesday, 08 / 16 / 2017

Hyd. No. 4

WB Soil Amendment

Hydrograph type Peak discharge = 0.401 cfs= Reservoir Storm frequency Time to peak = 740 min = 1 yrsTime interval = 1 min Hyd. volume = 2,729 cuftInflow hyd. No. Max. Elevation = 2 - DA-FR-118B POST $= 955.53 \, \text{ft}$ Reservoir name = Waterbar Soil Amendment Max. Storage = 573 cuft

Storage Indication method used.



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 16 / 2017

Pond No. 1 - Waterbar Soil Amendment

Pond Data

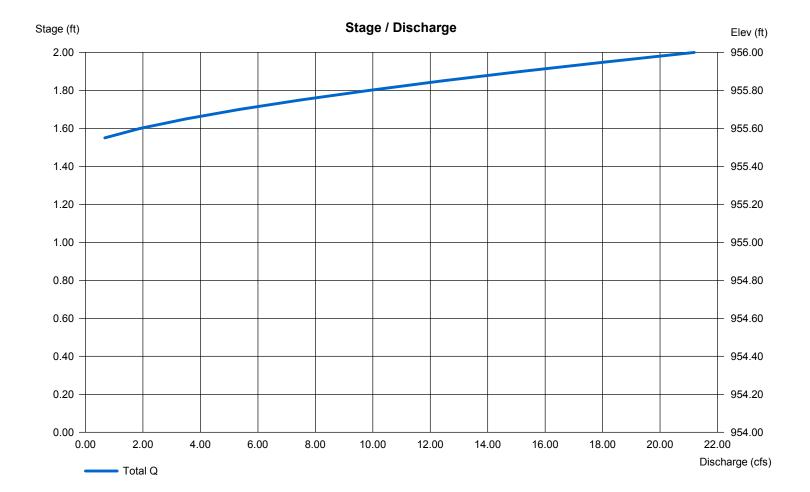
Pond storage is based on user-defined values.

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	954.00	n/a	0	0
0.50	954.50	n/a	54	54
1.00	955.00	n/a	54	108
1.50	955.50	n/a	420	528
2.00	956.00	n/a	744	1,272

Culvert / Orifice Structures Weir Structures [A] [B] [C] [D] [A] [B] [C] [PrfRsr] Rise (in) = 0.000.00 0.00 0.00 Crest Len (ft) = 18.00 0.00 0.00 0.00 Span (in) = 0.00 0.00 0.00 0.00 Crest El. (ft) = 955.50 0.00 0.00 0.00 No. Barrels = 0 0 Weir Coeff. = 3.333.33 3.33 3.33 = 0.000.00 0.00 0.00 Weir Type = Rect Invert El. (ft) = 0.000.00 0.00 0.00 Multi-Stage Length (ft) = No No No No Slope (%) = 0.000.00 0.00 n/a N-Value = .013 .013 .013 n/a 0.60 = 0.600.60 0.60 Exfil.(in/hr) = 0.000 (by Wet area) Orifice Coeff. Multi-Stage = n/aNo No No TW Elev. (ft) = 0.00

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

		_				nyuranow n	yurograpris Exte	Rension for AutoCad® Civil 3D® 2016 by Autodesk, Inc. V10					ISION TO AUTOCADO CIVIL 3DO 2010 by Autodesk, Inc. V10			
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description							
1	SCS Runoff	0.875	1	728	4,260				DA-FR-118B PRE							
2	SCS Runoff	1.003	1	728	4,614				DA-FR-118B POST							
3	SCS Runoff	0.640	1	729	3,590				DA-FR-118B Pre-Forested							
4	Reservoir	0.911	1	732	4,086	2	955.56	617	WB Soil Amendment							
DA-	-FR-118b.gpv	V			Return F	Period: 2 Ye	ear	Wednesday	y, 08 / 16 / 2017							
								1								

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

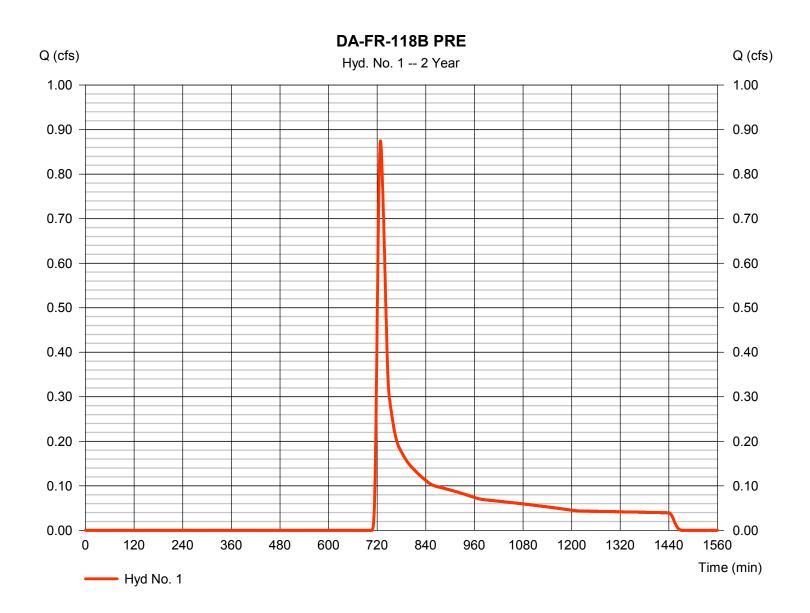
Wednesday, 08 / 16 / 2017

Hyd. No. 1

DA-FR-118B PRE

Hydrograph type = SCS Runoff Peak discharge = 0.875 cfsStorm frequency = 2 yrsTime to peak = 728 min Time interval = 1 min Hyd. volume = 4,260 cuftCurve number Drainage area = 2.380 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 18.80 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = [(0.128 x 85) + (0.620 x 58) + (1.635 x 55)] / 2.380



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

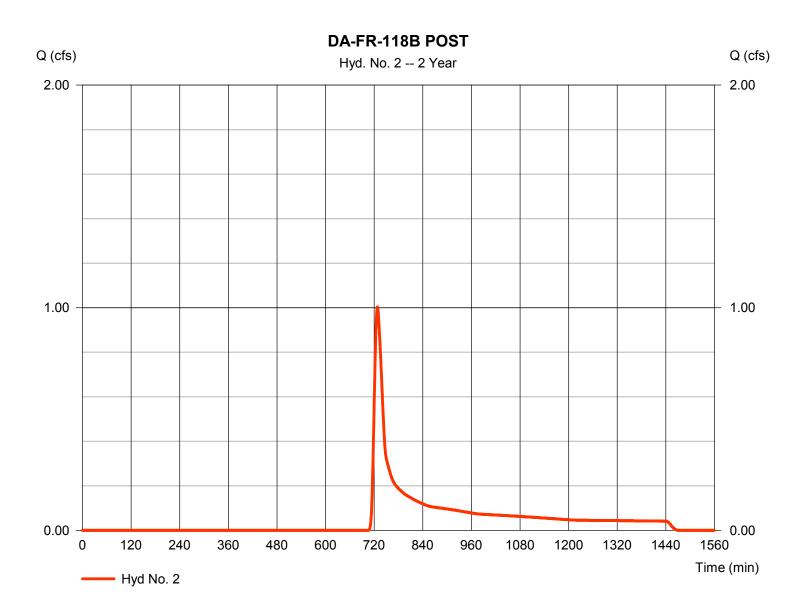
Wednesday, 08 / 16 / 2017

Hyd. No. 2

DA-FR-118B POST

Hydrograph type = SCS Runoff Peak discharge = 1.003 cfsStorm frequency = 2 yrsTime to peak = 728 min Time interval = 1 min Hyd. volume = 4,614 cuftCurve number Drainage area = 2.380 ac= 58* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 18.90 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.192 \times 48) + (0.128 \times 85) + (1.355 \times 58) + (0.707 \times 55)] / 2.380$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

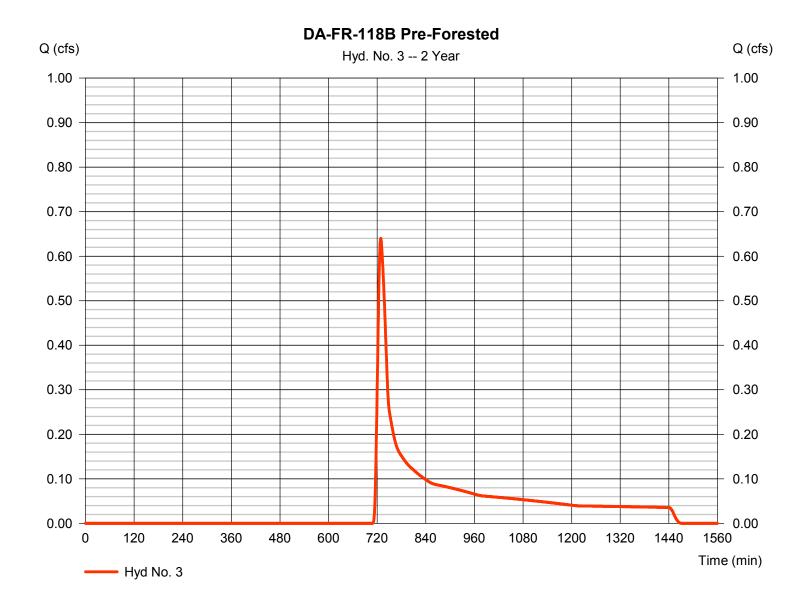
Wednesday, 08 / 16 / 2017

Hyd. No. 3

DA-FR-118B Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 0.640 cfsStorm frequency = 2 yrsTime to peak = 729 min Time interval = 1 min Hyd. volume = 3,590 cuftCurve number Drainage area = 2.380 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 18.80 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = [(2.380 x 55)] / 2.380



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

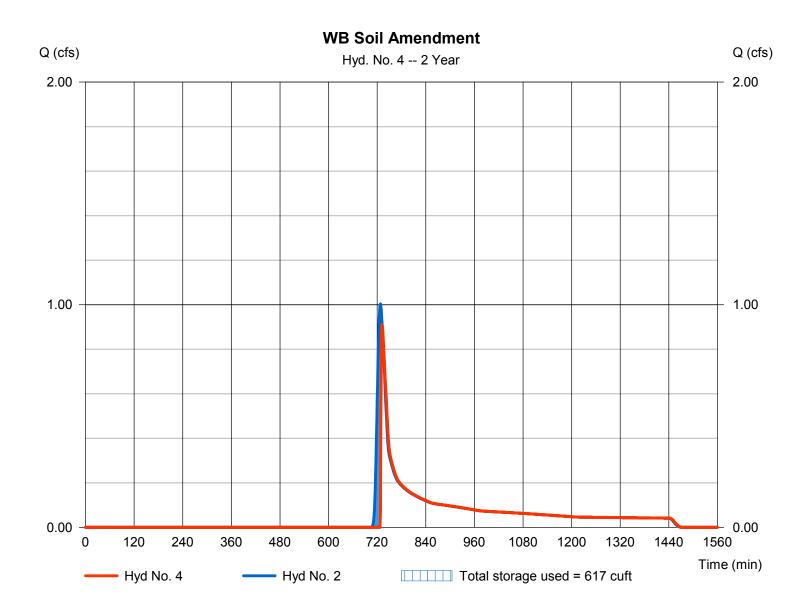
Wednesday, 08 / 16 / 2017

Hyd. No. 4

WB Soil Amendment

Hydrograph type Peak discharge = 0.911 cfs= Reservoir Storm frequency = 2 yrs Time to peak = 732 min Time interval = 1 min Hyd. volume = 4,086 cuftInflow hyd. No. = 2 - DA-FR-118B POST Max. Elevation $= 955.56 \, \text{ft}$ Reservoir name = Waterbar Soil Amendment Max. Storage = 617 cuft

Storage Indication method used.



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

			_				w riyarograpiis Ext		—,	
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	3.748	1	726	12,932				DA-FR-118B PRE	
2	SCS Runoff	3.993	1	726	13,589				DA-FR-118B POST	
3	SCS Runoff	3.264	1	726	11,650				DA-FR-118B Pre-Forested	
4	Reservoir	3.976	1	727	13,061	2	955.66	771	WB Soil Amendment	
DA	-FR-118b.gpv	N			Return F	Period: 10 \	 Year	Wednesda	y, 08 / 16 / 2017	

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

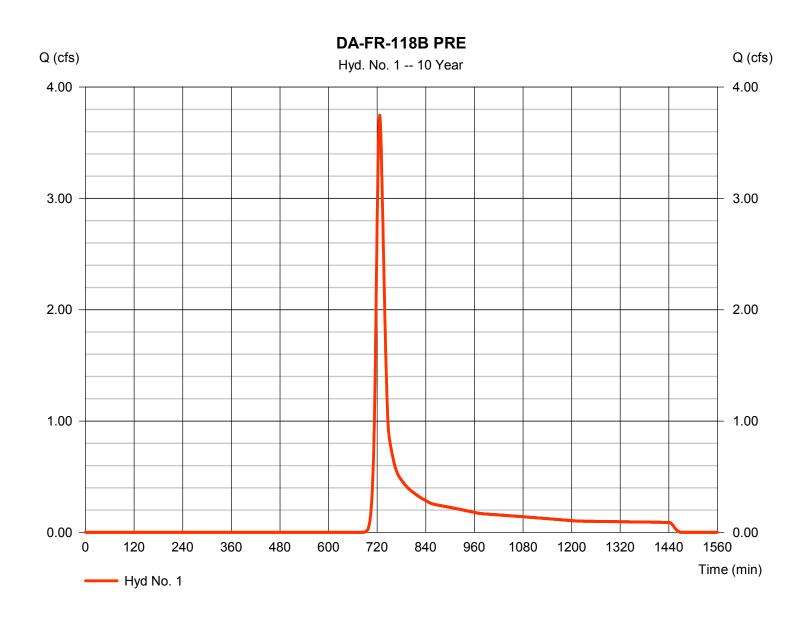
Wednesday, 08 / 16 / 2017

Hyd. No. 1

DA-FR-118B PRE

Hydrograph type = SCS Runoff Peak discharge = 3.748 cfsStorm frequency = 10 yrsTime to peak = 726 min Time interval = 1 min Hyd. volume = 12.932 cuft Curve number Drainage area = 2.380 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 18.80 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = [(0.128 x 85) + (0.620 x 58) + (1.635 x 55)] / 2.380



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

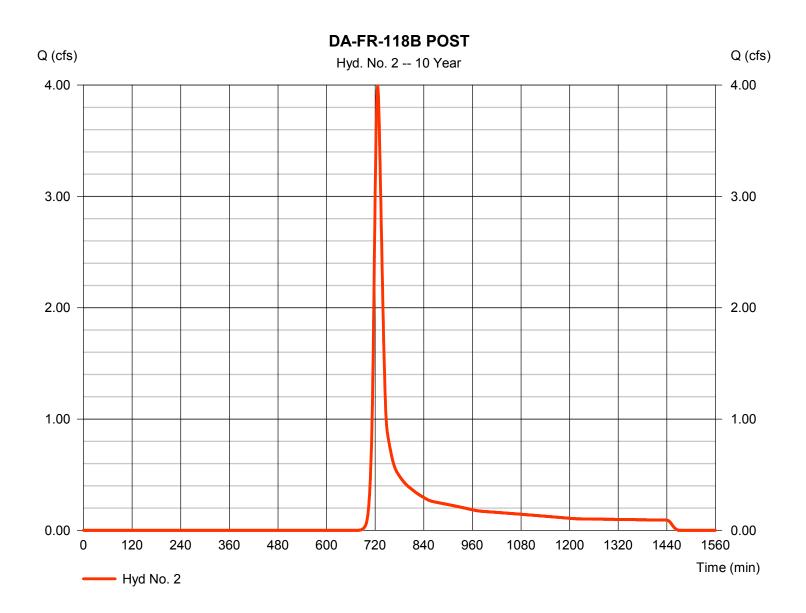
Wednesday, 08 / 16 / 2017

Hyd. No. 2

DA-FR-118B POST

Hydrograph type = SCS Runoff Peak discharge = 3.993 cfsStorm frequency = 10 yrsTime to peak = 726 min Time interval = 1 min Hyd. volume = 13.589 cuft Curve number Drainage area = 2.380 ac= 58* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 18.90 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.192 x 48) + (0.128 x 85) + (1.355 x 58) + (0.707 x 55)] / 2.380



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

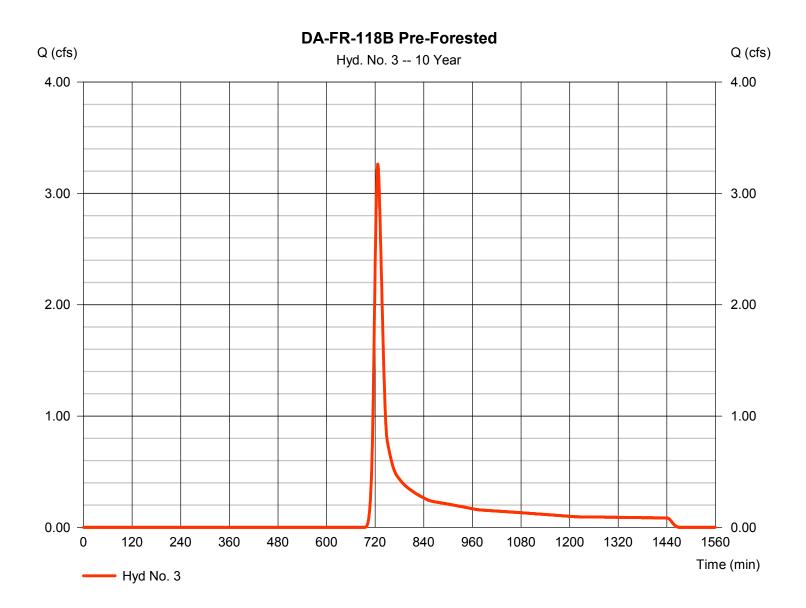
Wednesday, 08 / 16 / 2017

Hyd. No. 3

DA-FR-118B Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 3.264 cfsStorm frequency = 10 yrsTime to peak = 726 min Time interval = 1 min Hyd. volume = 11.650 cuft Curve number Drainage area = 2.380 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 18.80 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(2.380 x 55)] / 2.380



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

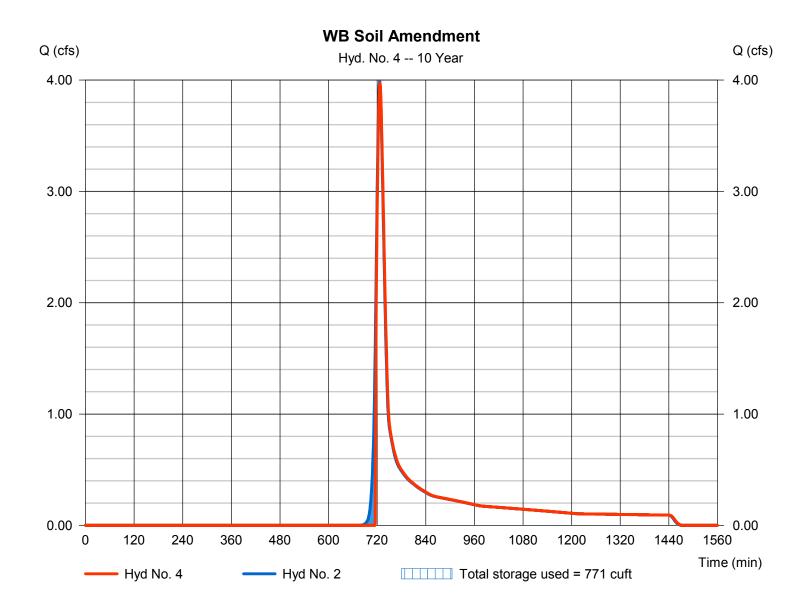
Wednesday, 08 / 16 / 2017

Hyd. No. 4

WB Soil Amendment

Hydrograph type Peak discharge = 3.976 cfs= Reservoir Storm frequency = 10 yrsTime to peak = 727 min Time interval = 1 min Hyd. volume = 13,061 cuft Inflow hyd. No. Max. Elevation = 955.66 ft = 2 - DA-FR-118B POST = 771 cuft Reservoir name = Waterbar Soil Amendment Max. Storage

Storage Indication method used.



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 16 / 2017

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)								
(Yrs)	В	D	E	(N/A)					
1	0.0000	0.0000	0.0000						
2	69.8703	13.1000	0.8658						
3	0.0000	0.0000	0.0000						
5	79.2597	14.6000	0.8369						
10	88.2351	15.5000	0.8279						
25	102.6072	16.5000	0.8217						
50	114.8193	17.2000	0.8199						
100	127.1596	17.8000	0.8186						

File name: SampleFHA.idf

Intensity = B / (Tc + D)^E

Return	Intensity Values (in/hr)												
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60	
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70	
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15	
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46	
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91	
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25	
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60	

Tc = time in minutes. Values may exceed 60.

Precip. file name: P:\MVP ESC&SWM Review\Engineering\SW Calculations\Franklin County.pcp

		Rainfall Precipitation Table (in)									
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr			
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00			
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			



STORAGE VOLUME OF WATERBAR WITH SOIL COMPOST AMENDMENT AREA

Equations Used:
¹Vgravel storage = L*W*D_{gravel}*(40/100)

²Vsoil storage = L*W*D_{soil}*(20/100)

 3 V surface storage = [W*S*D^2]+[L*S*D^2]+[W*L*D]+[((2*S*D)^2*D)/3]

¹Equation #2b under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, assuming that gravel is made up of 40% voids.

²Equation #2b under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, assuming that soil compost amendment is made up of 20% voids.

³Equation #1 under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, but calculation also takes into account surface side slopes.

Inputs:	Depth	of Gravel Layer, D _{gravel} (ft) =	0	
	Depth of Soil	Amendment Area, D _{soil} (ft) =	1	Refer to Table 4.3 in VA DEQ Stormwater Design Specification No. 4; Note that compost amendment may not be necessary for HSG A/B soils
	Length of Waterbar So	oil Amendment Area, L (ft) =	50	Assume max. length of 50' for waterbar soil amendment areas (i.e., limited to permanent ROW)
	Width of Waterbar So	il Amendment Area, W (ft) =	2	
		kment Side Slopes, S (H:V) =	2	Assume 2H:1V surface side slopes for waterbars
	Number of Perm. Wat	erbars in Drainage Area, n =	9	
	Design	Infiltration Rate, IR (in/hr) =	0.2	Min. rate of 0.30 in/hr for HSG A soils and 0.15-0.30 in/hr for HSG B soils (see Chap. 4, p. 4-30 in VA Stormwater Management Handbook Volume II (First Edition, 1999)
	Sur	face Ponding Depth, D (ft) =	0.5	Assume 0.5' CFS height at the end of waterbars
Calculations:		torage Depth per BMP (ft) =		
		rage Volume per BMP (cf) =		
	Subsurface Sto	rage Volume per BMP (cf) =	20	
	Total Sto	rage Volume per BMP (cf) =	96.6666667	
	Total BMP Storage Vo	lume in Drainage Area (cf) =	870	
	Calculated Infilt	ration Period per BMP (hr) =	58	
		Depth-Storage Da		
Depth (ft)	Width (ft)	Length (ft)	Storage Volume per BMP (cf)	Storage Volume in Drainage Area (cf)
0	2	50	0	0
0.5	2	50	10	90
1	2	50	20	180
1.5	4	52	96.66666667	870
2	6	54	229.3333333	2064

ENERGY BALANCE METHOD

Inputs:

	1-Yr Event					
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)				
Pre-Developed Condition	2.438	6226				
Developed Condition	2.223	5356				
Pre-Developed (Forest) Condition	2.106	5529				

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.8

Calculations:	Check #1:	$Q_{\text{developed}} \leq \text{IF x } \left[\left(Q_{\text{pre-developed}} \times RV_{\text{pre-developed}} \right) / RV_{\text{developed}} \right]$	Q (cfs) 2.223	≤ OK	Q (cfs) 2.267
	Check #2:	$Q_{developed} \le Q_{pre-developed}$	2.223	≤ OK	2.438
	Check #3:	$Q_{developed}$ shall not be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed}$ >	2.223	shall not be required to be ≤	2.174

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 – Manning's n Values for Open Channel Flow

hanr	nel T	уре	Manning n				
			Min.	Normal	Max.		
1.	Exc	avated or Dredged Channels ¹					
	a.	Earth, Straight, and Uniform:					
		Clean, recently completed	0.016	0.018	0.020		
		Clean, after weathering	0.018	0.022	0.025		
		Gravel, uniform section, clean	0.022	0.025	0.030		
		With short grass, few weeds	0.022	0.027	0.033		
	b.	Earth Winding and Sluggish:					
		No vegetation	0.023	0.025	0.030		
		Grass, some weeds	0.025	0.030	0.033		
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040		
		Earth bottom and rubble sides	0.028	0.030	0.035		
		Stony bottom and weedy banks	0.025	0.035	0.040		
		Cobble bottom and clean sides	0.030	0.040	0.050		
	c.	Dragline-Excavated or Dredged:		•			
		No vegetation	0.025	0.028	0.033		
		Light brush on banks	0.035	0.050	0.060		
	d.	Rock Cuts:					
		Smooth and uniform	0.025	0.035	0.040		
		Jagged and irregular	0.035	0.040	0.050		
	e.	Channels not Maintained, Weeds and Brush Uncut:		•			
		Dense weeds, high as flow depth	0.050	0.080	0.120		
		Clean bottom, brush on sides	0.040	0.050	0.080		
		Same as above, highest stage of flow	0.045	0.070	0.110		
		Dense brush, high stage	0.080	0.100	0.140		
2.	Mai	n Channels²					
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.033		
	b.	Same as above, but more stones and weeds	0.030	0.035	0.040		
	c.	Clean, winding, some pools and shoals	0.033	0.040	0.04		
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050		
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.055		
	f.	Same as (d) with more stones	0.045	0.050	0.060		
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080		
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150		

Notes:

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

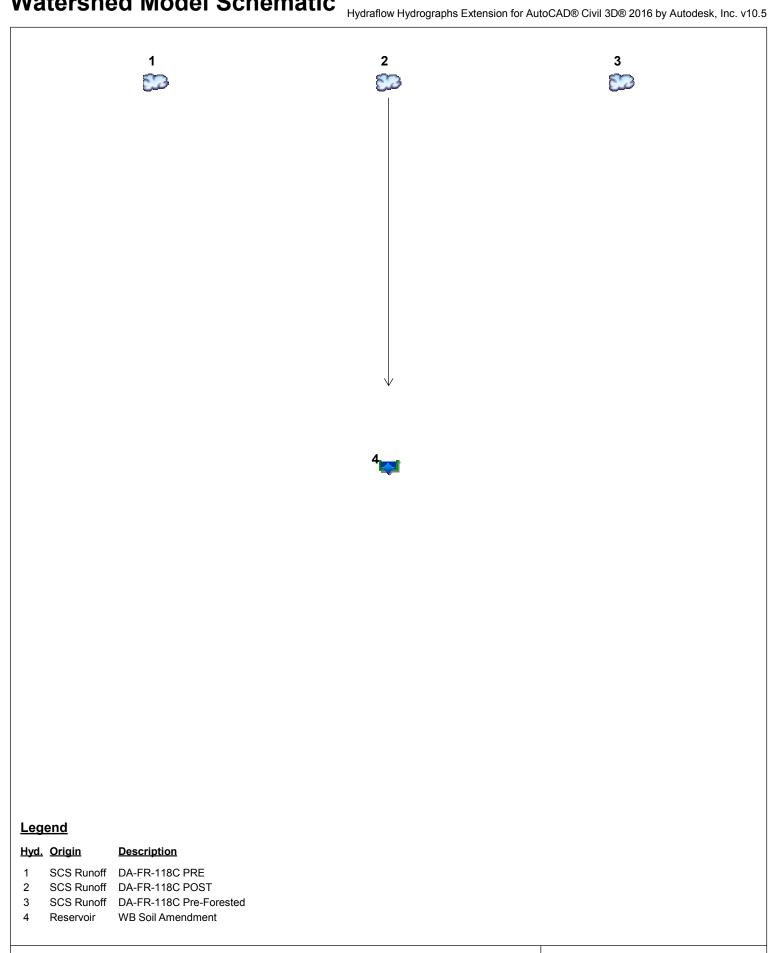
-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5 $\,$

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year Summary Report	4 5 6 7 8 9 10
2 - Year Summary Report	13 13 14 15
10 - Year Summary Report	18 18 19 20
IDE Bonort	2

Project: DA-FR-118c.gpw



Hydrograph Return Period Recap Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	Hydrograph	Inflow				Hydrograph						
lo.	type (origin)	hyd(s)	1-yr	2-yr	3-yr 5-yr		-yr 10-yr		25-yr 50-yr		Description	
1	SCS Runoff		2.438	3.188			7.444				DA-FR-118C PRE	
2	SCS Runoff		2.438	3.188			7.444				DA-FR-118C POST	
3	SCS Runoff		2.106	2.817			6.916				DA-FR-118C Pre-Forested	
4	Reservoir	2	2.223	3.087			7.366				WB Soil Amendment	

Proj. file: DA-FR-118c.gpw

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

			_		_	,	., a. eg. ap e =		The Civil 3D® 2010 by Autodesk, Inc.			
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description			
1	SCS Runoff	2.438	1	721	6,226				DA-FR-118C PRE			
2	SCS Runoff	2.438	1	721	6,226				DA-FR-118C POST			
3	SCS Runoff	2.106	1	721	5,529				DA-FR-118C Pre-Forested			
4	Reservoir	2.223	1	724	5,356	2	921.61	1,134	WB Soil Amendment			
	-FR-118c.gp\	w			Return	Return Period: 1 Year			Wednesday, 08 / 16 / 2017			

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

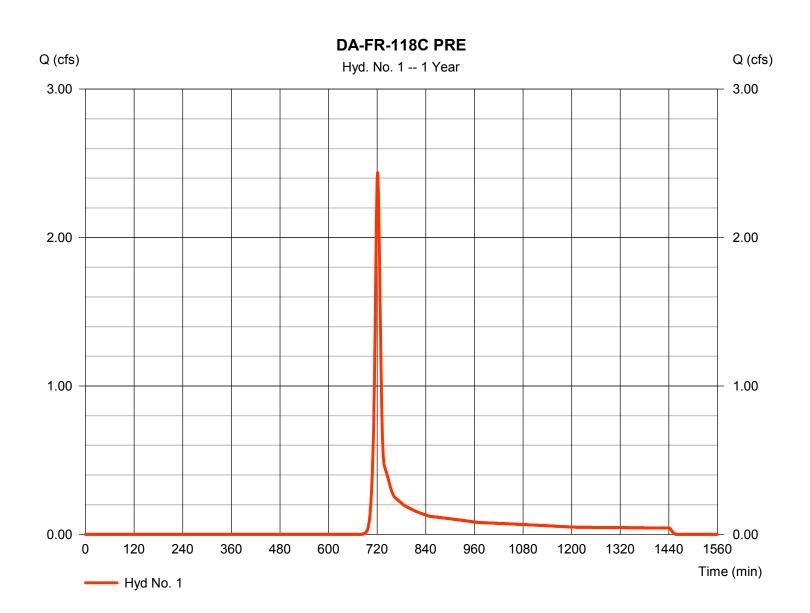
Wednesday, 08 / 16 / 2017

Hyd. No. 1

DA-FR-118C PRE

Hydrograph type = SCS Runoff Peak discharge = 2.438 cfsStorm frequency Time to peak = 721 min = 1 yrsTime interval = 1 min Hyd. volume = 6.226 cuft Curve number Drainage area = 1.900 ac= 70* Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 11.40 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs = 484 Shape factor

^{*} Composite (Area/CN) = $[(0.029 \times 85) + (0.105 \times 89) + (0.068 \times 58) + (0.751 \times 71) + (0.179 \times 55) + (0.770 \times 70)] / 1.900$



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1DA-FR-118C PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 10.70	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	10.21
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 401.70 = 12.50 = Unpaved =5.70	i	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.17	+	0.00	+	0.00	=	1.17
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.040 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							11.40 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

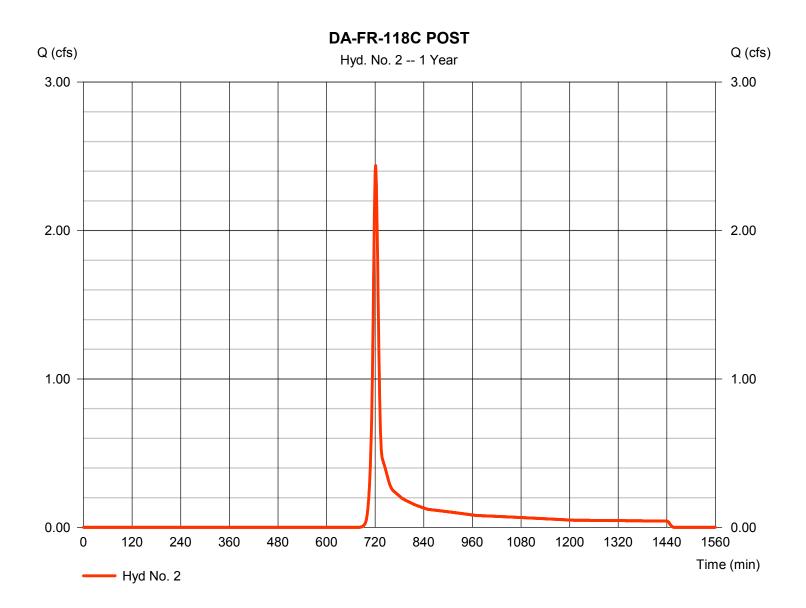
Wednesday, 08 / 16 / 2017

Hyd. No. 2

DA-FR-118C POST

Hydrograph type = SCS Runoff Peak discharge = 2.438 cfsStorm frequency Time to peak = 721 min = 1 yrsTime interval = 1 min Hyd. volume = 6,226 cuft Curve number Drainage area = 1.900 ac= 70* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 11.50 min = TR55 Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(1.900 x 70)] / 1.900



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2DA-FR-118C POST

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 10.70		0.800 0.0 0.00 0.00		0.011 0.0 0.00 0.00			
Travel Time (min)	= 10.21	+	0.00	+	0.00	=	10.21	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 127.10 = 12.30 = Unpaved =5.66	d	271.60 12.30 Unpave 5.66	d	0.00 0.00 Paved 0.00			
Travel Time (min)	= 0.37	+	0.80	+	0.00	=	1.17	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015			
Flow length (ft)	({0})20.5		0.0		0.0			
Travel Time (min)	= 0.07	+	0.00	+	0.00	=	0.07	
Total Travel Time, Tc1								

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

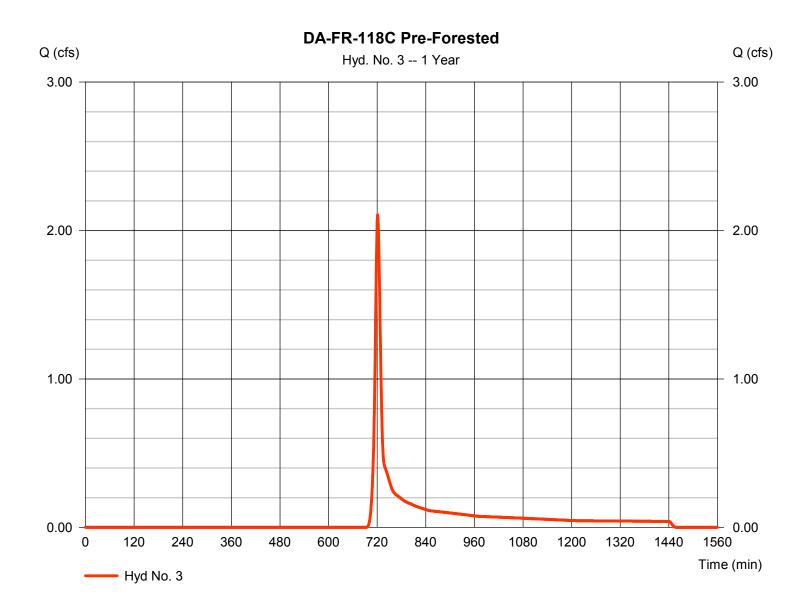
Wednesday, 08 / 16 / 2017

Hyd. No. 3

DA-FR-118C Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 2.106 cfsStorm frequency Time to peak = 721 min = 1 yrsTime interval = 1 min Hyd. volume = 5,529 cuftCurve number Drainage area = 1.900 ac= 68* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 11.40 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.280 x 55) + (1.620 x 70)] / 1.900



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3DA-FR-118C Pre-Forested

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 10.70	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00	=	10.21	
maver mine (min)	- 10.21	•	0.00	•	0.00	_	10.21	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 401.70 = 12.50 = Unpaved =5.70	I	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00			
Travel Time (min)	= 1.17	+	0.00	+	0.00	=	1.17	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.030 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015			
Flow length (ft)	({0})0.0		0.0		0.0			
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00	
Total Travel Time, Tc								

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

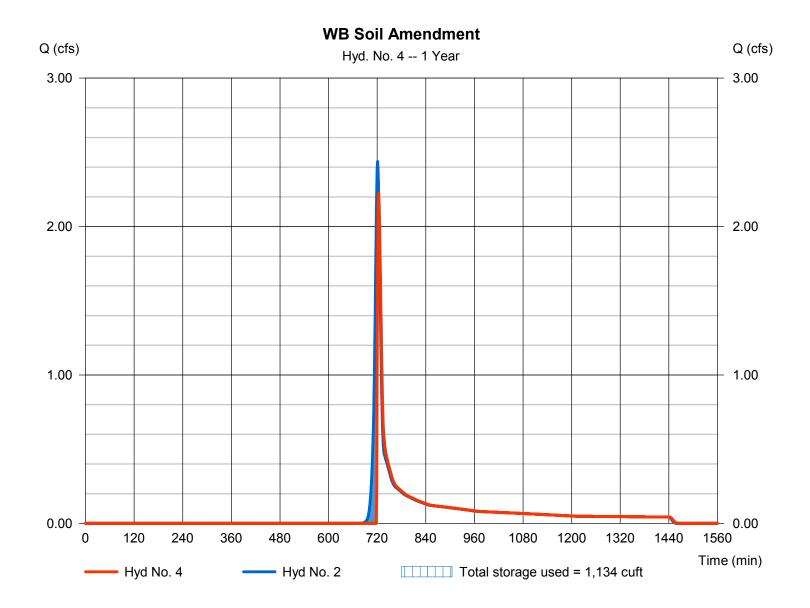
Wednesday, 08 / 16 / 2017

Hyd. No. 4

WB Soil Amendment

Hydrograph type Peak discharge = 2.223 cfs= Reservoir Storm frequency Time to peak = 724 min = 1 yrsTime interval = 1 min Hyd. volume = 5,356 cuftInflow hyd. No. = 2 - DA-FR-118C POST Max. Elevation $= 921.61 \, \text{ft}$ Reservoir name = Waterbar Soil Amendment Max. Storage = 1,134 cuft

Storage Indication method used.



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 16 / 2017

Pond No. 1 - Waterbar Soil Amendment

Pond Data

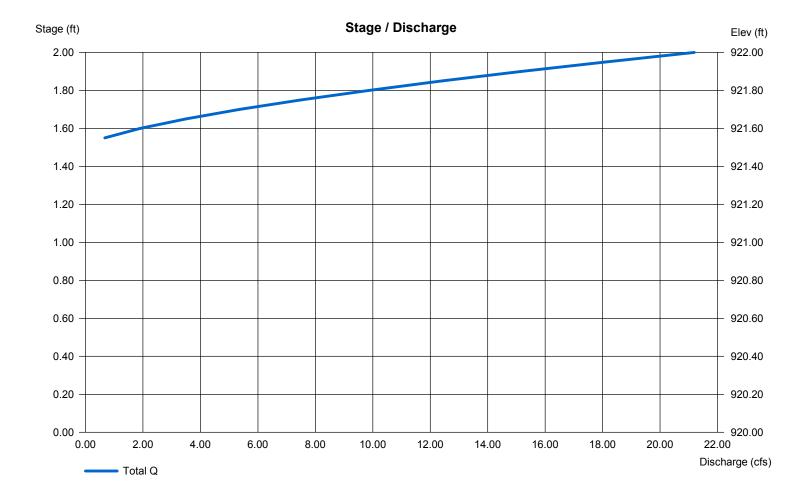
Pond storage is based on user-defined values.

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	920.00	n/a	0	0
0.50	920.50	n/a	90	90
1.00	921.00	n/a	90	180
1.50	921.50	n/a	690	870
2.00	922.00	n/a	1,194	2,064

Culvert / Orifice Structures Weir Structures [A] [B] [C] [D] [A] [B] [C] [PrfRsr] Rise (in) = 0.000.00 0.00 0.00 Crest Len (ft) = 18.00 0.00 0.00 0.00 Span (in) = 0.00 0.00 0.00 0.00 Crest El. (ft) = 921.50 0.00 0.00 0.00 No. Barrels = 0 0 Weir Coeff. = 3.333.33 3.33 3.33 = 0.000.00 0.00 0.00 Weir Type = Rect Invert El. (ft) = 0.000.00 0.00 0.00 Multi-Stage Length (ft) = No No No No Slope (%) = 0.000.00 0.00 n/a N-Value = .013 .013 .013 n/a 0.60 0.60 = 0.600.60 Exfil.(in/hr) = 0.000 (by Wet area) Orifice Coeff. Multi-Stage = n/aNo No No TW Elev. (ft) = 0.00

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	1	I Tryuranow Tryurograph's Extension for Autocade of							T
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	3.188	1	721	7,959				DA-FR-118C PRE
2	SCS Runoff	3.188	1	721	7,959				DA-FR-118C POST
3	SCS Runoff	2.817	1	721	7,158				DA-FR-118C Pre-Forested
4	Reservoir	3.087	1	722	7,089	2	921.64	1,199	WB Soil Amendment
DA	-FR-118c.gpv	N	1	1	Return	Period: 2 Y	ear	Wednesda	ıy, 08 / 16 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

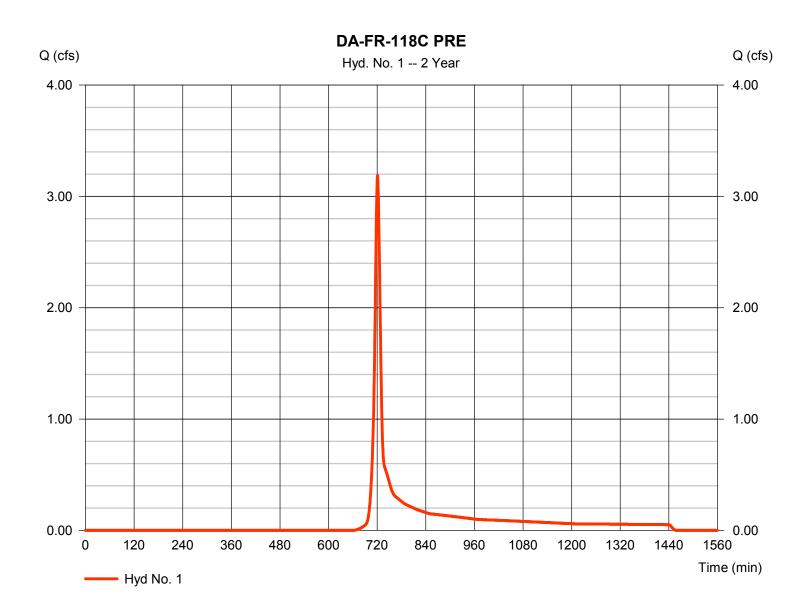
Wednesday, 08 / 16 / 2017

Hyd. No. 1

DA-FR-118C PRE

Hydrograph type = SCS Runoff Peak discharge = 3.188 cfsStorm frequency = 2 yrsTime to peak = 721 min Time interval = 1 min Hyd. volume = 7,959 cuftCurve number Drainage area = 1.900 ac= 70* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 11.40 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = $[(0.029 \times 85) + (0.105 \times 89) + (0.068 \times 58) + (0.751 \times 71) + (0.179 \times 55) + (0.770 \times 70)] / 1.900$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

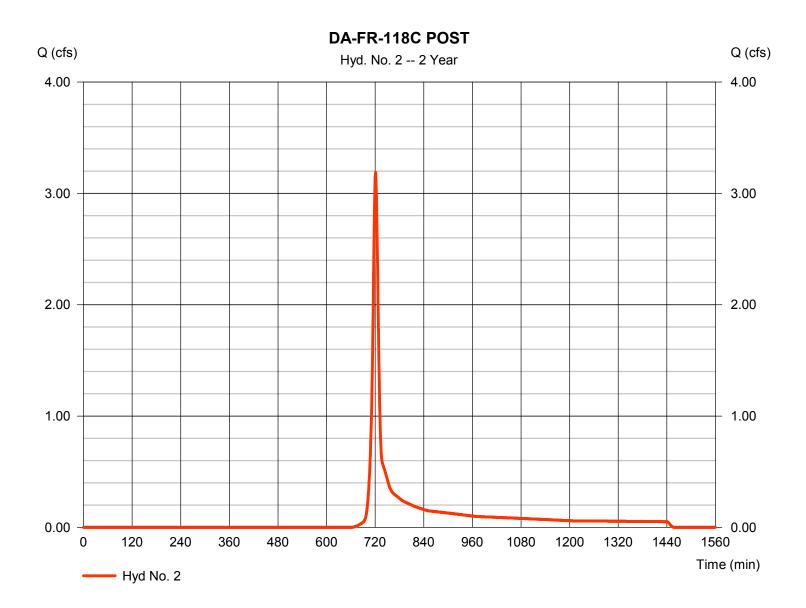
Wednesday, 08 / 16 / 2017

Hyd. No. 2

DA-FR-118C POST

Hydrograph type = SCS Runoff Peak discharge = 3.188 cfsStorm frequency = 2 yrsTime to peak = 721 min Time interval = 1 min Hyd. volume = 7,959 cuftCurve number Drainage area = 1.900 ac= 70* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 11.50 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(1.900 x 70)] / 1.900



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

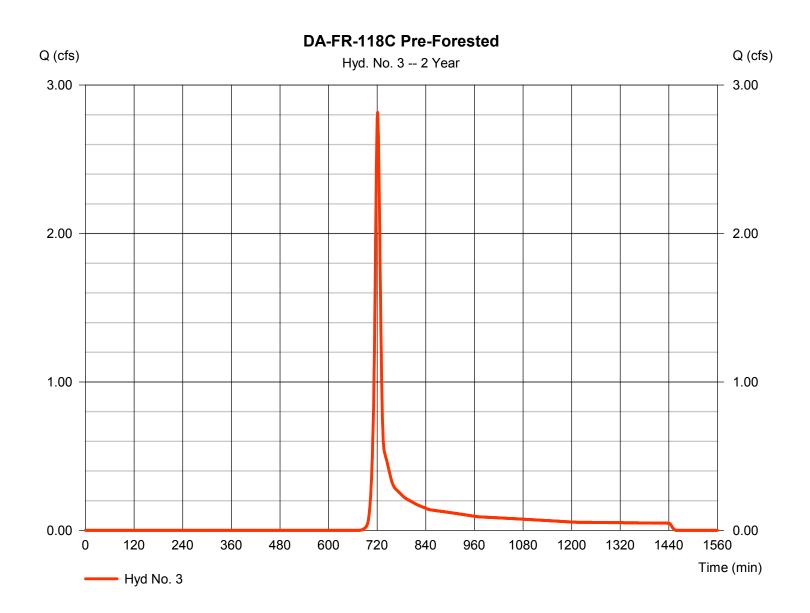
Wednesday, 08 / 16 / 2017

Hyd. No. 3

DA-FR-118C Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 2.817 cfsStorm frequency = 2 yrsTime to peak = 721 min Time interval = 1 min Hyd. volume = 7,158 cuftCurve number Drainage area = 1.900 ac= 68* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 11.40 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.280 x 55) + (1.620 x 70)] / 1.900



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

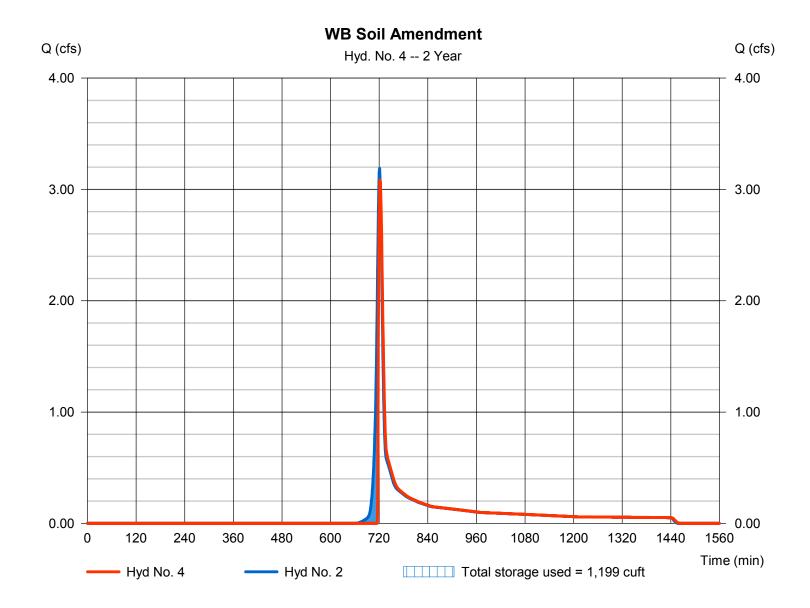
Wednesday, 08 / 16 / 2017

Hyd. No. 4

WB Soil Amendment

Hydrograph type Peak discharge = 3.087 cfs= Reservoir Storm frequency = 2 yrs Time to peak = 722 min Time interval = 1 min Hyd. volume = 7,089 cuftInflow hyd. No. = 2 - DA-FR-118C POST Max. Elevation = 921.64 ftReservoir name = Waterbar Soil Amendment Max. Storage = 1,199 cuft

Storage Indication method used.



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

			_			nyuranow n	yurograpiis Exte	IISIOII IOI AULOCA		
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	7.444	1	720	18,036				DA-FR-118C PRE	
2	SCS Runoff	7.444	1	720	18,036				DA-FR-118C POST	
3	SCS Runoff	6.916	1	721	16,797				DA-FR-118C Pre-Forested	
4	Reservoir	7.366	1	721	17,166	2	921.75	1,460	WB Soil Amendment	
	FD 440							Wester		
DA-	-FR-118c.gpw	V			Return F	Period: 10 Y	'ear	Wednesday, 08 / 16 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

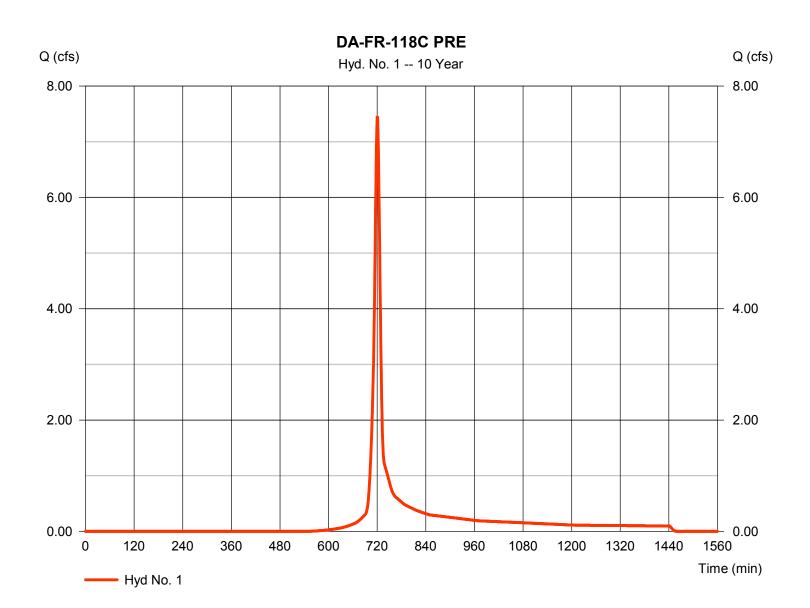
Wednesday, 08 / 16 / 2017

Hyd. No. 1

DA-FR-118C PRE

Hydrograph type = SCS Runoff Peak discharge = 7.444 cfsStorm frequency = 10 yrsTime to peak = 720 min Time interval = 1 min Hyd. volume = 18.036 cuft = 70* Curve number Drainage area = 1.900 acBasin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 11.40 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.029 \times 85) + (0.105 \times 89) + (0.068 \times 58) + (0.751 \times 71) + (0.179 \times 55) + (0.770 \times 70)] / 1.900$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

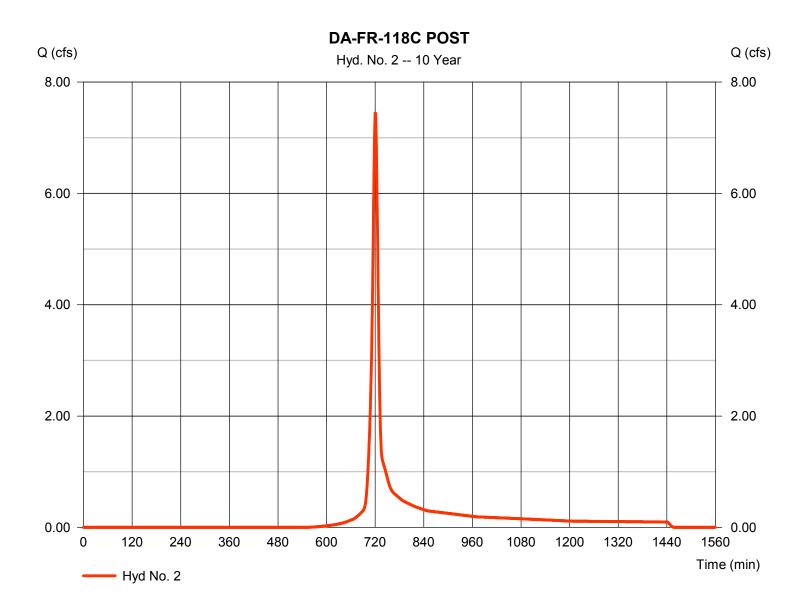
Wednesday, 08 / 16 / 2017

Hyd. No. 2

DA-FR-118C POST

Hydrograph type = SCS Runoff Peak discharge = 7.444 cfsStorm frequency = 10 yrsTime to peak = 720 min Time interval = 1 min Hyd. volume = 18.036 cuft Curve number Drainage area = 1.900 ac= 70* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 11.50 min = TR55 Total precip. Distribution = Type II = 5.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(1.900 x 70)] / 1.900



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

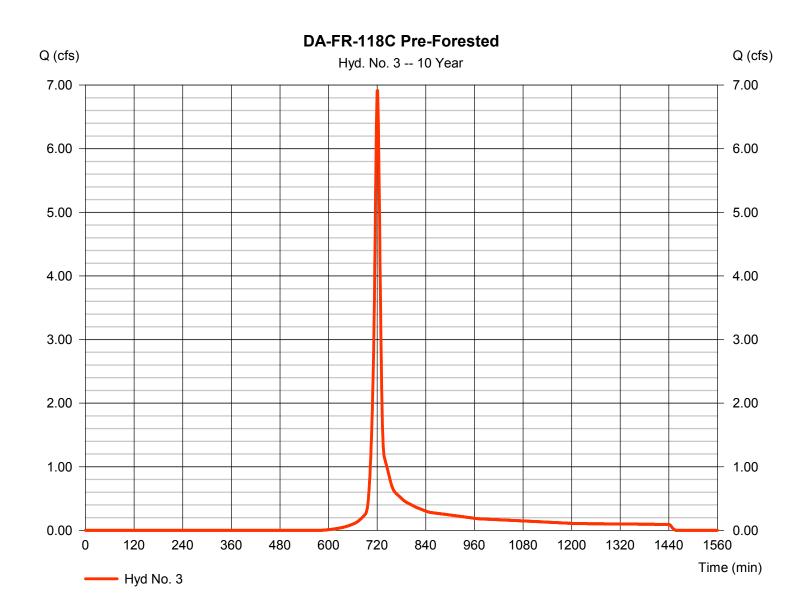
Wednesday, 08 / 16 / 2017

Hyd. No. 3

DA-FR-118C Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 6.916 cfsStorm frequency = 10 yrsTime to peak = 721 min Time interval = 1 min Hyd. volume = 16.797 cuft Curve number Drainage area = 1.900 ac= 68* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 11.40 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = [(0.280 x 55) + (1.620 x 70)] / 1.900



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

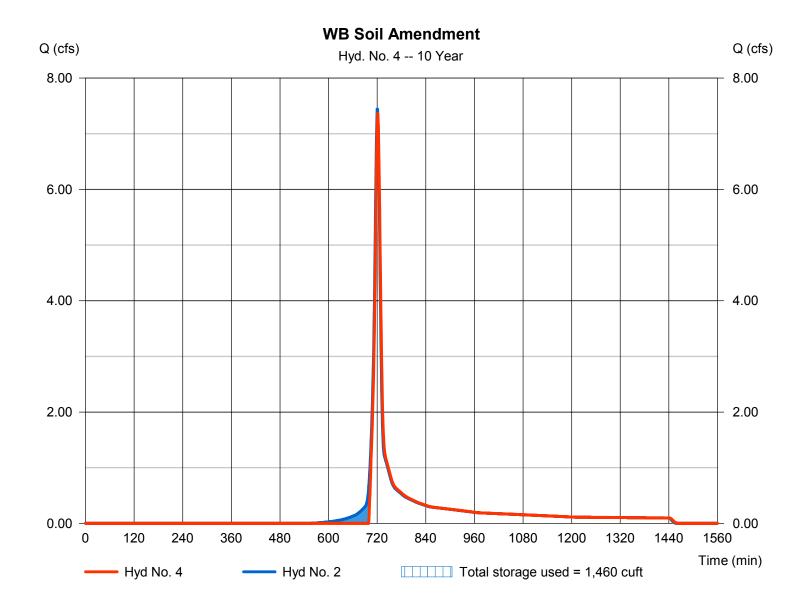
Wednesday, 08 / 16 / 2017

Hyd. No. 4

WB Soil Amendment

Hydrograph type Peak discharge = 7.366 cfs= Reservoir Storm frequency = 10 yrsTime to peak = 721 min Time interval = 1 min Hyd. volume = 17,166 cuft Inflow hyd. No. Max. Elevation = 2 - DA-FR-118C POST $= 921.75 \, \text{ft}$ Reservoir name = Waterbar Soil Amendment Max. Storage = 1,460 cuft

Storage Indication method used.



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 16 / 2017

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)							
(Yrs)	В	D	E	(N/A)				
1	0.0000	0.0000	0.0000					
2	69.8703	13.1000	0.8658					
3	0.0000	0.0000	0.0000					
5	79.2597	14.6000	0.8369					
10	88.2351	15.5000	0.8279					
25	102.6072	16.5000	0.8217					
50	114.8193	17.2000	0.8199					
100	127.1596	17.8000	0.8186					

File name: SampleFHA.idf

Intensity = B / (Tc + D)^E

Return	Intensity Values (in/hr)											
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

Precip. file name: P:\MVP ESC&SWM Review\Engineering\SW Calculations\Franklin County.pcp

		Rainfall Precipitation Table (in)								
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr		
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00		
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		



ENERGY BALANCE METHOD

Inputs:

	1-Y	r Event
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)
Pre-Developed Condition	3.890	12967
Developed Condition	3.890	12967
Pre-Developed (Forest) Condition	3.134	10931

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF =

<u>Calculations:</u>	Check #1:	$Q_{developed} \! \leq \! IF \times \left[\left(Q_{pre\ developed} \times RV_{pre\ developed} \right) / RV_{developed} \right] - \!\!\!\! - \!$	Q (cfs) 3.890	≤ OK	Q (cfs) 3.890
	Check #2:	Q _{developed} ≤ Q _{pre-developed}	3.890	≤ OK	3.890
	Check #3:	$Q_{developed}$ shall not be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed}$ >	3.890	shall not be required to be ≤	2.642

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 - 0.40
Light Turf	0.20
Dense Turf	0.17 - 0.80
Dense Grass	0.17 - 0.30
Bermuda Grass	0.30 - 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	·
Short Grass Prairie	0.10 - 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 - 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	·
Typical	0.13
No Debris Cover	0.09 - 0.34
20% Debris Cover	0.05 – 0.25
Woods:	<u>.</u>
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

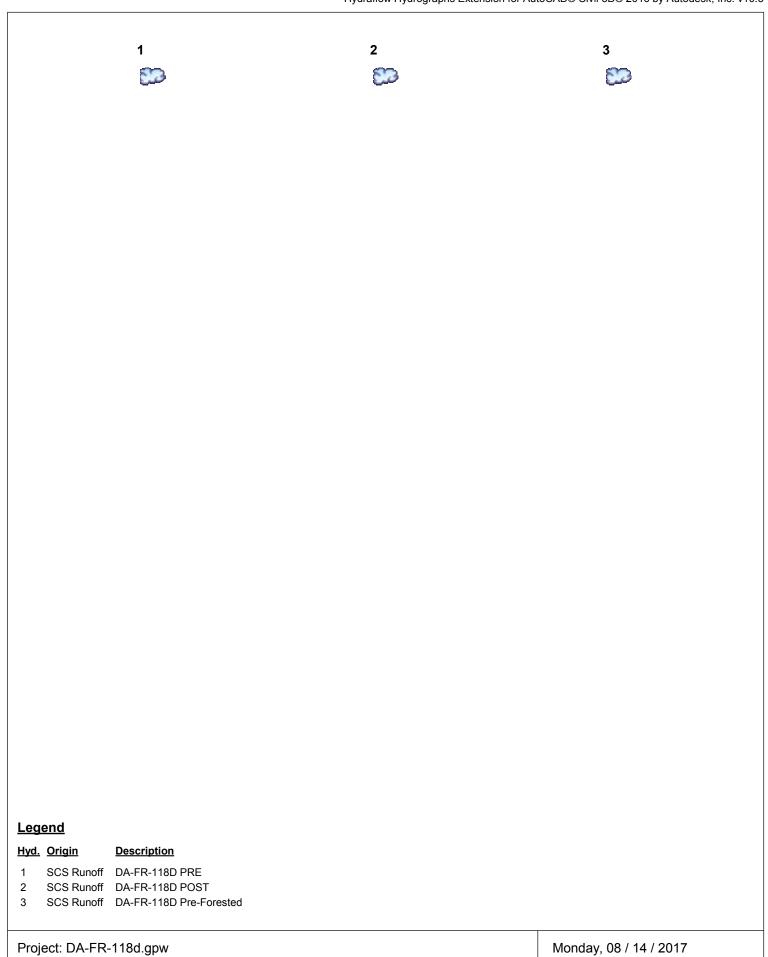
-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 14 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-118D PRE	
TR-55 Tc Worksheet	
Hydrograph No. 2, SCS Runoff, DA-FR-118D POST	6
TR-55 Tc Worksheet	
Hydrograph No. 3, SCS Runoff, DA-FR-118D Pre-Forested	
TR-55 Tc Worksheet	9
2 - Year	
Summary Report	10
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-118D PRE	11
Hydrograph No. 2, SCS Runoff, DA-FR-118D POST	
Hydrograph No. 3, SCS Runoff, DA-FR-118D Pre-Forested	
10 - Year	
Summary Report	14
Hydrograph Reports	15
Hydrograph No. 1, SCS Runoff, DA-FR-118D PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-118D POST	
Hydrograph No. 3, SCS Runoff, DA-FR-118D Pre-Forested	
IDF Report	18



Hydrograph Return Period Recap Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

lyd.	Hydrograph								Hydrograph				
о.	type (origin)	hyd(s)	1-yr 2-yr		3-yr 5-yr		10-yr 25-yr		50-yr	100-yr	Description		
1	SCS Runoff		3.890	5.029			11.46				DA-FR-118D PRE		
2	SCS Runoff		3.890	5.029			11.46				DA-FR-118D POST		
3	SCS Runoff		3.134	4.186			10.26				DA-FR-118D Pre-Forested		

Proj. file: DA-FR-118d.gpw

Monday, 08 / 14 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	Hydrograph	Peak	Time	Time to	Hyd.	Inflow	Maximum	Total	Hydrograph		
No.	type (origin)	flow (cfs)	interval (min)	Peak (min)	volume (cuft)	hyd(s)	elevation (ft)	strge used (cuft)	Description		
1	SCS Runoff	3.890	1	726	12,967				DA-FR-118D PRE		
2	SCS Runoff	3.890	1	726	12,967				DA-FR-118D POST		
3	SCS Runoff	3.134	1	726	10,931				DA-FR-118D Pre-Forested		
DA	DA-FR-118d.gpw				Return F	Period: 1 Ye	ear	Monday, 08 / 14 / 2017			

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

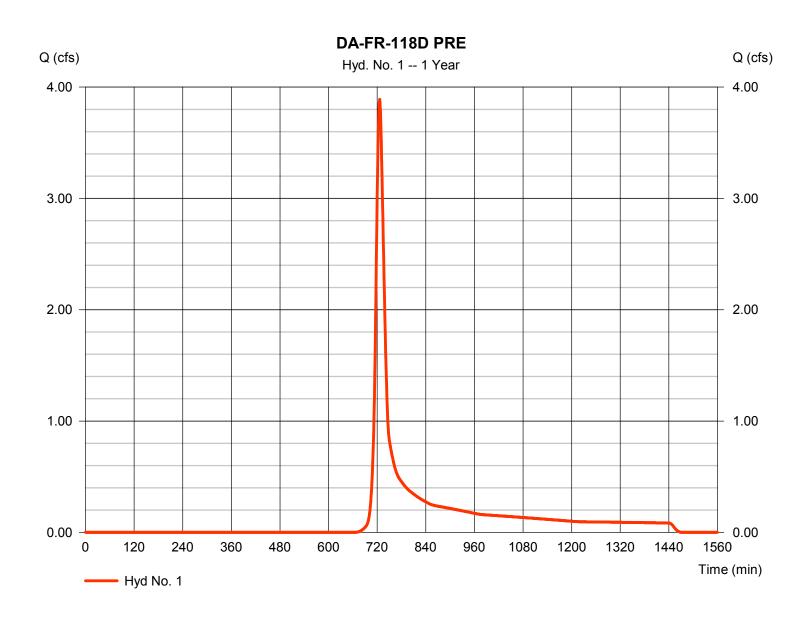
Monday, 08 / 14 / 2017

Hyd. No. 1

DA-FR-118D PRE

Hydrograph type = SCS Runoff Peak discharge = 3.890 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 min Hyd. volume = 12.967 cuft Curve number Drainage area = 3.600 ac= 72* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 20.00 min = TR55 Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.001 \times 85) + (0.318 \times 89) + (0.211 \times 58) + (3.067 \times 71)] / 3.600$



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1DA-FR-118D PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>c</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 3.10		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00			
Travel Time (min)	= 16.76	+	0.00	+	0.00	=	16.76	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 602.20 = 3.60 = Unpaved =3.06	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00			
Travel Time (min)	= 3.28	+	0.00	+	0.00	=	3.28	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.040 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015			
Flow length (ft)	({0})0.0		0.0		0.0			
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00	
Total Travel Time, Tc								

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

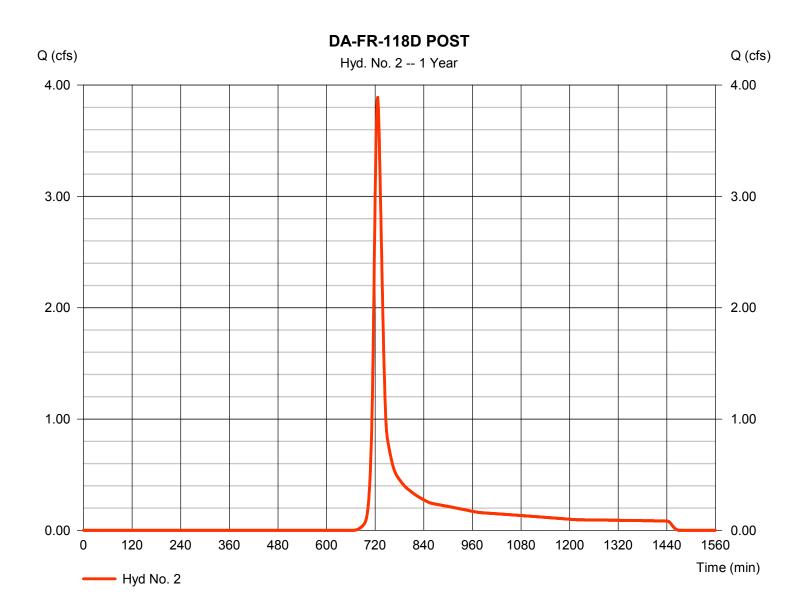
Monday, 08 / 14 / 2017

Hyd. No. 2

DA-FR-118D POST

Hydrograph type = SCS Runoff Peak discharge = 3.890 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 min Hyd. volume = 12.967 cuft Curve number Drainage area = 3.600 ac= 72* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 20.00 min = TR55 Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $+(0.320 \times 89) + (0.210 \times 58) + (3.070 \times 71)$] / 3.600



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2DA-FR-118D POST

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 3.10 = 16.76	+	0.800 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	=	16.76
, ,							
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 602.20 = 3.60 = Unpaved =3.06	t	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 3.28	+	0.00	+	0.00	=	3.28
Channel Flow							
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.040 =0.00		0.00 0.00 0.00 0.015		0.00 0.00 0.00 0.015		
					0.00		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							20.00 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

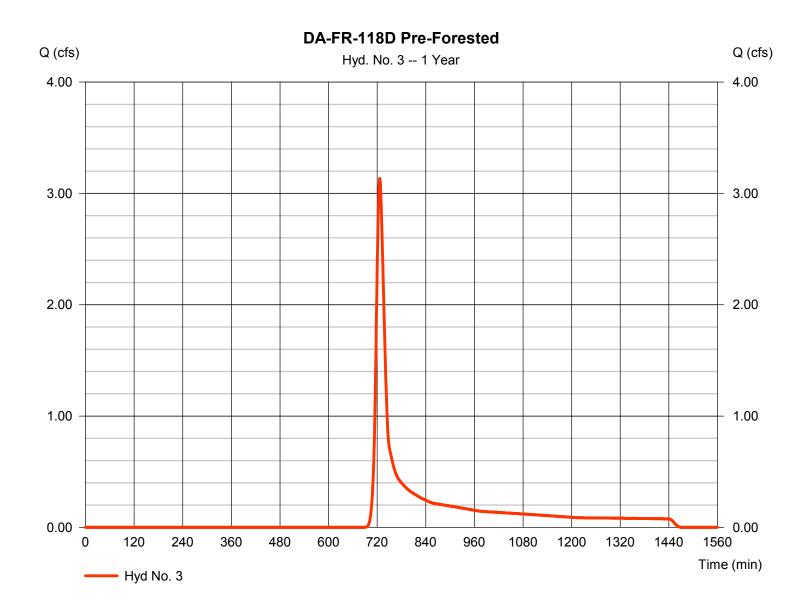
Monday, 08 / 14 / 2017

Hyd. No. 3

DA-FR-118D Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 3.134 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 min Hyd. volume = 10.931 cuft Drainage area Curve number = 3.600 ac= 69* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 20.00 min = TR55 Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.212 x 55) + (3.385 x 70)] / 3.600



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3DA-FR-118D Pre-Forested

<u>Description</u>	<u>A</u>		<u>B</u>		<u>c</u>		<u>Totals</u>		
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 3.10		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00				
Travel Time (min)	= 16.76	+	0.00	+	0.00	=	16.76		
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 602.20 = 3.60 = Unpaved =3.06	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00				
Travel Time (min)	= 3.28	+	0.00	+	0.00	=	3.28		
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.030 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015				
Flow length (ft)	({0})0.0		0.0		0.0				
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00		
Total Travel Time, Tc									

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

łyd. lo.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description		
1	SCS Runoff	5.029	1	726	16,384				DA-FR-118D PRE		
2	SCS Runoff	5.029	1	726	16,384				DA-FR-118D POST		
2 3	SCS Runoff SCS Runoff	5.029 4.186	1 1	726	16,384 14,060				DA-FR-118D Pre-Forested DA-FR-118D Pre-Forested		
DA	-FR-118d.gp\	N	•	•	Return F	Period: 2 Ye	ear	Monday, 08	Monday, 08 / 14 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

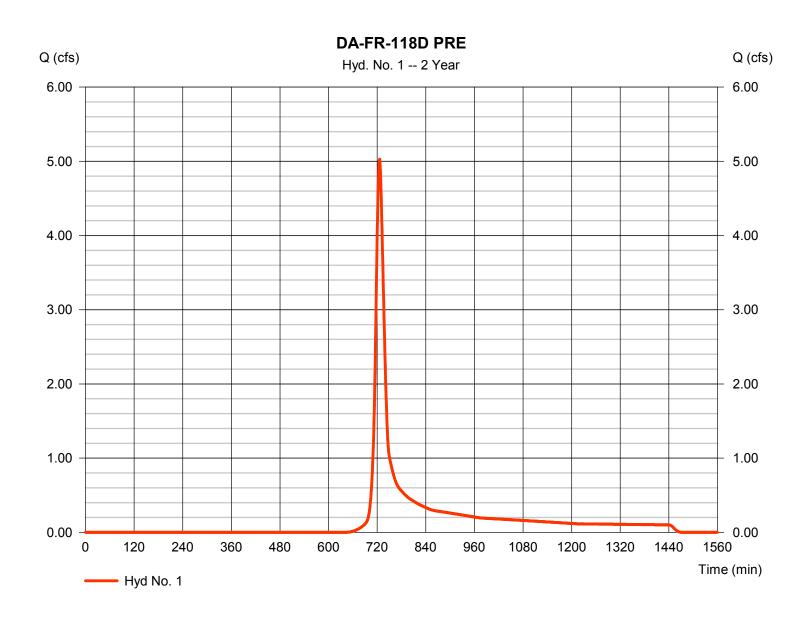
Monday, 08 / 14 / 2017

Hyd. No. 1

DA-FR-118D PRE

Hydrograph type = SCS Runoff Peak discharge = 5.029 cfsStorm frequency = 2 yrsTime to peak = 726 min Time interval = 1 min Hyd. volume = 16.384 cuft Curve number Drainage area = 3.600 ac= 72* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 20.00 min = TR55 Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = $[(0.001 \times 85) + (0.318 \times 89) + (0.211 \times 58) + (3.067 \times 71)] / 3.600$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

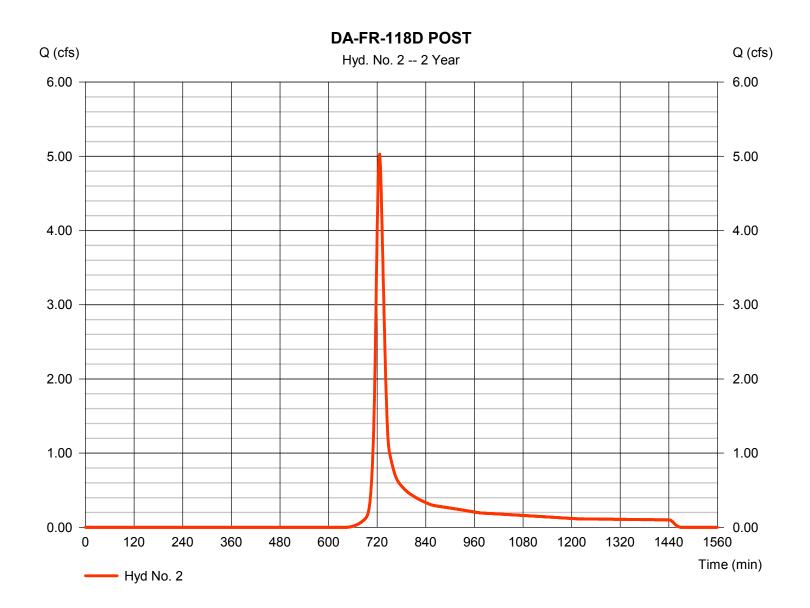
Monday, 08 / 14 / 2017

Hyd. No. 2

DA-FR-118D POST

Hydrograph type = SCS Runoff Peak discharge = 5.029 cfsStorm frequency = 2 yrsTime to peak = 726 min Time interval = 1 min Hyd. volume = 16.384 cuft Curve number Drainage area = 3.600 ac= 72* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 20.00 min = TR55 Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = $+(0.320 \times 89) + (0.210 \times 58) + (3.070 \times 71)$] / 3.600



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

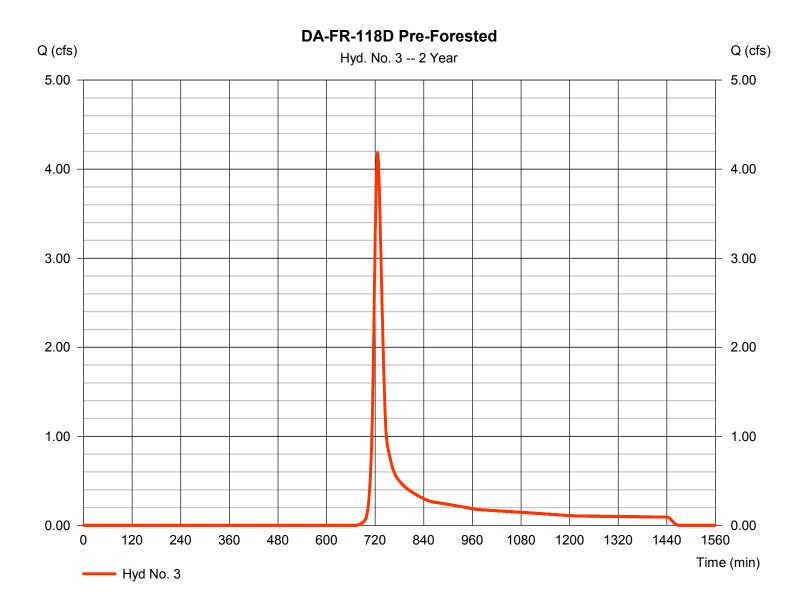
Monday, 08 / 14 / 2017

Hyd. No. 3

DA-FR-118D Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 4.186 cfsStorm frequency = 2 yrsTime to peak = 726 min Time interval = 1 min Hyd. volume = 14.060 cuftCurve number Drainage area = 3.600 ac= 69* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 20.00 min = TR55 Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = [(0.212 x 55) + (3.385 x 70)] / 3.600



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	Hydrograph	Peak	Time	Time to	Hyd.	Inflow	Maximum	Total	Hydrograph	
No.	type (origin)	flow (cfs)	interval (min)	Peak (min)	volume (cuft)	hyd(s)	elevation (ft)	strge used (cuft)	Description	
1	SCS Runoff	11.46	1	725	35,934				DA-FR-118D PRE	
2	SCS Runoff	11.46	1	725	35,934				DA-FR-118D POST	
3	SCS Runoff	10.26	1	725	32,415				DA-FR-118D Pre-Forested	
DA	DA-FR-118d.gpw					Period: 10 Y	ear	Monday, 08 / 14 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

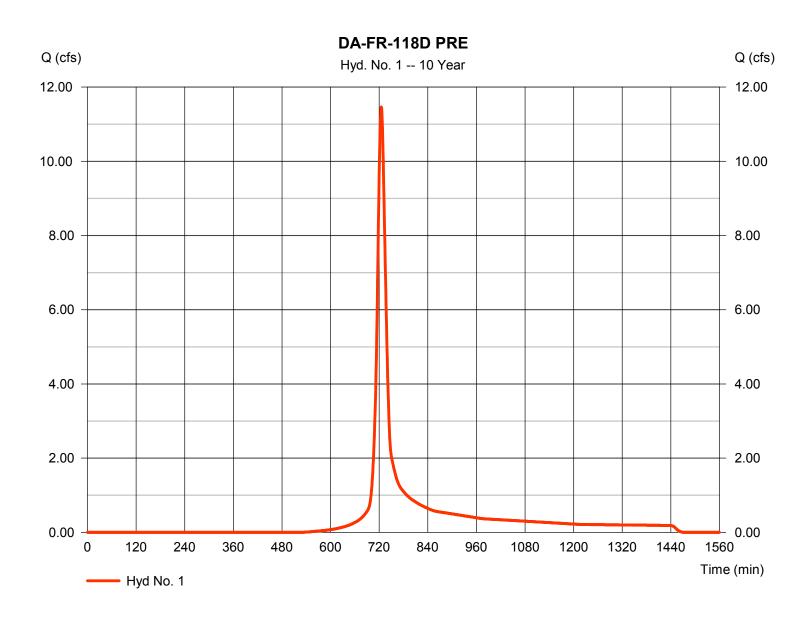
Monday, 08 / 14 / 2017

Hyd. No. 1

DA-FR-118D PRE

Hydrograph type = SCS Runoff Peak discharge = 11.46 cfsStorm frequency = 10 yrsTime to peak = 725 min Time interval = 1 min Hyd. volume = 35.934 cuft = 72* Curve number Drainage area = 3.600 acBasin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 20.00 min = TR55 Total precip. Distribution = Type II = 5.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = [(0.001 x 85) + (0.318 x 89) + (0.211 x 58) + (3.067 x 71)] / 3.600



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

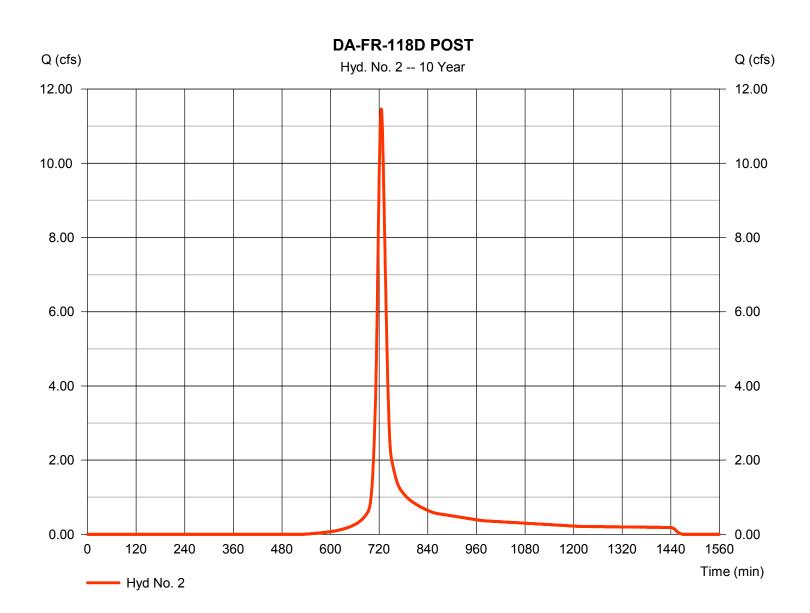
Monday, 08 / 14 / 2017

Hyd. No. 2

DA-FR-118D POST

Hydrograph type = SCS Runoff Peak discharge = 11.46 cfsStorm frequency = 10 yrsTime to peak = 725 min Time interval = 1 min Hyd. volume = 35.934 cuft = 72* Curve number Drainage area = 3.600 acBasin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 20.00 min = TR55 Total precip. Distribution = Type II = 5.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = $+(0.320 \times 89) + (0.210 \times 58) + (3.070 \times 71)$] / 3.600



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

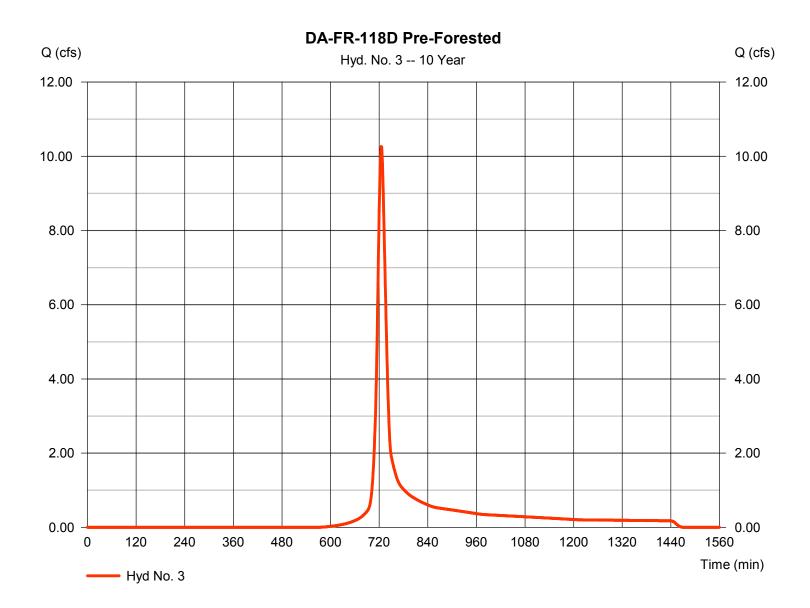
Monday, 08 / 14 / 2017

Hyd. No. 3

DA-FR-118D Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 10.26 cfsStorm frequency = 10 yrsTime to peak = 725 min Time interval = 1 min Hyd. volume = 32.415 cuft Drainage area Curve number = 3.600 ac= 69* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 20.00 min = TR55 Total precip. Distribution = Type II = 5.70 inStorm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.212 x 55) + (3.385 x 70)] / 3.600



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 14 / 2017

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)								
(Yrs)	В	D	E	(N/A)					
1	0.0000	0.0000	0.0000						
2	69.8703	13.1000	0.8658						
3	0.0000	0.0000	0.0000						
5	79.2597	14.6000	0.8369						
10	88.2351	15.5000	0.8279						
25	102.6072	16.5000	0.8217						
50	114.8193	17.2000	0.8199						
100	127.1596	17.8000	0.8186						

File name: SampleFHA.idf

Intensity = B / (Tc + D)^E

Return					Intens	sity Values	(in/hr)					
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

Precip. file name: P:\MVP ESC&SWM Review\Engineering\SW Calculations\Franklin County.pcp

		Rainfall Precipitation Table (in)											
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr					
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00					
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					



ENERGY BALANCE METHOD

In		

	1-	-Yr Event
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)
Pre-Developed Condition	0.491	1771
Developed Condition	0.491	1771
Pre-Developed (Forest) Condition	0.398	1500

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF =

<u>Calculations:</u>	Check #1:	$Q_{developed} \le IF \times [(Q_{gre-developed} \times RV_{gre-developed}) / RV_{developed}]$	Q (cfs) 0.491	≤ OK	Q (cfs) 0.491
	Check #2:	Q _{developed} ≤ Q _{pre-developed} >	0.491	≤ OK	0.491
	Check #3:	$Q_{developed}$ shall not be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed} \longrightarrow$	0.491	shall not be required to be ≤	0.337

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 - 0.40
Light Turf	0.20
Dense Turf	0.17 - 0.80
Dense Grass	0.17 - 0.30
Bermuda Grass	0.30 - 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	·
Short Grass Prairie	0.10 - 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 - 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	·
Typical	0.13
No Debris Cover	0.09 - 0.34
20% Debris Cover	0.05 – 0.25
Woods:	<u>.</u>
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

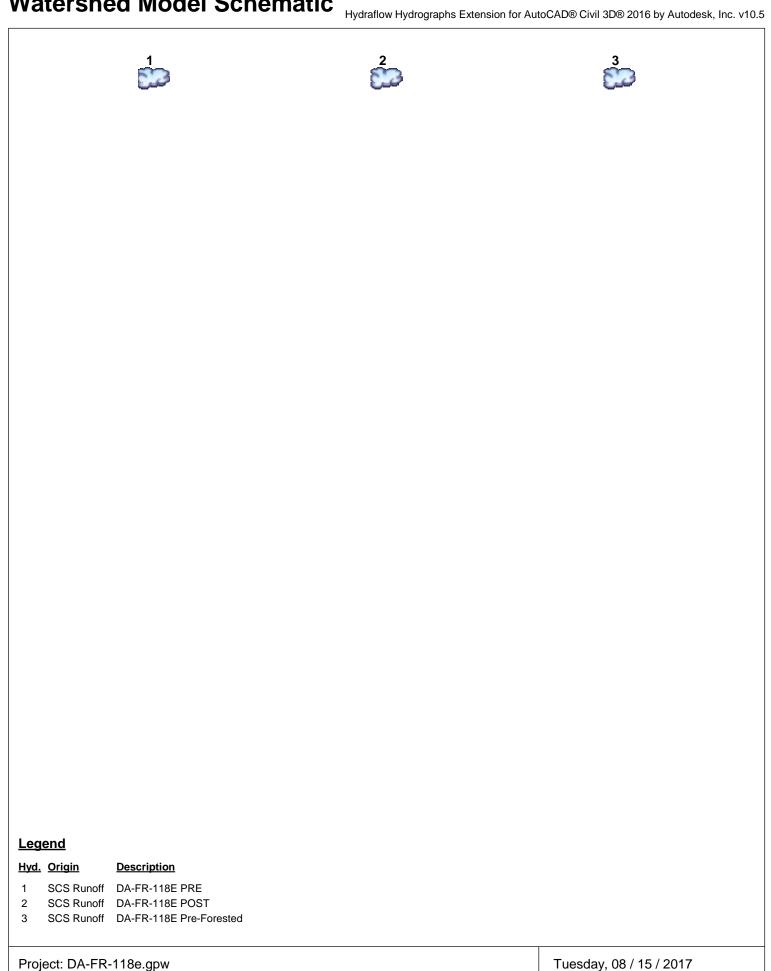
-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Tuesday, 08 / 15 / 2017

Watershed Model Schematic	. 1
Hydrograph Return Period Recap	. 2
1 - Year	
Summary Report	
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-118E PRE	
TR-55 Tc Worksheet	
Hydrograph No. 2, SCS Runoff, DA-FR-118E POST	
TR-55 Tc Worksheet	
Hydrograph No. 3, SCS Runoff, DA-FR-118E Pre-Forested	
TR-55 Tc Worksheet	9
2 - Year	
Summary Report	10
Hydrograph Reports	11
Hydrograph No. 1, SCS Runoff, DA-FR-118E PRE	11
Hydrograph No. 2, SCS Runoff, DA-FR-118E POST	12
Hydrograph No. 3, SCS Runoff, DA-FR-118E Pre-Forested	13
10 - Year	
Summary Report	14
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-118E PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-118E POST	
Hydrograph No. 3, SCS Runoff, DA-FR-118E Pre-Forested	
IDE Penort	1Ω



Hydrograph Return Period Recap Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	Hydrograph	Inflow	Peak Outflow (cfs)						Hydrograph		
No.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		0.491	0.630			1.411				DA-FR-118E PRE
2	SCS Runoff		0.491	0.630			1.411				DA-FR-118E POST
3	SCS Runoff		0.398	0.527			1.266				DA-FR-118E Pre-Forested

Proj. file: DA-FR-118e.gpw

Tuesday, 08 / 15 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

								1.0.0	
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.491	1	728	1,771				DA-FR-118E PRE
2	SCS Runoff	0.491	1	728	1,771				DA-FR-118E POST
3	SCS Runoff	0.398	1	728	1,500				DA-FR-118E Pre-Forested
DA-	-FR-118e.gpv	W	-1	1	Return F	Period: 1 Ye	ear	Tuesday, 0	8 / 15 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

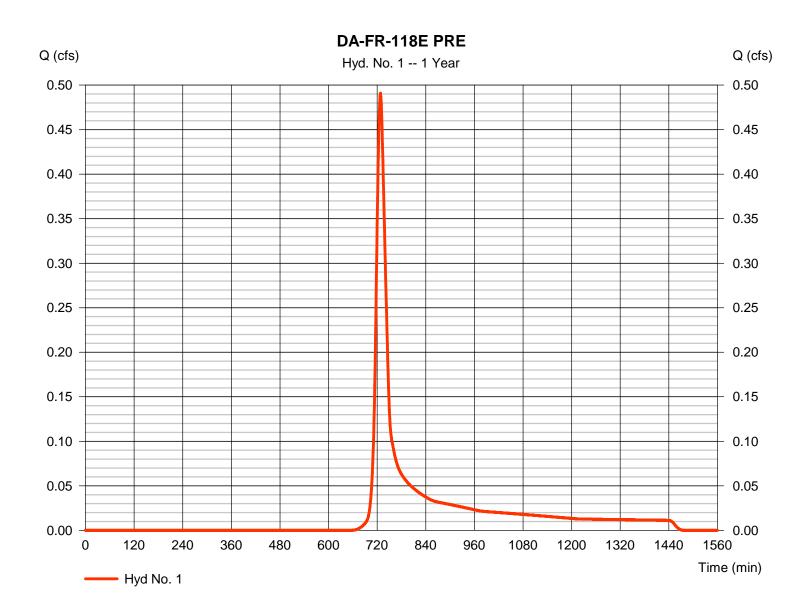
Tuesday, 08 / 15 / 2017

Hyd. No. 1

DA-FR-118E PRE

Hydrograph type = SCS Runoff Peak discharge = 0.491 cfsStorm frequency Time to peak = 728 min = 1 yrsTime interval = 1 min Hyd. volume = 1.771 cuftDrainage area Curve number = 0.470 ac= 73*Basin Slope = 0.0 % Hydraulic length = 0 ftTc method Time of conc. (Tc) = 23.30 min = TR55 Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.050 \times 89) + (0.420 \times 71)] / 0.470$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1

DA-FR-118E PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 1.40	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	_	23.03
Traver Time (IIIII)	- 25.05	т	0.00	т	0.00	_	23.03
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 52.80 = 3.60 = Unpaved =3.06	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.29	+	0.00	+	0.00	=	0.29
Channel Flow							
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.040 =0.00		0.00 0.00 0.00 0.015		0.00 0.00 0.00 0.015		
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value	= 0.00 = 0.00 = 0.040		0.00 0.00 0.015		0.00 0.00		
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value	= 0.00 = 0.00 = 0.040		0.00 0.00 0.015		0.00 0.00 0.015		
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.040 =0.00	+	0.00 0.00 0.015 0.00	+	0.00 0.00 0.015 0.00	=	0.00

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

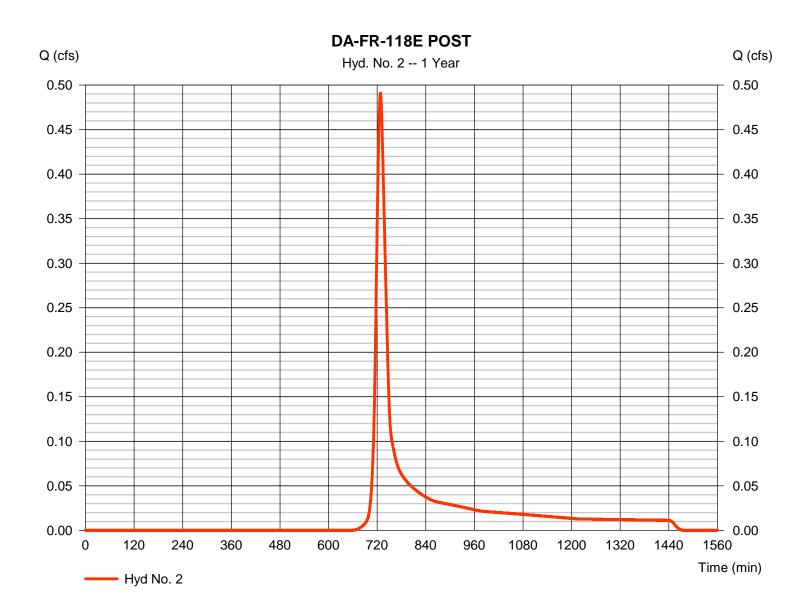
Tuesday, 08 / 15 / 2017

Hyd. No. 2

DA-FR-118E POST

Hydrograph type = SCS Runoff Peak discharge = 0.491 cfsStorm frequency Time to peak = 728 min = 1 yrsTime interval = 1 min Hyd. volume = 1.771 cuftDrainage area Curve number = 0.470 ac= 73*Basin Slope = 0.0 % Hydraulic length = 0 ftTc method Time of conc. (Tc) = 23.30 min = TR55 Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.050 \times 89) + (0.420 \times 71)] / 0.470$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2DA-FR-118E POST

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 1.40 = 23.03	+	0.800 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	_	23.03
Traver Time (IIIII)	- 20.00	•	0.00	•	0.00	_	20.00
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 52.80 = 3.60 = Unpaved =3.06	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.29	+	0.00	+	0.00	=	0.29
• •							
Channel Flow							
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.040 =0.00		0.00 0.00 0.00 0.015		0.00 0.00 0.00 0.015		
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value	= 0.00 $= 0.00$ $= 0.040$		0.00 0.00 0.00 0.015		0.00 0.00		
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value	= 0.00 $= 0.00$ $= 0.040$		0.00 0.00 0.00 0.015		0.00 0.00 0.015		
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.040 =0.00	+	0.00 0.00 0.00 0.015 0.00	+	0.00 0.00 0.015 0.00	=	0.00

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

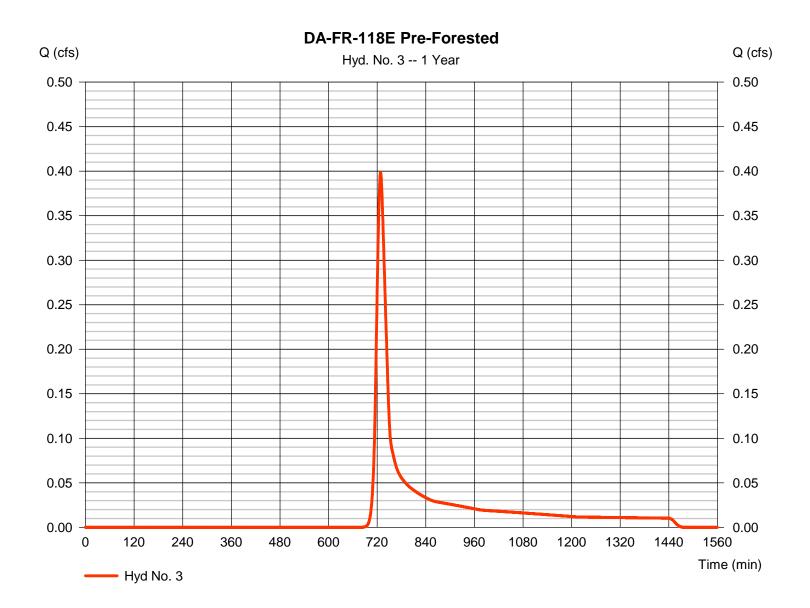
Tuesday, 08 / 15 / 2017

Hyd. No. 3

DA-FR-118E Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 0.398 cfsStorm frequency Time to peak = 728 min = 1 yrsTime interval = 1 min Hyd. volume = 1,500 cuftDrainage area Curve number = 0.470 ac= 70*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 23.30 \, \text{min}$ = TR55 Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.470 x 70)] / 0.470



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3DA-FR-118E Pre-Forested

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 1.40	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	_	23.03
, ,		-	0.00		0.00		
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 52.80 = 3.60 = Unpaved =3.06	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.29	+	0.00	+	0.00	=	0.29
Channel Flow							
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.030 =0.00		0.00 0.00 0.00 0.015		0.00 0.00 0.00 0.015		
					0.00		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							23.30 min

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

			_		_				
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.630	1	728	2,225				DA-FR-118E PRE
2	SCS Runoff	0.630	1	728	2,225				DA-FR-118E POST
3	SCS Runoff	0.527	1	728	1,917				DA-FR-118E Pre-Forested
DA:	 -FR-118e.gp\	N			Return F	eriod: 2 Ye	 ear	Tuesday, 0	 8 / 15 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

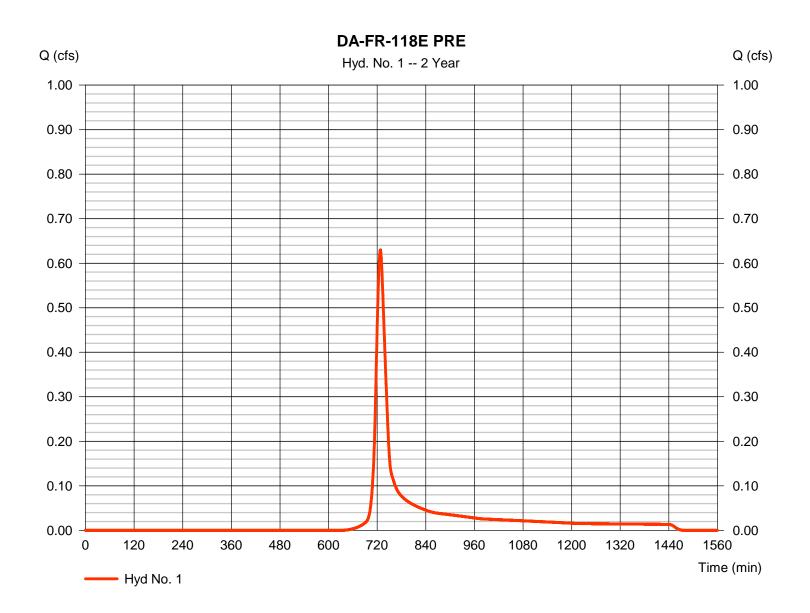
Tuesday, 08 / 15 / 2017

Hyd. No. 1

DA-FR-118E PRE

Hydrograph type = SCS Runoff Peak discharge = 0.630 cfsStorm frequency Time to peak = 728 min = 2 yrsTime interval = 1 min Hyd. volume = 2.225 cuft Drainage area Curve number = 0.470 ac= 73*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 23.30 min = TR55 Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.050 \times 89) + (0.420 \times 71)] / 0.470$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

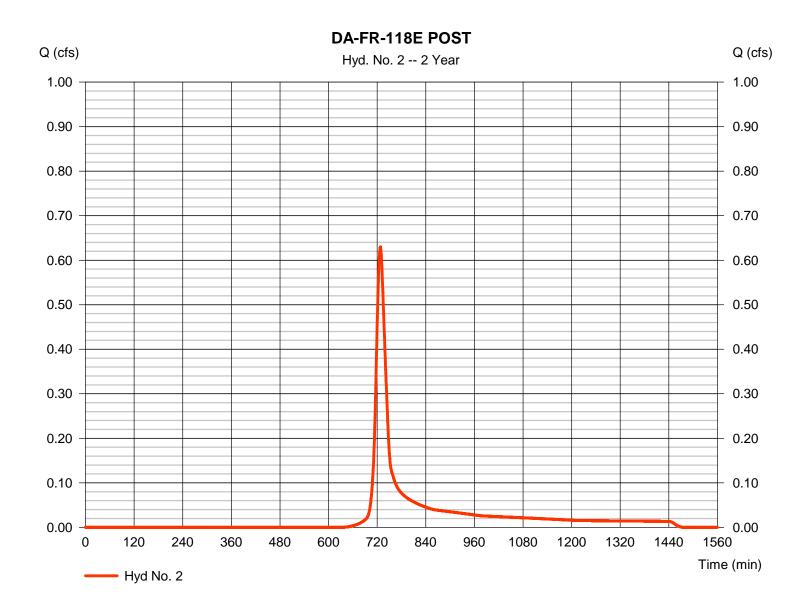
Tuesday, 08 / 15 / 2017

Hyd. No. 2

DA-FR-118E POST

Hydrograph type = SCS Runoff Peak discharge = 0.630 cfsStorm frequency = 2 yrsTime to peak = 728 min Time interval = 1 min Hyd. volume = 2.225 cuft Drainage area Curve number = 0.470 ac= 73*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 23.30 min = TR55 Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.050 \times 89) + (0.420 \times 71)] / 0.470$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

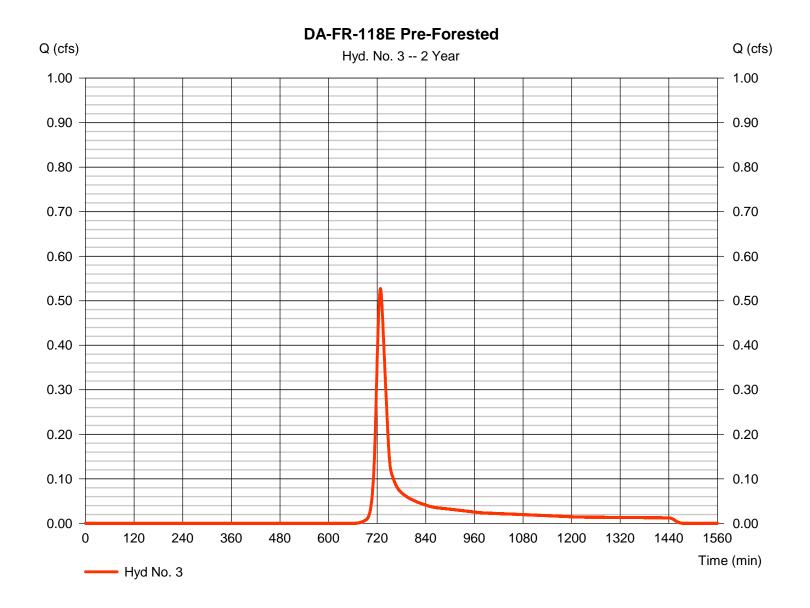
Tuesday, 08 / 15 / 2017

Hyd. No. 3

DA-FR-118E Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 0.527 cfsStorm frequency = 2 yrsTime to peak = 728 min Time interval = 1 min Hyd. volume = 1,917 cuftDrainage area Curve number = 0.470 ac= 70*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 23.30 min = TR55 Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.470 x 70)] / 0.470



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

								1	T
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	1.411	1	727	4,805				DA-FR-118E PRE
2	SCS Runoff	1.411	1	727	4,805				DA-FR-118E POST
3	SCS Runoff	1.266	1	727	4,344				DA-FR-118E Pre-Forested
DA	-FR-118e.gp	W			Return F	Period: 10 Y	'ear	Tuesday, 0	8 / 15 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

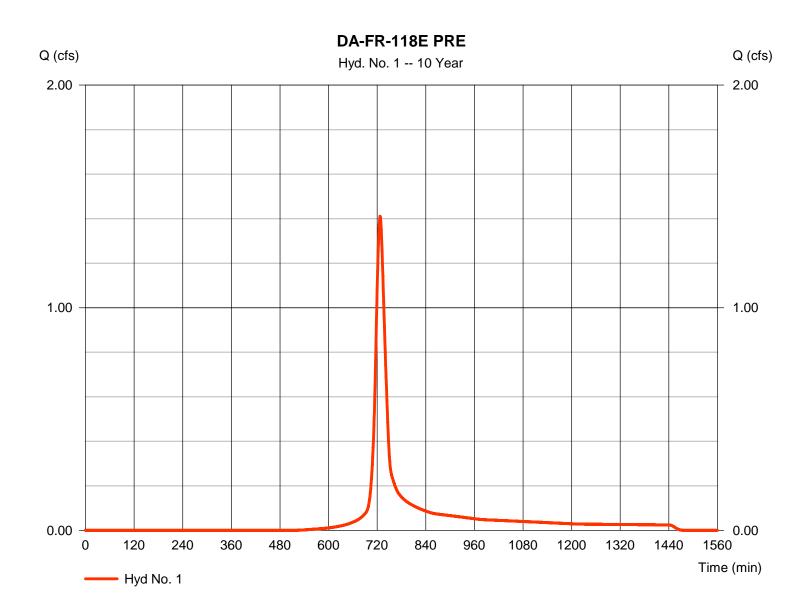
Tuesday, 08 / 15 / 2017

Hyd. No. 1

DA-FR-118E PRE

Hydrograph type = SCS Runoff Peak discharge = 1.411 cfsStorm frequency = 10 yrsTime to peak = 727 min Time interval = 1 minHyd. volume = 4.805 cuftDrainage area Curve number = 0.470 ac= 73*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 23.30 \, \text{min}$ = TR55 Total precip. = 5.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.050 \times 89) + (0.420 \times 71)] / 0.470$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

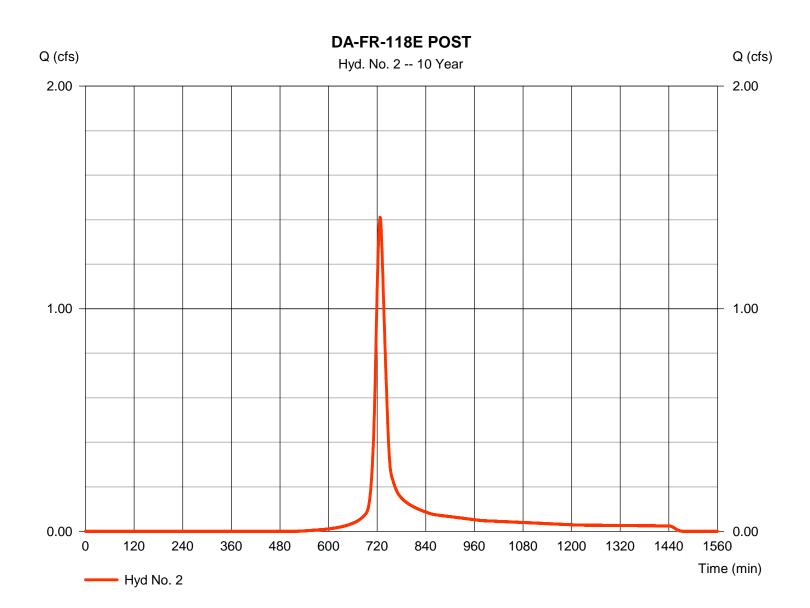
Tuesday, 08 / 15 / 2017

Hyd. No. 2

DA-FR-118E POST

Hydrograph type = SCS Runoff Peak discharge = 1.411 cfsStorm frequency = 10 yrsTime to peak = 727 min Time interval = 1 minHyd. volume = 4.805 cuftDrainage area Curve number = 0.470 ac= 73*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 23.30 \, \text{min}$ = TR55 Total precip. = 5.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.050 \times 89) + (0.420 \times 71)] / 0.470$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

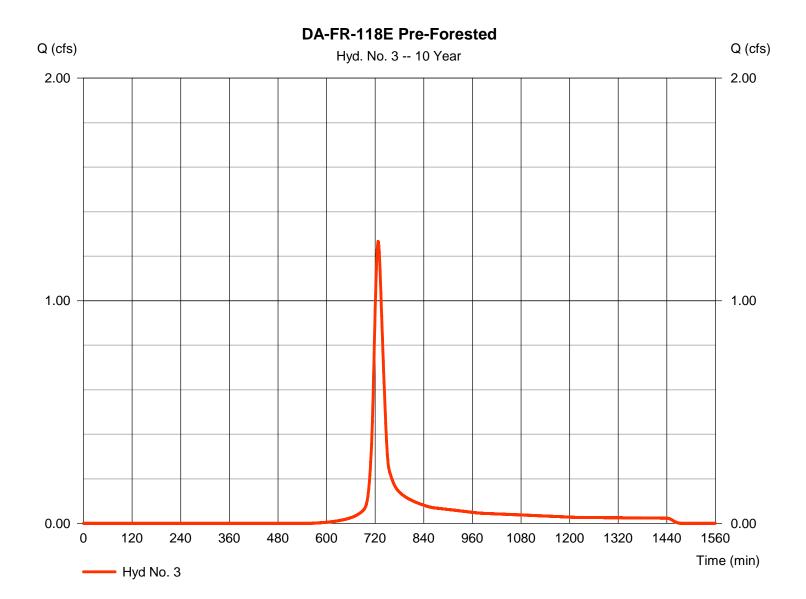
Tuesday, 08 / 15 / 2017

Hyd. No. 3

DA-FR-118E Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 1.266 cfsStorm frequency = 10 yrsTime to peak = 727 min Time interval = 1 minHyd. volume = 4.344 cuftDrainage area Curve number = 70* = 0.470 acBasin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 23.30 \, \text{min}$ = TR55 Total precip. = 5.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.470 x 70)] / 0.470



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Tuesday, 08 / 15 / 2017

Return Period	Intensity-Du	uration-Frequency E	quation Coefficients	(FHA)
(Yrs)	В	D	E	(N/A)
1	0.0000	0.0000	0.0000	
2	69.8703	13.1000	0.8658	
3	0.0000	0.0000	0.0000	
5	79.2597	14.6000	0.8369	
10	88.2351	15.5000	0.8279	
25	102.6072	16.5000	0.8217	
50	114.8193	17.2000	0.8199	
100	127.1596	17.8000	0.8186	

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return					Intens	ity Values	(in/hr)					
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

Precip. file name: P:\MVP ESC&SWM Review\Engineering\SW Calculations\Franklin County.pcp

		R	ainfall P	recipitat	ion Tab	le (in)		
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

DA-FR-119

DA-FR-119 is located in a meadow and forested areas with rolling to hilly slopes and contains agricultural land, existing dirt road and gravel road. Multiple points of analysis were evaluated within DA-FR-119 to evaluate the effects on each receiving stream/channel following construction. Specifically, DA-FR-119 was divided into four sub-drainage areas (sub-areas A to D).

Pre-construction agricultural areas will be returned to agricultural land use (i.e., returned to crop production, in identical condition) following construction. In non-agricultural areas, land use will be restored following construction as noted in the Stormwater Management (SWM) Narrative and the Annual Standards and Specifications. Agricultural areas within the limits of disturbance (LOD) are included in the SWM quality analysis and the total permanent right-of-way is analyzed via VRRM; in these calculations agricultural areas are considered "Forest/Open Space". The total phosphorus load reduction required for DA-FR-119 is -0.37 lb/yr.

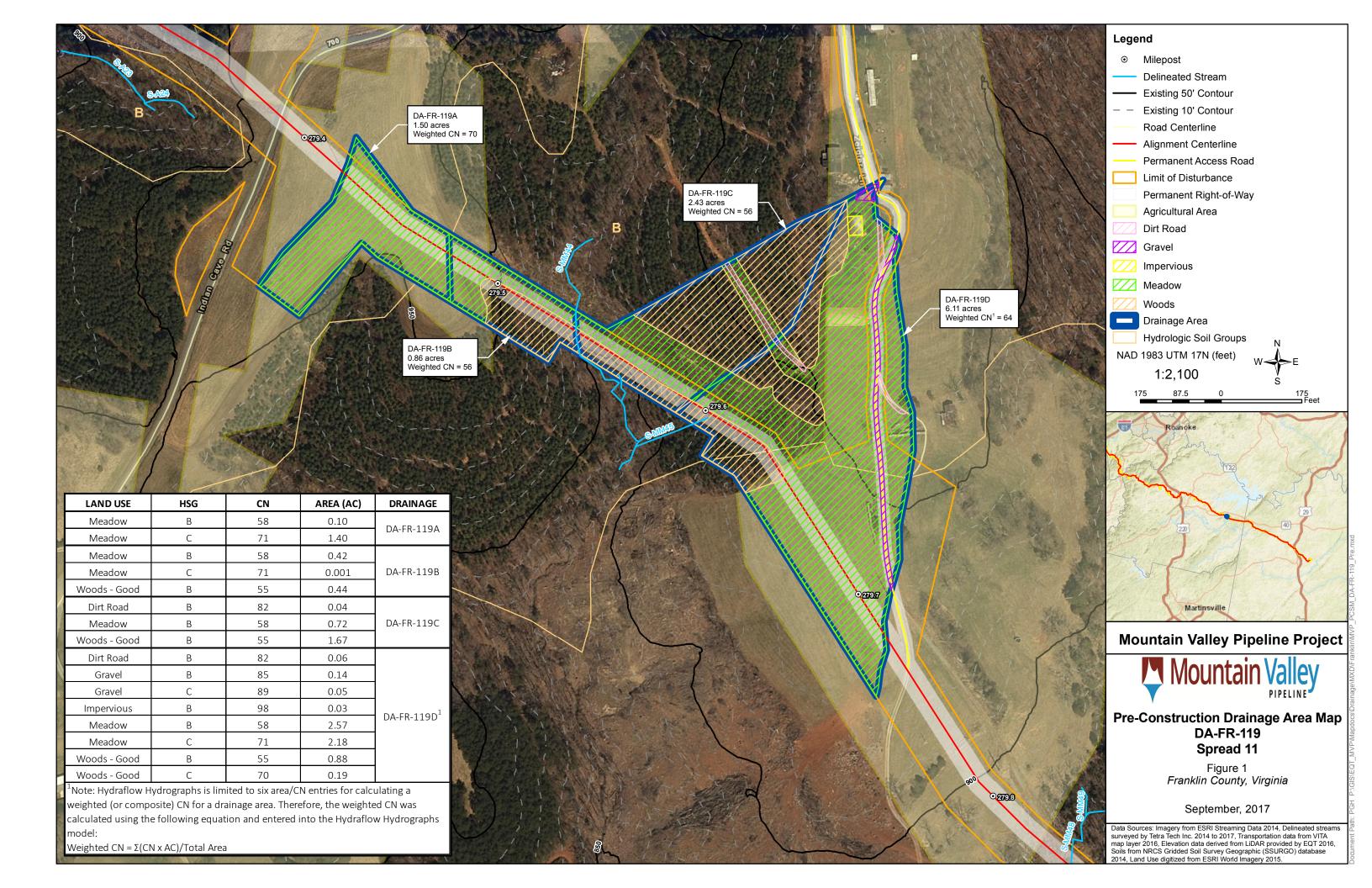
Agricultural areas/fields that will be returned to crop production, in identical condition, upon completion of pipeline construction are exempt from meeting the VA water quality (9VAC25-870-63) and the VA water quantity (9VAC25-870-66) requirements per § 62.1-44.15:34 and 9VAC25-870-300. Therefore, the provisions of water quality design criteria requirements and permanent stormwater management controls for DA-FR-119A are exempt and no stormwater BMPs are required.

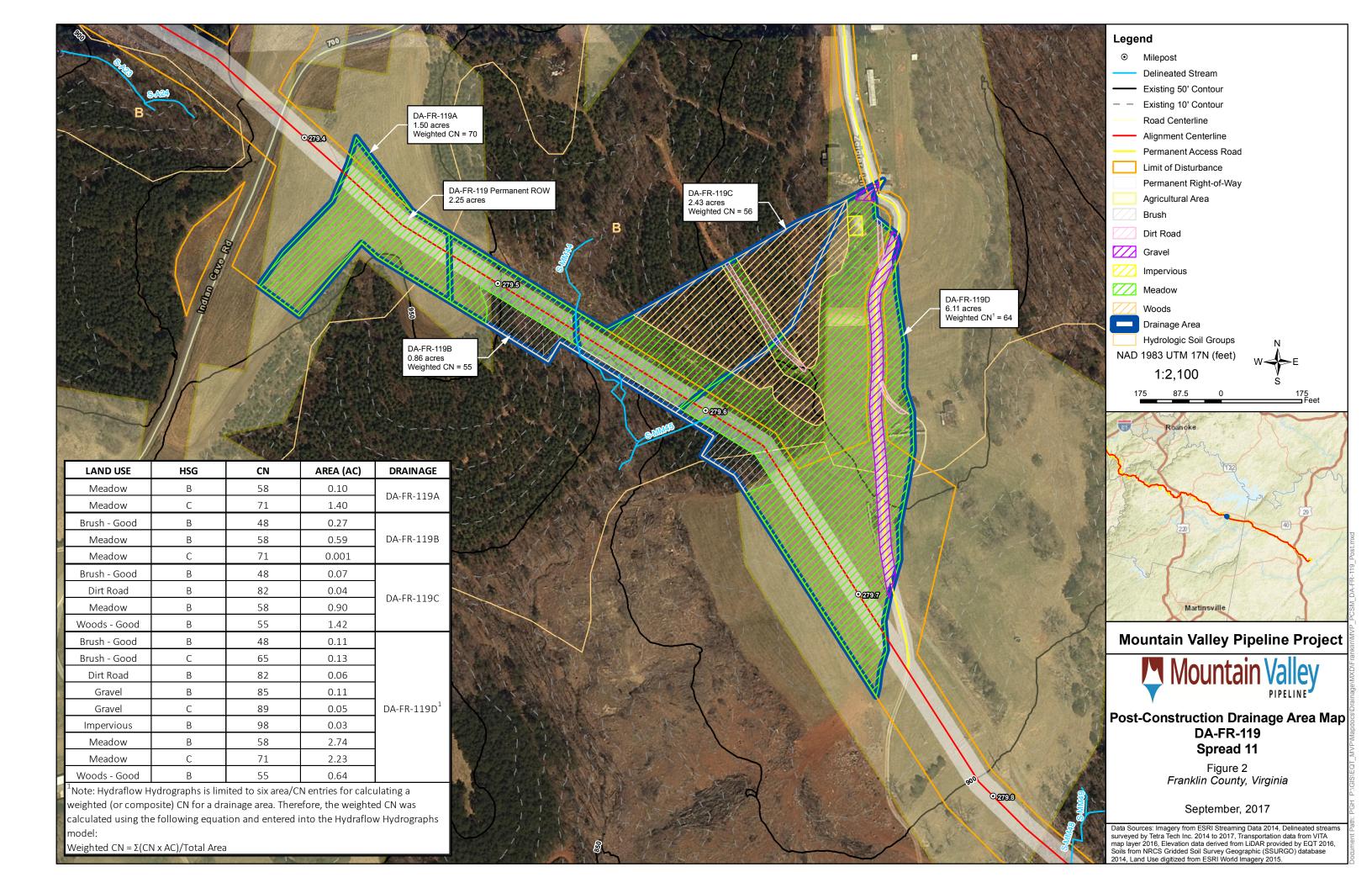
Stormwater quantity is met via the energy balance method for DA-FR-119B and DA-FR-119D. In addition to the energy balance method, storm water quantity in sub-area DA-FR-119C is met by two 2 ft by 50 ft compost amended water bar areas (See General Detail MVP-ES38 for a full schedule). Sub-area DA-FR-119C contains entirely non-agricultural areas within the LOD, therefore an Improvement Factor of 0.8 is used when applying the Energy Balance Method per 9VAC25-870-66.B.3.a. Sub-areas DA-FR-119B and DA-FR-119D contain both agricultural and non-agricultural areas within the LOD. For SWM quantity, agricultural areas within the study area are considered/included but an Improvement Factor of 1.0 is used when applying the Energy Balance Method to account for the exemption of agricultural areas (§ 62.1-44.15:34 and 9VAC25-870-300) since such areas will be returned to agricultural land use (i.e., returned to crop production, in identical condition) following construction.

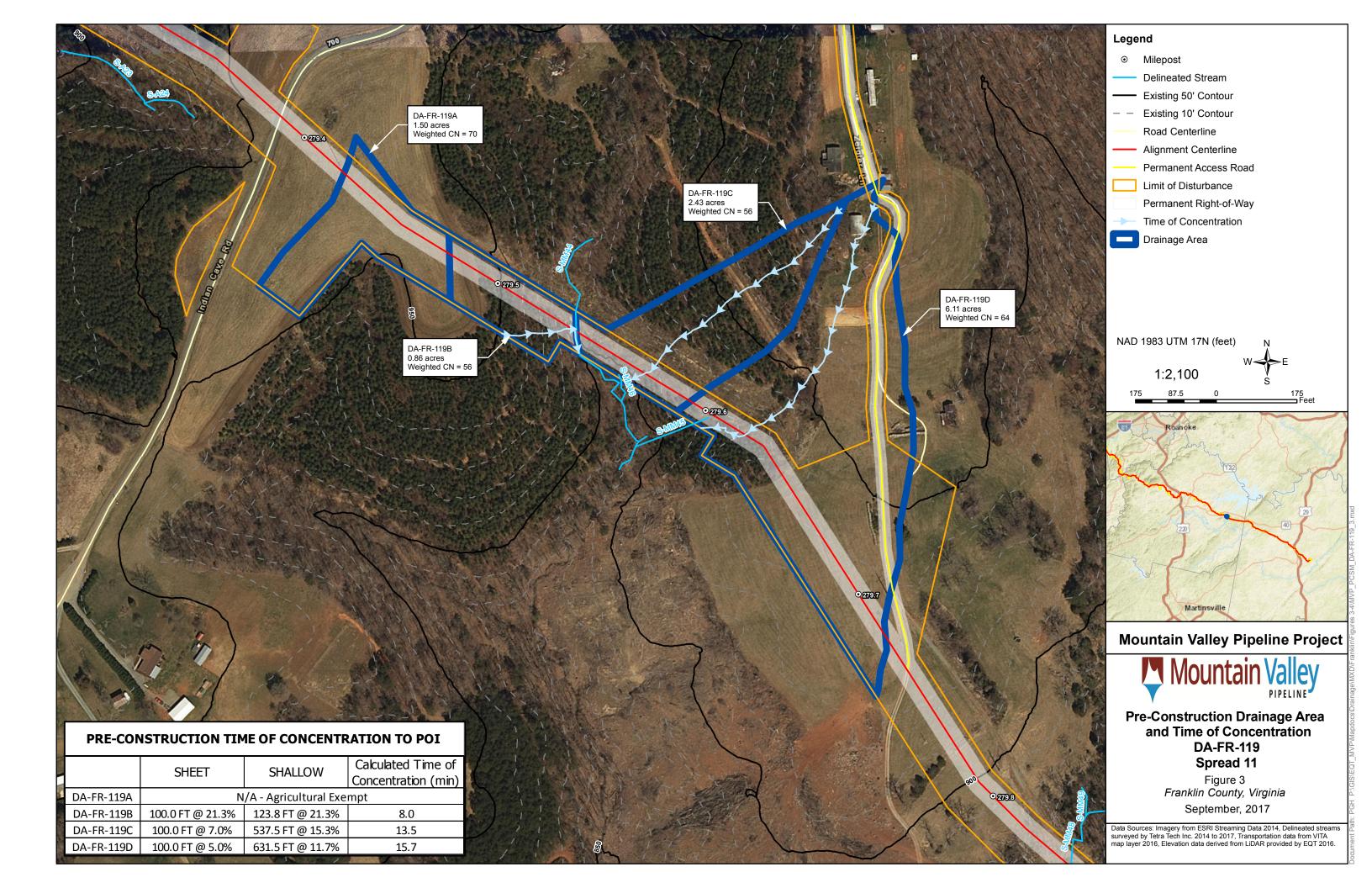
The Hydraflow 10-year 24-hour peak discharge results indicate a reduction in flows ranging from 0 to 0.12 cfs for sub-drainage areas DA-FR-119 B to D (as seen in the following table).

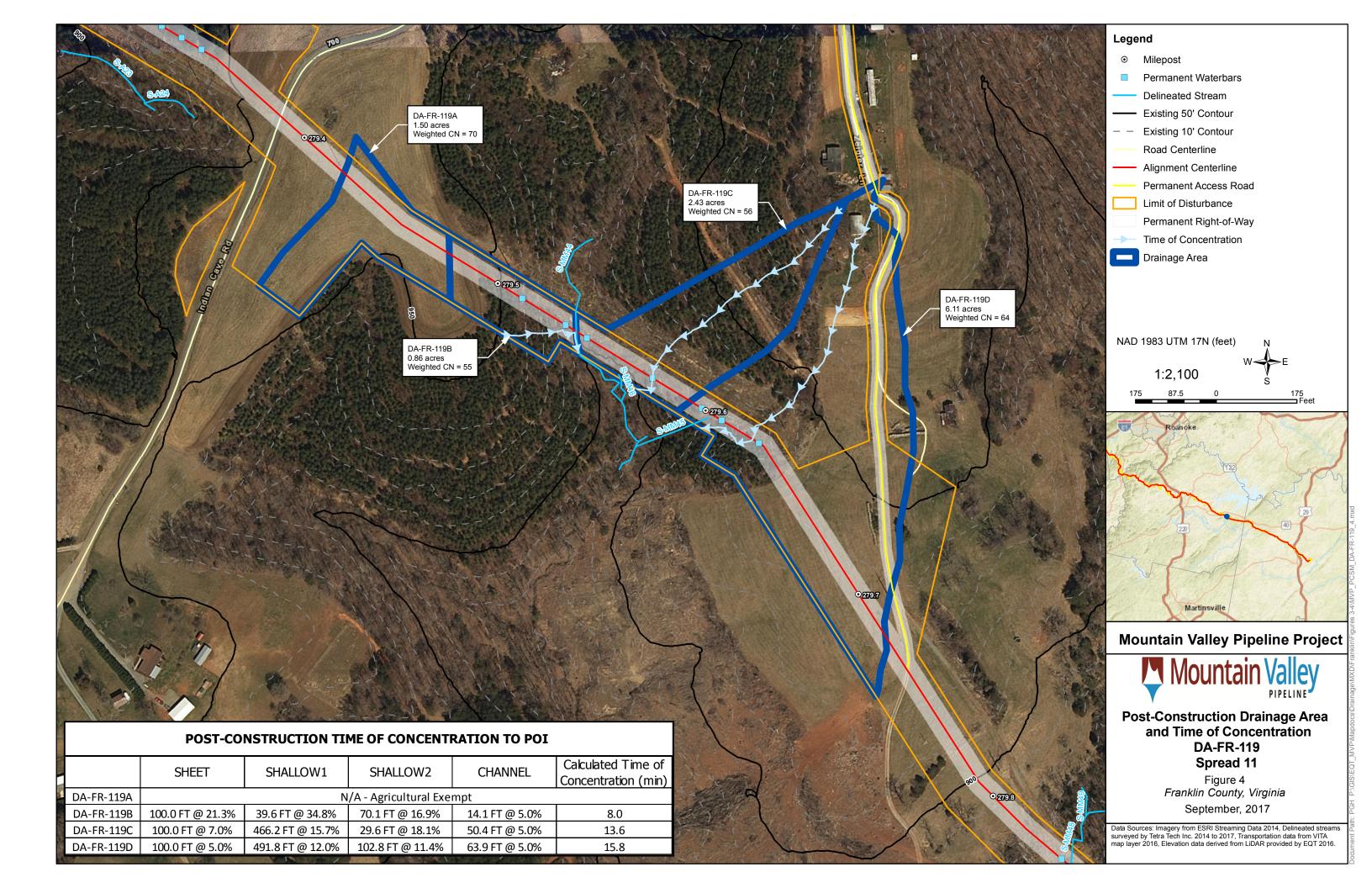
Sub-Area	Pre Peak Flow, 10-yr Q (cfs)	Post Peak Flow, 10-yr Q (cfs)	Flow Differential
DA-FR-119B	1.96	1.84	-0.12
DA-FR-119C	4.26	4.24	-0.02
DA-FR-119D	15.73	15.73	0.00

Figures and calculations for each of the sub-areas for DA-FR-119 follow. See Appendix D of the Annual Standards and Specifications for further detail on stormwater methodology.









DEQ Virginia Runoff Reduction Method Re-Development Compliance Spreadsheet - Version 3.0

BMP Design Specifications List: 2013 Draft Stds & Specs

Site Summary - Linear Development Project***

Total Rainfall (in):	43
Total Disturbed Acreage:	2.25

Site Land Cover Summary

Pre-ReDevelopment Land Cover (acres)

	A soils	B Soils	C Soils	D Soils	Totals	% of Total
Forest/Open (acres)	0.00	1.12	0.95	0.00	2.07	92
Managed Turf (acres)	0.00	0.00	0.00	0.00	0.00	0
Impervious Cover (acres)	0.00	0.13	0.05	0.00	0.18	8
					2.25	100

Post-ReDevelopment Land Cover (acres)

	A soils	B Soils	C Soils	D Soils	Totals	% of Total
Forest/Open (acres)	0.00	1.12	0.95	0.00	2.07	92
Managed Turf (acres)	0.00	0.00	0.00	0.00	0.00	0
Impervious Cover (acres)	0.00	0.13	0.05	0.00	0.18	8
* Forest/Open Space areas must be protected in	accordance with t	he Virginia Runoff Re	duction Method		2 25	100

Site Tv and Land Cover Nutrient Loads

	Final Post-Development (Post-ReDevelopment & New Impervious)	Post- ReDevelopment	Post- Development (New Impervious)	Adjusted Pre- ReDevelopment
Site Rv	0.11	0.11		0.11
Treatment Volume (ft³)	881	881		881
TP Load (lb/yr)	0.55	0.55		0.55

Baseline TP Load (lb/yr): 0.9225* *Reduction below new development load limitation not required

Total TP Load Reduction Required (lb/yr) -0.37 N/A*** N/A***
--

***This is a linear development project

	Final Post-Development Load (Post-ReDevelopment & New Impervious)	Pre- ReDevelopment
TN Load (lb/yr)	3.96	3.96

PreReDevelopment
TP Load per acre
(lb/acre/yr)

Pinal Post-Development
TP Load per acre
(lb/acre/yr)

Post-ReDevelopment TP
Load per acre
(lb/acre/yr)

(lb/acre/yr)

0.25

0.25

0.25

Site Compliance Summary - ***Linear Development Project

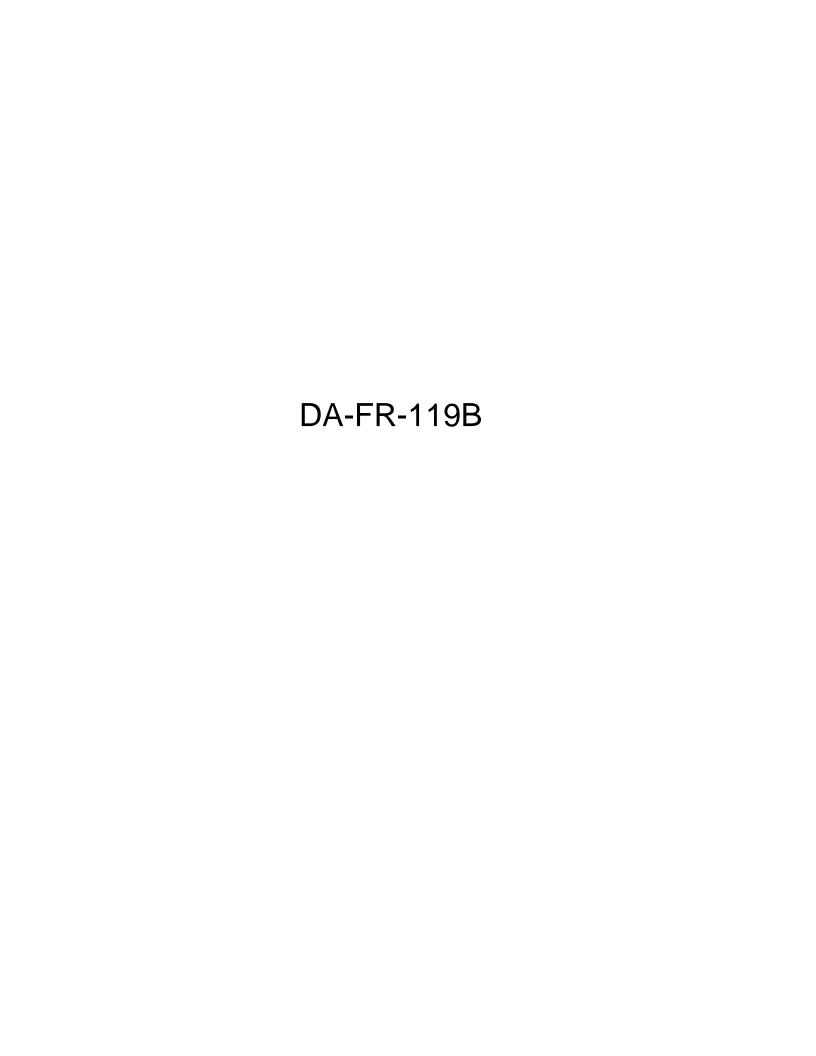
Maximum % Reduction Required Below	20%
Pre-ReDevelopment Load	20%

* Note: % Reduction will reduce post-development TP load to less than or equal to baseline load of 0.92 lb/yr (0.41 lb/ac/yr)
[Post-Dev Reduction Requirement = Post-Dev TP load - baseline load of 0.92 lb/yr], baseline load = site area x 0.41 lb/ac/yr

0	Total Runoff Volume Reduction (ft ³)	
0.00	Total TP Load Reduction Achieved (lb/yr)	
0.00	Total TN Load Reduction Achieved (lb/yr)	
0.55	Remaining Post Development TP Load (lb/yr)	
0.00	Remaining TP Load Reduction (lb/yr) Required	
*Padvation below now development load limitation not required		

** TARGET TP REDUCTION EXCEEDED BY 0.37 LB/YEAR **

^{*}Reduction below new development load limitation not required



ENERGY BALANCE METHOD

In		

Calculations:

	1-Yr Event	
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)
Pre-Developed Condition	0.256	949
Developed Condition	0.204	856
Pre-Developed (Forest) Condition	0.204	856

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF =

Check #1:	$Q_{developed} \leq IF \times \left[\left(Q_{gre\ developed} \times RV_{pre\ developed} \right) / RV_{developed} \right] >$	Q (cfs) 0.204	≤ OK	Q (cfs) 0.284
Check #2:	Q _{developed} ≤ Q _{pre-developed} >	0.204	≤ OK	0.256
Check #3:	$Q_{developed} \underline{shall not}$ be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed} \longrightarrow$	0.204	<u>shall not</u> be required to be ≤	0.204

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 - Manning's n Values for Open Channel Flow

nan	nel T	уре		Manning n	
			Min.	Normal	Max.
1.	Exc	cavated or Dredged Channels ¹			
	a.	Earth, Straight, and Uniform:			
		Clean, recently completed	0.016	0.018	0.020
		Clean, after weathering	0.018	0.022	0.02
		Gravel, uniform section, clean	0.022	0.025	0.030
		With short grass, few weeds	0.022	0.027	0.03
	b.	Earth Winding and Sluggish:			
		No vegetation	0.023	0.025	0.030
		Grass, some weeds	0.025	0.030	0.03
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040
		Earth bottom and rubble sides	0.028	0.030	0.03
		Stony bottom and weedy banks	0.025	0.035	0.040
		Cobble bottom and clean sides	0.030	0.040	0.050
	c.	Dragline-Excavated or Dredged:			
		No vegetation	0.025	0.028	0.03
		Light brush on banks	0.035	0.050	0.060
	d.	Rock Cuts:			
		Smooth and uniform	0.025	0.035	0.040
		Jagged and irregular	0.035	0.040	0.050
	e.	Channels not Maintained, Weeds and Brush Uncut:			
		Dense weeds, high as flow depth	0.050	0.080	0.120
		Clean bottom, brush on sides	0.040	0.050	0.080
		Same as above, highest stage of flow	0.045	0.070	0.110
		Dense brush, high stage	0.080	0.100	0.140
2.	Mai	n Channels²			
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.03
	b.	Same as above, but more stones and weeds	0.030	0.035	0.040
	C.	Clean, winding, some pools and shoals	0.033	0.040	0.04
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.05
	f.	Same as (d) with more stones	0.045	0.050	0.060
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

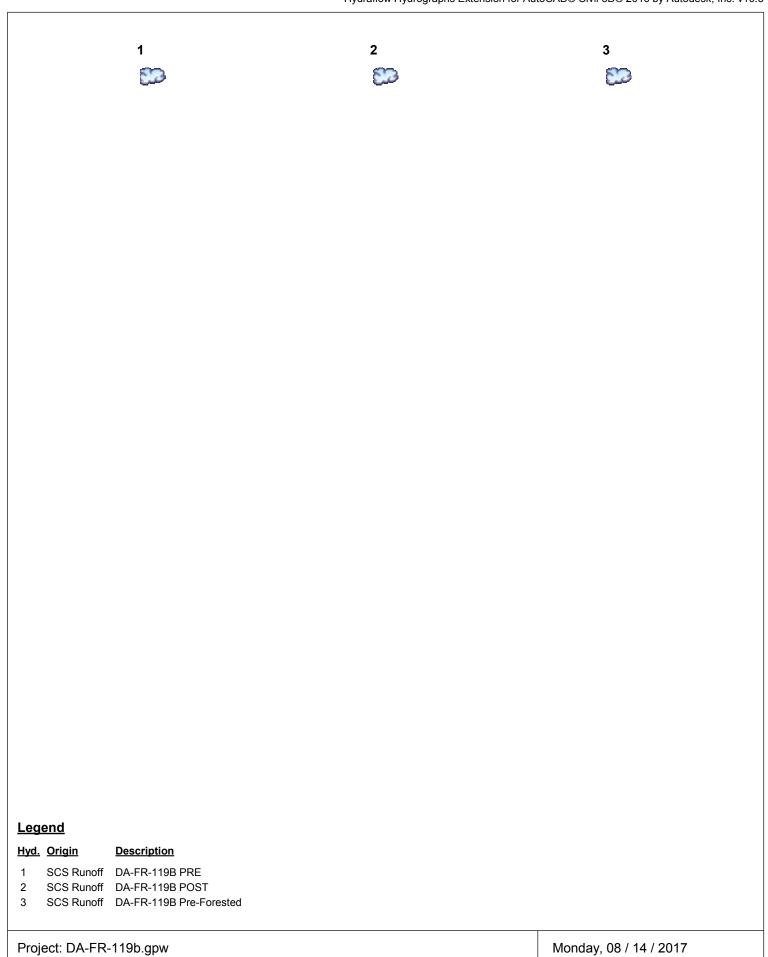
-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 14 / 2017

Watershed Model Schematic	. 1
Hydrograph Return Period Recap	. 2
1 - Year	
Summary Report	. 3
Hydrograph Reports	. 4
Hydrograph No. 1, SCS Runoff, DA-FR-119B PRE	. 4
TR-55 Tc Worksheet	
Hydrograph No. 2, SCS Runoff, DA-FR-119B POST	6
TR-55 Tc Worksheet	
Hydrograph No. 3, SCS Runoff, DA-FR-119B Pre-Forested	
TR-55 Tc Worksheet	9
2 - Year	
Summary Report	10
Hydrograph Reports	11
Hydrograph No. 1, SCS Runoff, DA-FR-119B PRE	11
Hydrograph No. 2, SCS Runoff, DA-FR-119B POST	12
Hydrograph No. 3, SCS Runoff, DA-FR-119B Pre-Forested	13
10 - Year	
Summary Report	14
Hydrograph Reports	15
Hydrograph No. 1, SCS Runoff, DA-FR-119B PRE	15
Hydrograph No. 2, SCS Runoff, DA-FR-119B POST	
Hydrograph No. 3, SCS Runoff, DA-FR-119B Pre-Forested	
IDF Report	18



Hydrograph Return Period Recap Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

lyd.	Hydrograph type	Inflow				Hydrograph					
lo.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		0.256	0.468			1.963				DA-FR-119B PRE
2	SCS Runoff		0.204	0.399			1.841				DA-FR-119B POST
3	SCS Runoff		0.204	0.399			1.841				DA-FR-119B Pre-Forested

Proj. file: DA-FR-119b.gpw

Monday, 08 / 14 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hvd	Hydrograph	Peak	Time	Time to	Hyd.	Inflow	Maximum	Total	Hydrograph
No.	type (origin)	flow (cfs)	interval (min)	Peak (min)	volume (cuft)	hyd(s)	elevation (ft)	strge used (cuft)	Description
1	SCS Runoff	0.256	1	721	949				DA-FR-119B PRE
2	SCS Runoff	0.204	1	721	856				DA-FR-119B POST
3	SCS Runoff	0.204	1	721	856				DA-FR-119B Pre-Forested
DA:	-FR-119b.gp	W	•	•	Return F	Period: 1 Ye	ear	Monday, 08	3 / 14 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

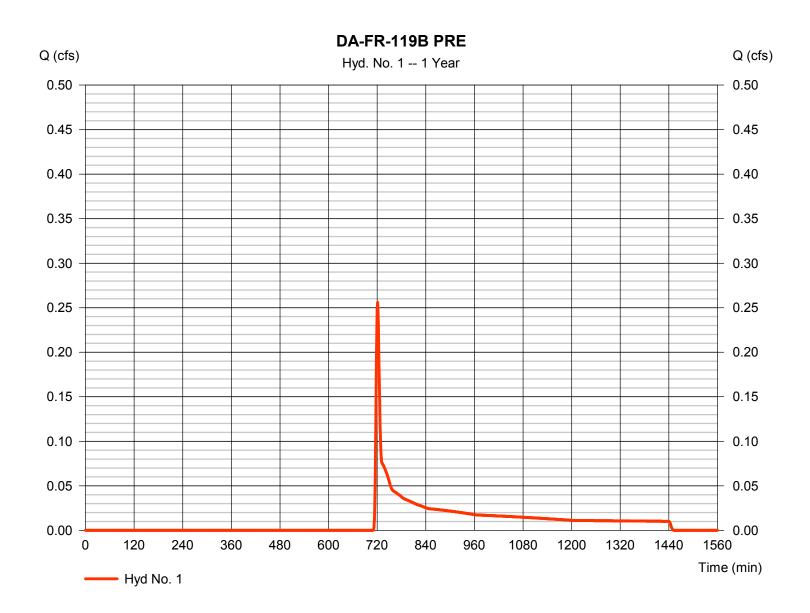
Monday, 08 / 14 / 2017

Hyd. No. 1

DA-FR-119B PRE

Hydrograph type = SCS Runoff Peak discharge = 0.256 cfsStorm frequency Time to peak = 721 min = 1 yrsTime interval = 1 min Hyd. volume = 949 cuft Curve number Drainage area = 0.860 ac= 56* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 8.00 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.421 \times 58) + (0.001 \times 71) + (0.439 \times 55)] / 0.860$



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1DA-FR-119B PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 21.30		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 7.75	+	0.00	+	0.00	=	7.75
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 123.80 = 21.30 = Unpaved =7.45	t	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.28	+	0.00	+	0.00	=	0.28
Channel Flow							
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.040 = 0.040		0.00 0.00 0.00 0.015		0.00 0.00 0.00 0.015		
					0.00		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							8.00 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

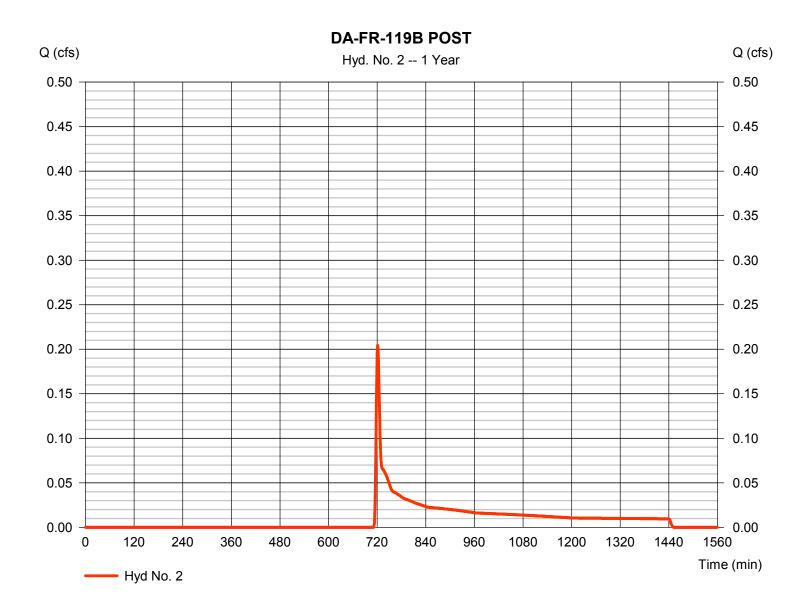
Monday, 08 / 14 / 2017

Hyd. No. 2

DA-FR-119B POST

Hydrograph type = SCS Runoff Peak discharge = 0.204 cfsStorm frequency Time to peak = 721 min = 1 yrsTime interval = 1 min Hyd. volume = 856 cuft Curve number Drainage area = 0.860 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 8.00 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.271 x 48) + (0.589 x 58) + (0.001 x 71)] / 0.860



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2DA-FR-119B POST

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 21.30		0.800 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 7.75	+	0.00	+	0.00	=	7.75
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 39.60 = 34.80 = Unpaved =9.52	d	70.10 16.90 Unpave 6.63	d	0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.07	+	0.18	+	0.00	=	0.25
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})14.1		0.0		0.0		
Travel Time (min)	= 0.05	+	0.00	+	0.00	=	0.05
Total Travel Time, Tc							8.00 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

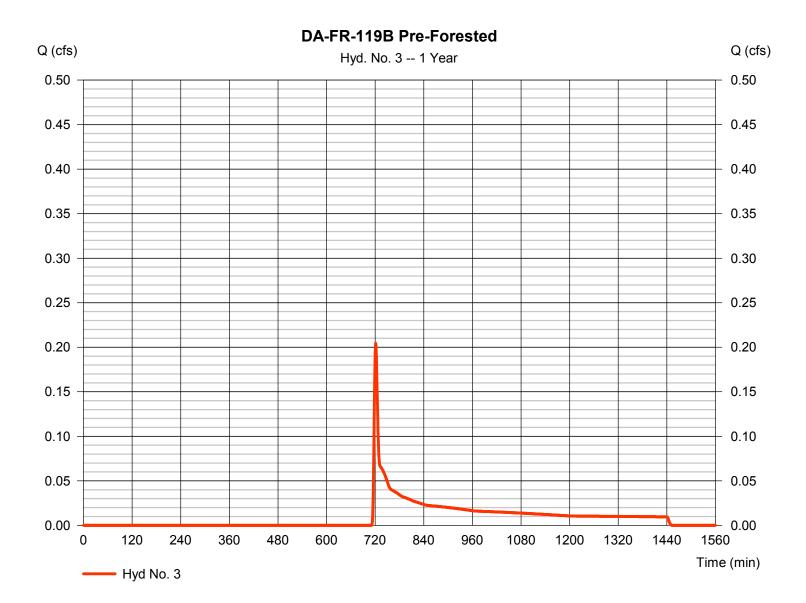
Monday, 08 / 14 / 2017

Hyd. No. 3

DA-FR-119B Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 0.204 cfsStorm frequency Time to peak = 721 min = 1 yrsTime interval = 1 min Hyd. volume = 856 cuft Curve number Drainage area = 0.860 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 8.00 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.860 x 55) + (0.001 x 70)] / 0.860



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3DA-FR-119B Pre-Forested

<u>Description</u>	<u>A</u>		<u>B</u>		<u>c</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 21.30		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 7.75	+	0.00	+	0.00	=	7.75
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 123.80 = 21.30 = Unpaved =7.45	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.28	+	0.00	+	0.00	=	0.28
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.030 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							8.00 min

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	Hydrograph	Peak	Time	Time to	Hyd.	Inflow	Maximum	Total	Hydrograph		
No.	type (origin)	flow (cfs)	interval (min)	Peak (min)	volume (cuft)	hyd(s)	elevation (ft)	strge used (cuft)	Description		
1	SCS Runoff	0.468	1	720	1,381				DA-FR-119B PRE		
2	SCS Runoff	0.399	1	721	1,265				DA-FR-119B POST		
3	SCS Runoff	0.399	1	721	1,265				DA-FR-119B Pre-Forested		
DA	-FR-119b.gp	W			Return F	Period: 2 Ye	ear	Monday, 08 / 14 / 2017			

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

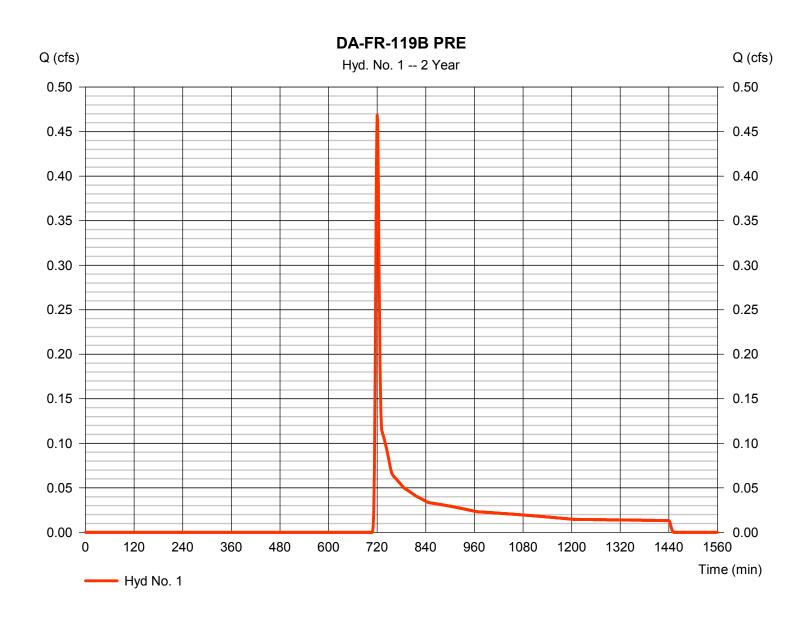
Monday, 08 / 14 / 2017

Hyd. No. 1

DA-FR-119B PRE

Hydrograph type = SCS Runoff Peak discharge = 0.468 cfsStorm frequency = 2 yrsTime to peak = 720 min Time interval = 1 min Hyd. volume = 1,381 cuft Curve number Drainage area = 0.860 ac= 56* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 8.00 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = $[(0.421 \times 58) + (0.001 \times 71) + (0.439 \times 55)] / 0.860$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

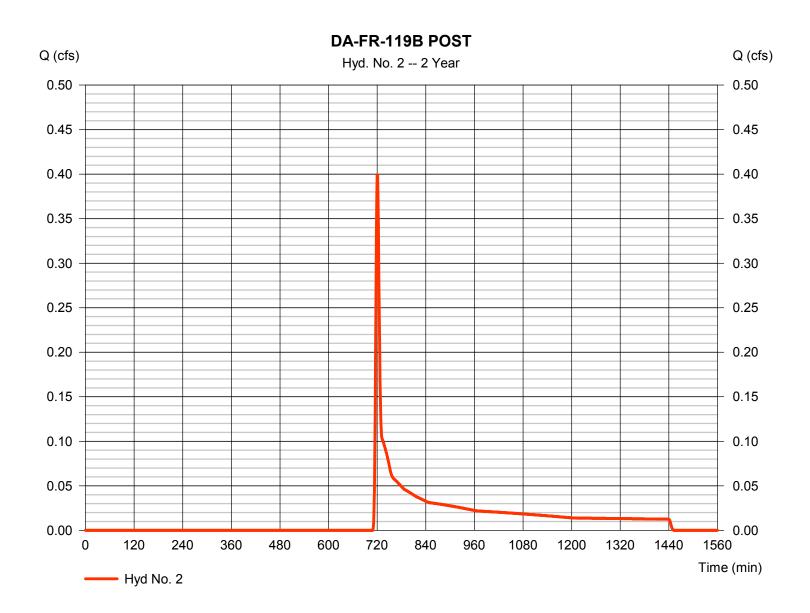
Monday, 08 / 14 / 2017

Hyd. No. 2

DA-FR-119B POST

Hydrograph type = SCS Runoff Peak discharge = 0.399 cfsStorm frequency = 2 yrsTime to peak = 721 min Time interval = 1 min Hyd. volume = 1.265 cuft Curve number Drainage area = 0.860 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 8.00 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = [(0.271 x 48) + (0.589 x 58) + (0.001 x 71)] / 0.860



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

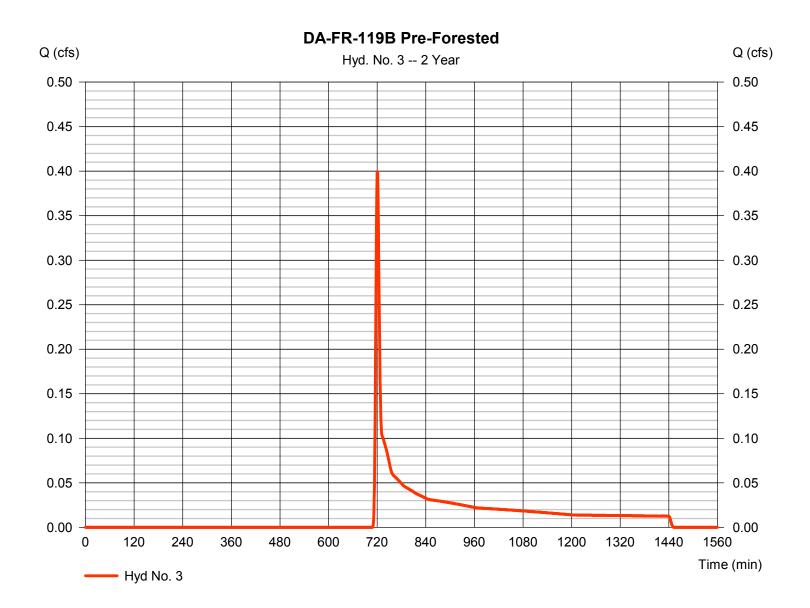
Monday, 08 / 14 / 2017

Hyd. No. 3

DA-FR-119B Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 0.399 cfsStorm frequency = 2 yrsTime to peak = 721 min Time interval = 1 min Hyd. volume = 1.265 cuft Curve number Drainage area = 0.860 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 8.00 min = TR55 Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = [(0.860 x 55) + (0.001 x 70)] / 0.860



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

yd. o.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	1.963	1	719	4,329				DA-FR-119B PRE
2	SCS Runoff	1.841	1	719	4,105				DA-FR-119B POST
2 3									
· Λ	⊥ -FR-119b.gp\			1	-	□ Period: 10 \			⊥ 8 / 14 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

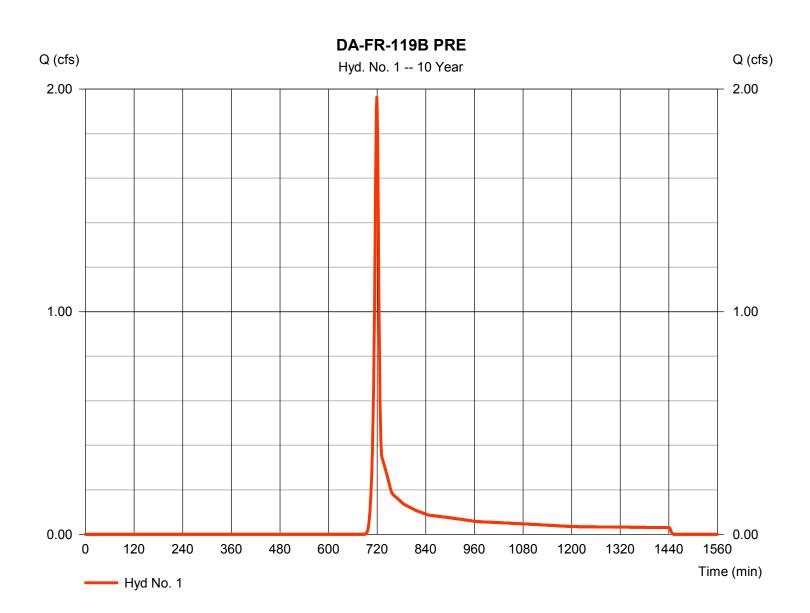
Monday, 08 / 14 / 2017

Hyd. No. 1

DA-FR-119B PRE

Hydrograph type = SCS Runoff Peak discharge = 1.963 cfsStorm frequency = 10 yrsTime to peak = 719 min Time interval = 1 min Hyd. volume = 4,329 cuftCurve number Drainage area = 0.860 ac= 56* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 8.00 min = TR55 Total precip. Distribution = Type II = 5.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.421 \times 58) + (0.001 \times 71) + (0.439 \times 55)] / 0.860$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

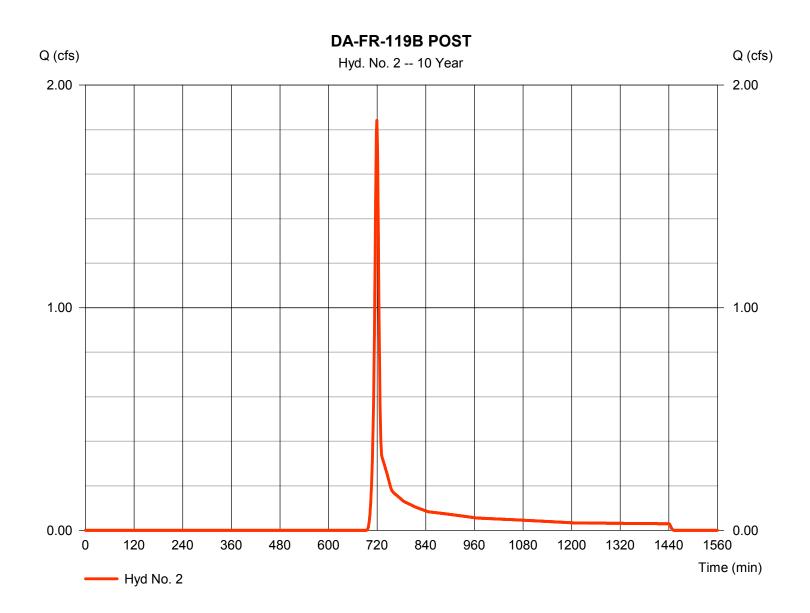
Monday, 08 / 14 / 2017

Hyd. No. 2

DA-FR-119B POST

Hydrograph type = SCS Runoff Peak discharge = 1.841 cfsStorm frequency = 10 yrsTime to peak = 719 min Time interval = 1 min Hyd. volume = 4,105 cuftCurve number Drainage area = 0.860 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 8.00 min = TR55 Total precip. Distribution = Type II = 5.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.271 x 48) + (0.589 x 58) + (0.001 x 71)] / 0.860



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

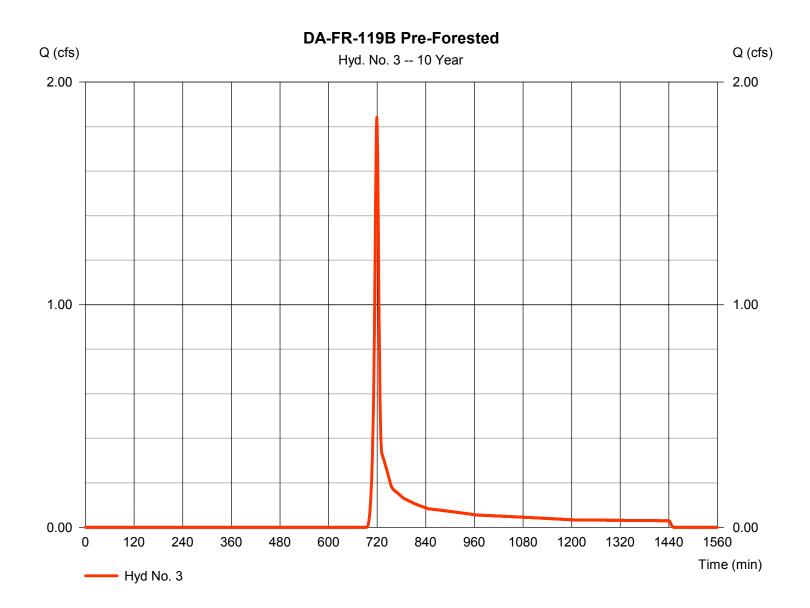
Monday, 08 / 14 / 2017

Hyd. No. 3

DA-FR-119B Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 1.841 cfsStorm frequency = 10 yrsTime to peak = 719 min Time interval = 1 min Hyd. volume = 4,105 cuftDrainage area Curve number = 0.860 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 8.00 min = TR55 Total precip. Distribution = Type II = 5.70 inStorm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.860 x 55) + (0.001 x 70)] / 0.860



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 14 / 2017

Return Period	Intensity-Du	ıration-Frequency E	quation Coefficients	(FHA)
(Yrs)	В	D	E	(N/A)
1	0.0000	0.0000	0.0000	
2	69.8703	13.1000	0.8658	
3	0.0000	0.0000	0.0000	
5	79.2597	14.6000	0.8369	
10	88.2351	15.5000	0.8279	
25	102.6072	16.5000	0.8217	
50	114.8193	17.2000	0.8199	
100	127.1596	17.8000	0.8186	

File name: SampleFHA.idf

Intensity = B / (Tc + D)^E

Return					Intens	sity Values	(in/hr)					
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

Precip. file name: P:\MVP ESC&SWM Review\Engineering\SW Calculations\Franklin County.pcp

		R	ainfall P	recipitat	ion Tab	le (in)		
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00



STORAGE VOLUME OF WATERBAR WITH SOIL COMPOST AMENDMENT AREA

Equations Used:
¹Vgravel storage = L*W*D_{gravel}*(40/100)

²Vsoil storage = L*W*D_{soil}*(20/100)

 3 V surface storage = [W*S*D^2]+[L*S*D^2]+[W*L*D]+[((2*S*D)^2*D)/3]

¹Equation #2b under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, assuming that gravel is made up of 40% voids.

²Equation #2b under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, assuming that soil compost amendment is made up of 20% voids.

³Equation #1 under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, but calculation also takes into account surface side slopes.

Inputs:	Depti	n of Gravel Layer, D _{gravel} (ft) =	0	
	Depth of Soil	Depth of Soil Amendment Area, D _{soil} (ft) =		Refer to Table 4.3 in VA DEQ Stormwater Design Specification No. 4; Note that compost amendment may not be necessary for HSG A/B soils
	Length of Waterbar Soil Amendment Area, L (ft) =		50	Assume max. length of 50' for waterbar soil amendment areas (i.e., limited to permanent ROW)
	Width of Waterbar So	il Amendment Area, W (ft) =	2	
	Inside Emban	kment Side Slopes, S (H:V) =	2	Assume 2H:1V surface side slopes for waterbars
	Number of Perm. Wa	terbars in Drainage Area, n =	2	
	Design	Infiltration Rate, IR (in/hr) =	0.2	Min. rate of 0.30 in/hr for HSG A soils and 0.15-0.30 in/hr for HSG B soils (see Chap. 4, p. 4-30 in VA Stormwater Management Handbook Volume II (First Edition, 1999)
	Su	rface Ponding Depth, D (ft) =	0.5	Assume 0.5' CFS height at the end of waterbars
Calandadanan	T-4-16	****** Danah DAAD (f4)	4.5	
Calculations:	Total Storage Depth per BMP (ft) =			
		orage Volume per BMP (cf) =		
	Subsurface Sto	orage Volume per BMP (cf) =	20	
	Total Sto	orage Volume per BMP (cf) =	96.6666667	
	Total BMP Storage Vo	olume in Drainage Area (cf) =	193.3333333	
	Calculated Infilt	ration Period per BMP (hr) =	58	
		Depth-Storage Da	22	
Depth (ft)	Width (ft)	Length (ft)	Storage Volume per BMP (cf)	Storage Volume in Drainage Area (cf)
Deptii (it)	width (it)	50	Storage volume per bivir (cr)	Storage volume in an age Area (cr)
0	2		0	0
0.5	2	50	10	20
1	2	50	20	40
1.5	4	52	96.6666667	193.333333
2	6	54	229.3333333	458.666667

ENERGY BALANCE METHOD

Inputs:

	1-Yr Event				
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)			
Pre-Developed Condition	0.502	2749			
Developed Condition	0.431	2555			
Pre-Developed (Forest) Condition	0.401	2479			

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.8

Calculations:	Check #1:	$Q_{\text{developed}} \leq \text{IF x } \left[\left(Q_{\text{pre-developed}} \times \text{RV}_{\text{pre-developed}} \right) / \text{RV}_{\text{developed}} \right]>$	Q (cfs) 0.431	≤ OK	Q (cfs) 0.432
	Check #2:	$Q_{developed} \le Q_{pre-developed}$ >	0.431	≤ OK	0.502
	Check #3:	$Q_{tanahonart}$ shall not be required to be $\leq (Q_{toract} \times RV_{toract}) / RV_{tanahonart}$ >	0.431	shall not be required to be ≤	0.389

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 – Manning's n Values for Open Channel Flow

Channel Type		I			
			Min.	Normal	Max.
1.	Exc	avated or Dredged Channels ¹			
	a.	Earth, Straight, and Uniform:			
		Clean, recently completed	0.016	0.018	0.020
		Clean, after weathering	0.018	0.022	0.025
		Gravel, uniform section, clean	0.022	0.025	0.030
		With short grass, few weeds	0.022	0.027	0.033
	b.	Earth Winding and Sluggish:			
		No vegetation	0.023	0.025	0.030
		Grass, some weeds	0.025	0.030	0.033
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040
		Earth bottom and rubble sides	0.028	0.030	0.035
		Stony bottom and weedy banks	0.025	0.035	0.040
		Cobble bottom and clean sides	0.030	0.040	0.050
	c.	Dragline-Excavated or Dredged:		•	
		No vegetation	0.025	0.028	0.033
		Light brush on banks	0.035	0.050	0.060
	d.	Rock Cuts:			
		Smooth and uniform	0.025	0.035	0.040
		Jagged and irregular	0.035	0.040	0.050
	e.	Channels not Maintained, Weeds and Brush Uncut:		•	
		Dense weeds, high as flow depth	0.050	0.080	0.120
		Clean bottom, brush on sides	0.040	0.050	0.080
		Same as above, highest stage of flow	0.045	0.070	0.110
		Dense brush, high stage	0.080	0.100	0.140
2.	Mai	n Channels²			
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.033
	b.	Same as above, but more stones and weeds	0.030	0.035	0.040
	c.	Clean, winding, some pools and shoals	0.033	0.040	0.04
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.055
	f.	Same as (d) with more stones	0.045	0.050	0.060
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150

Notes:

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

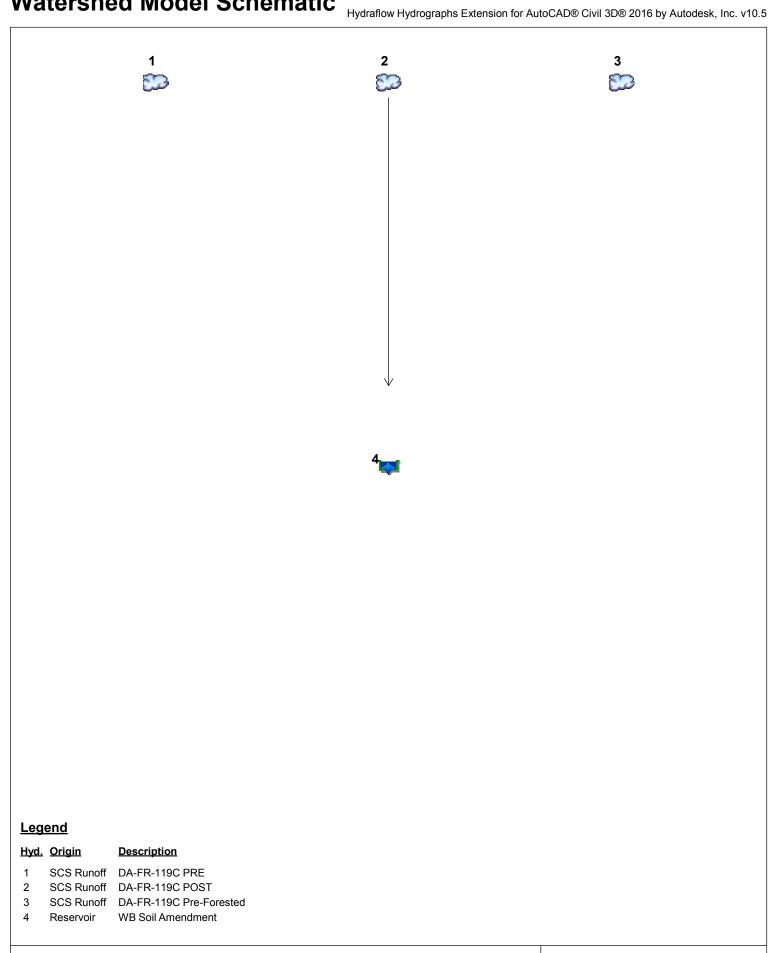
²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 16 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	4
Hydrograph No. 1, SCS Runoff, DA-FR-119C PRE	4
TR-55 Tc Worksheet	5
Hydrograph No. 2, SCS Runoff, DA-FR-119C POST	6
TR-55 Tc Worksheet	7
Hydrograph No. 3, SCS Runoff, DA-FR-119C Pre-Forested	
TR-55 Tc Worksheet	9
Hydrograph No. 4, Reservoir, WB Soil Amendment	10
Pond Report - Waterbar Soil Amendment	. 11
2 - Year	
Summary Report	. 12
Hydrograph ReportsHydrograph No. 1, SCS Runoff, DA-FR-119C PRE	. 13
Hydrograph No. 1, SCS Runoff, DA-FR-119C PRE	. 13
Hydrograph No. 2, SCS Runoff, DA-FR-119C POST	
Hydrograph No. 3, SCS Runoff, DA-FR-119C Pre-Forested	
Hydrograph No. 4, Reservoir, WB Soil Amendment	16
10 - Year	
	47
Summary Report	. 17
Hydrograph Reports Hydrograph No. 1, SCS Runoff, DA-FR-119C PRE	. 18 10
Hydrograph No. 1, SCS Runoff, DA FR-119C PRE	. 10 10
Hydrograph No. 2, SCS Runoff, DA-FR-119C POSTHydrograph No. 3, SCS Runoff, DA-FR-119C Pre-Forested	
Hydrograph No. 4, Reservoir, WB Soil Amendment	
riyarograpiriyo. 4, Neservoli, vvb soli Amenament	۱ ک
IDF Report	22
in izebott	~~

Project: DA-FR-119c.gpw



Wednesday, 08 / 16 / 2017

Hydrograph Return Period Recap Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	Hydrograph	Inflow				Hydrograph					
о.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		0.502	0.938			4.263				DA-FR-119C PRE
2	SCS Runoff		0.502	0.938			4.263				DA-FR-119C POST
3	SCS Runoff		0.401	0.797			3.980				DA-FR-119C Pre-Forested
4	Reservoir	2	0.431	0.912			4.237				WB Soil Amendment

Proj. file: DA-FR-119c.gpw

Wednesday, 08 / 16 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

						_			ision for AdioCAD® Civil 3D® 2010 by Adiodesk,		
lyd. lo.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description		
1	SCS Runoff	0.502	1	726	2,749				DA-FR-119C PRE		
2	SCS Runoff	0.502	1	726	2,749				DA-FR-119C POST		
3	SCS Runoff	0.401	1	726	2,479				DA-FR-119C Pre-Forested		
4	Reservoir	0.431	1	731	2,555	2	907.60	247	WB Soil Amendment		
DΑ	-FR-119c.gp\	W		•	Return	Period: 1 Y	ear	Wednesda	y, 08 / 16 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

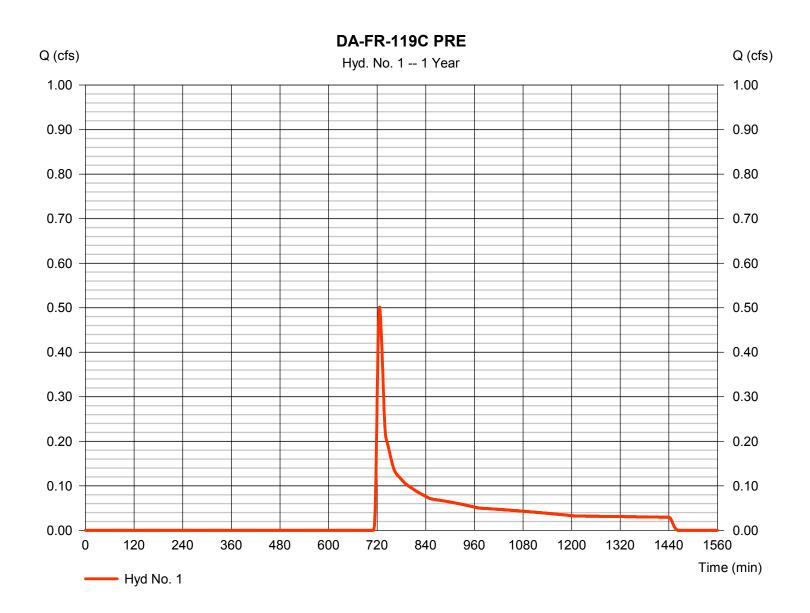
Wednesday, 08 / 16 / 2017

Hyd. No. 1

DA-FR-119C PRE

Hydrograph type = SCS Runoff Peak discharge = 0.502 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 min Hyd. volume = 2.749 cuftCurve number Drainage area = 2.430 ac= 56* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 13.50 \, \text{min}$ Total precip. Distribution = Type II = 3.30 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.040 x 82) + (0.720 x 58) + (1.670 x 55)] / 2.430



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1DA-FR-119C PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 7.00	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	12.10
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 537.50 = 15.60 = Unpaved =6.37	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.41	+	0.00	+	0.00	=	1.41
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.040 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							13.50 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

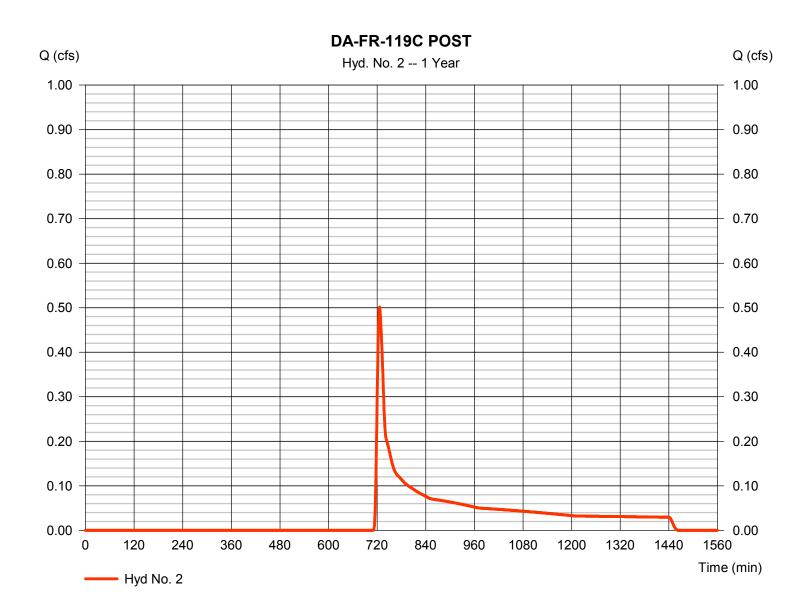
Wednesday, 08 / 16 / 2017

Hyd. No. 2

DA-FR-119C POST

Hydrograph type = SCS Runoff Peak discharge = 0.502 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 min Hyd. volume = 2.749 cuftCurve number Drainage area = 2.430 ac= 56* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 13.60 \, \text{min}$ Total precip. Distribution = Type II = 3.30 inStorm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.070 \times 48) + (0.040 \times 82) + (0.900 \times 58) + (1.420 \times 55)] / 2.430$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2DA-FR-119C POST

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 7.00		0.800 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 12.10	+	0.00	+	0.00	=	12.10
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 466.20 = 15.70 = Unpaved =6.39	t	29.60 18.10 Unpave 6.86	d	0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.22	+	0.07	+	0.00	=	1.29
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		0.00 0.00 0.00 0.015		0.00 0.00 0.00 0.015		
• ` , ,	-4.00		0.00		0.00		
Flow length (ft)	({0})50.4		0.00		0.00		
Flow length (ft) Travel Time (min)		+		+		=	0.17

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

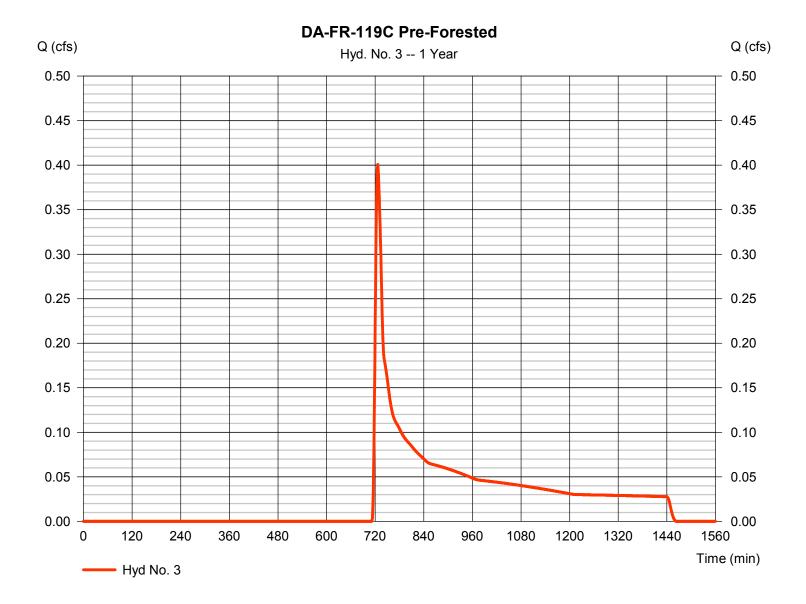
Wednesday, 08 / 16 / 2017

Hyd. No. 3

DA-FR-119C Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 0.401 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 min Hyd. volume = 2.479 cuftCurve number Drainage area = 2.430 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 13.50 min = TR55 Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(2.430 x 55)] / 2.430



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3DA-FR-119C Pre-Forested

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 7.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 12.10	+	0.00	+	0.00	=	12.10
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 537.50 = 15.60 = Unpaved =6.37	t	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.41	+	0.00	+	0.00	=	1.41
Channel Flow X sectional flow area (sqft)	= 0.00		0.00		0.00		
Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.030 =0.00		0.00 0.00 0.00 0.015		0.00 0.00 0.015		
Channel slope (%) Manning's n-value	= 0.00 = 0.030		0.00 0.00 0.015		0.00 0.00 0.015		
Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.030 =0.00	+	0.00 0.00 0.015 0.00	+	0.00 0.00 0.015 0.00	=	0.00

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

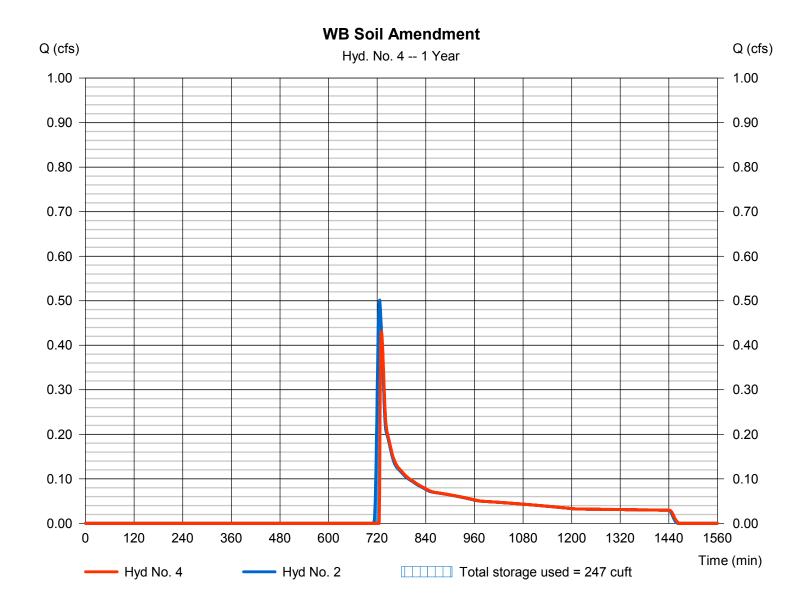
Wednesday, 08 / 16 / 2017

Hyd. No. 4

WB Soil Amendment

Hydrograph type Peak discharge = 0.431 cfs= Reservoir Storm frequency Time to peak = 731 min = 1 yrsTime interval = 1 min Hyd. volume = 2,555 cuftInflow hyd. No. Max. Elevation = 2 - DA-FR-119C POST = 907.60 ftReservoir name = Waterbar Soil Amendment Max. Storage = 247 cuft

Storage Indication method used.



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 16 / 2017

Pond No. 1 - Waterbar Soil Amendment

Pond Data

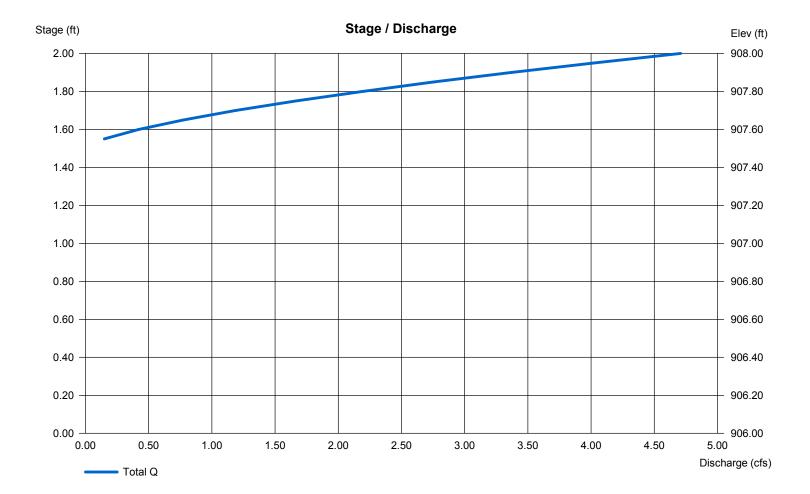
Pond storage is based on user-defined values.

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	906.00	n/a	0	0
0.50	906.50	n/a	20	20
1.00	907.00	n/a	20	40
1.50	907.50	n/a	153	193
2.00	908.00	n/a	265	459

Culvert / Orifice Structures Weir Structures [PrfRsr] [A] [B] [C] [D] [A] [B] [C] Rise (in) = 0.000.00 0.00 0.00 Crest Len (ft) = 4.00 0.00 0.00 0.00 Span (in) = 0.00 0.00 0.00 0.00 Crest El. (ft) = 907.50 0.00 0.00 0.00 No. Barrels = 0 0 Weir Coeff. = 3.333.33 3.33 3.33 = 0.000.00 0.00 0.00 Weir Type = Rect Invert El. (ft) = 0.000.00 0.00 0.00 Multi-Stage Length (ft) = No No No No Slope (%) = 0.000.00 0.00 n/a N-Value = .013 .013 .013 n/a 0.60 0.60 = 0.600.60 Exfil.(in/hr) = 0.000 (by Wet area) Orifice Coeff. Multi-Stage = n/aNo No No TW Elev. (ft) = 0.00

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

lyd. No.	Hydrograph type	Peak flow	Time interval		Hyd. volume	Inflow hyd(s)	Maximum elevation	Total strge used	Hydrograph Description
	(origin)	(cfs)	(min)	(min)	(cuft)		(ft)	(cuft)	
1	SCS Runoff	0.938	1	725	4,002				DA-FR-119C PRE
2	SCS Runoff	0.938	1	725	4,002				DA-FR-119C POST
3	SCS Runoff	0.797	1	725	3,666				DA-FR-119C Pre-Forested
4	Reservoir	0.912	1	727	3,808	2	907.67	282	WB Soil Amendment
DA	-FR-119c.gp\	W			Return	Period: 2 Y	ear	Wednesda	y, 08 / 16 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

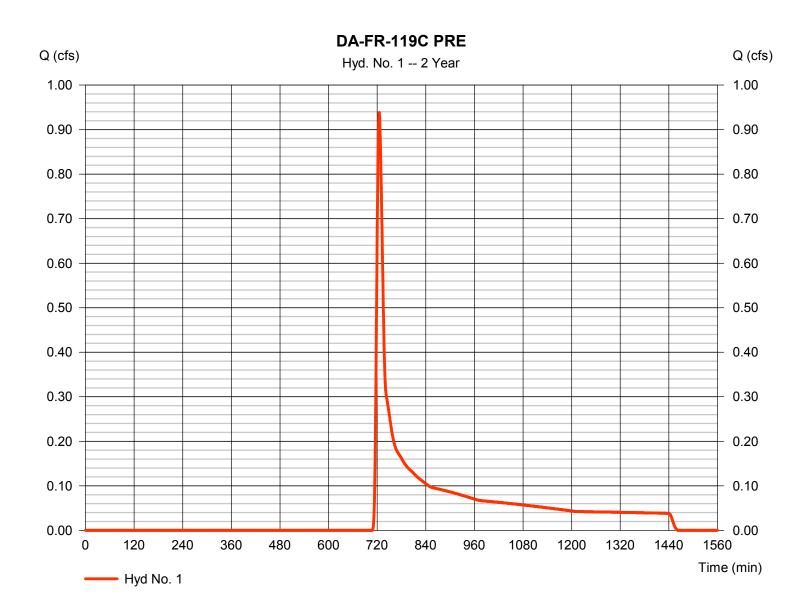
Wednesday, 08 / 16 / 2017

Hyd. No. 1

DA-FR-119C PRE

Hydrograph type = SCS Runoff Peak discharge = 0.938 cfsStorm frequency = 2 yrsTime to peak = 725 min Time interval = 1 min Hyd. volume = 4,002 cuftCurve number Drainage area = 2.430 ac= 56* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 13.50 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = [(0.040 x 82) + (0.720 x 58) + (1.670 x 55)] / 2.430



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

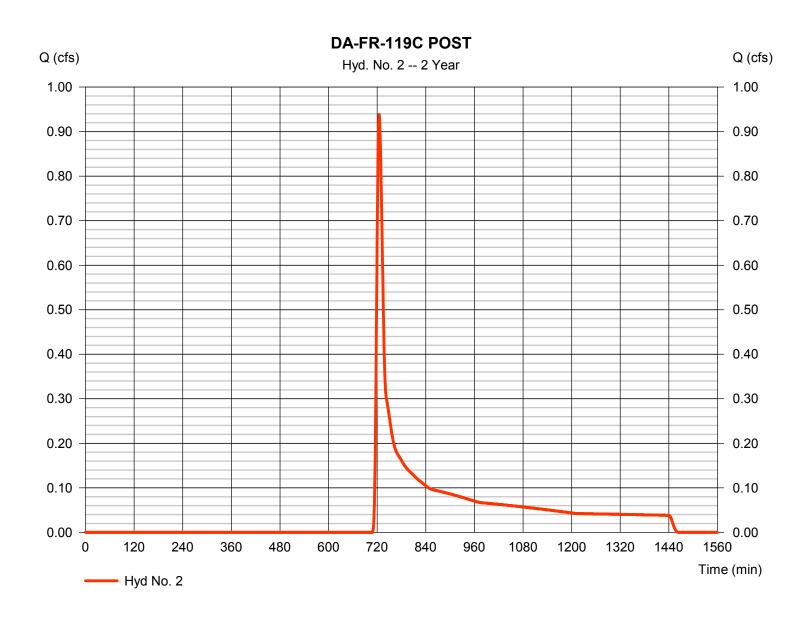
Wednesday, 08 / 16 / 2017

Hyd. No. 2

DA-FR-119C POST

Hydrograph type = SCS Runoff Peak discharge = 0.938 cfsStorm frequency = 2 yrsTime to peak = 725 min Time interval = 1 min Hyd. volume = 4.002 cuftCurve number Drainage area = 2.430 ac= 56* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 13.60 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inStorm duration Shape factor = 484 = 24 hrs

^{*} Composite (Area/CN) = $[(0.070 \times 48) + (0.040 \times 82) + (0.900 \times 58) + (1.420 \times 55)] / 2.430$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

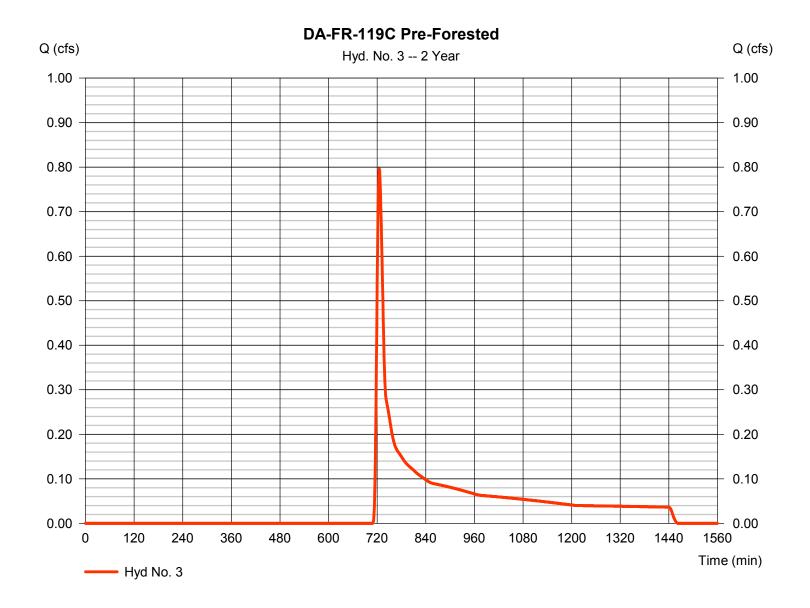
Wednesday, 08 / 16 / 2017

Hyd. No. 3

DA-FR-119C Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 0.797 cfsStorm frequency = 2 yrsTime to peak = 725 min Time interval = 1 min Hyd. volume = 3,666 cuftCurve number Drainage area = 2.430 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 13.50 min = TR55 Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = [(2.430 x 55)] / 2.430



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

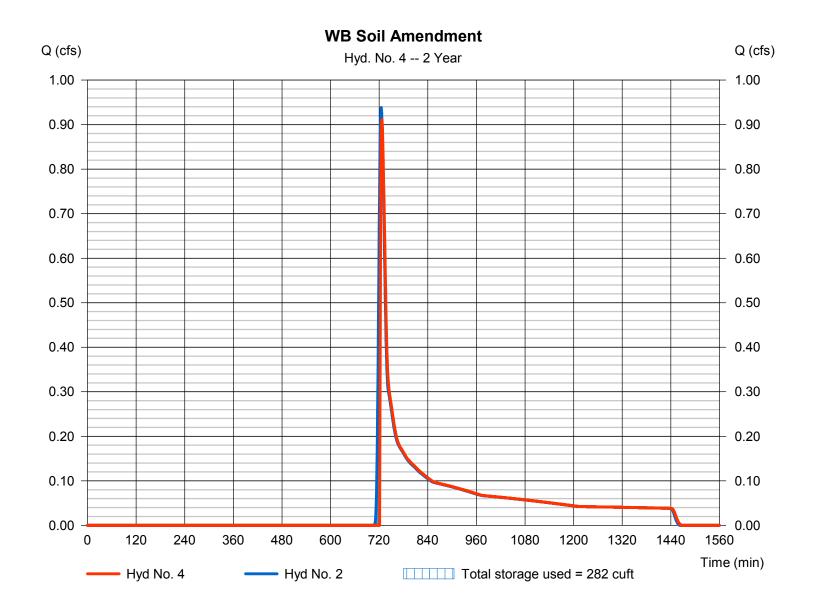
Wednesday, 08 / 16 / 2017

Hyd. No. 4

WB Soil Amendment

Hydrograph type Peak discharge = 0.912 cfs= Reservoir Storm frequency = 2 yrs Time to peak = 727 min Time interval = 1 min Hyd. volume = 3,808 cuft Inflow hyd. No. Max. Elevation = 2 - DA-FR-119C POST $= 907.67 \, \text{ft}$ Reservoir name = Waterbar Soil Amendment Max. Storage = 282 cuft

Storage Indication method used.



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

							1			
łyd. lo.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	4.263	1	723	12,544				DA-FR-119C PRE	
2	SCS Runoff	4.263	1	723	12,544				DA-FR-119C POST	
3	SCS Runoff	3.980	1	723	11,895				DA-FR-119C Pre-Forested	
4	Reservoir	4.237	1	724	12,351	2	907.97	441	WB Soil Amendment	
DΑ	-FR-119c.gp\	W			Return	Period: 10 `	Year	Wednesda	ay, 08 / 16 / 2017	

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

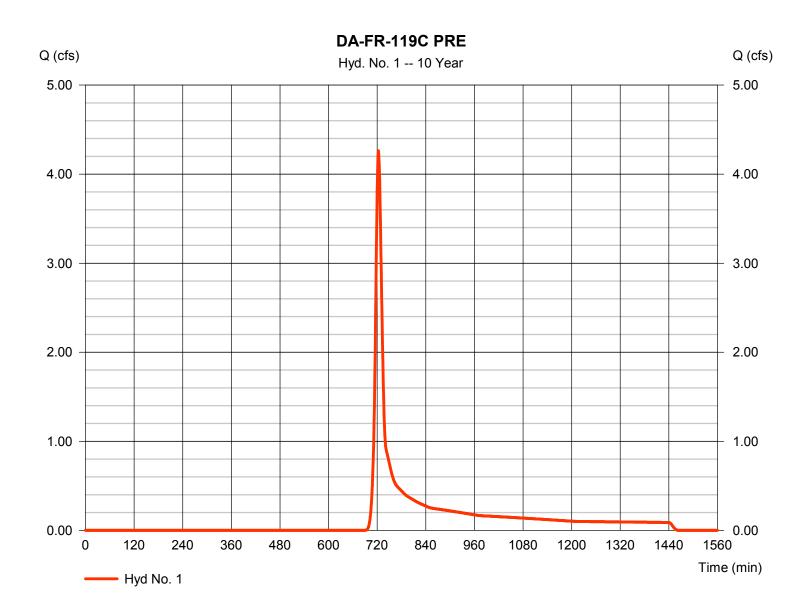
Wednesday, 08 / 16 / 2017

Hyd. No. 1

DA-FR-119C PRE

Hydrograph type = SCS Runoff Peak discharge = 4.263 cfsStorm frequency = 10 yrsTime to peak = 723 min Time interval = 1 min Hyd. volume = 12.544 cuft Drainage area = 2.430 acCurve number = 56* Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 13.50 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = [(0.040 x 82) + (0.720 x 58) + (1.670 x 55)] / 2.430



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

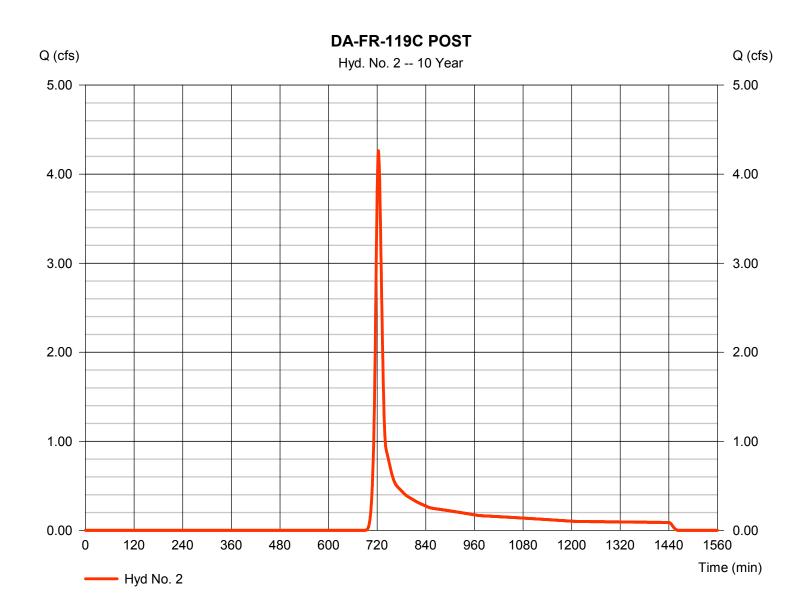
Wednesday, 08 / 16 / 2017

Hyd. No. 2

DA-FR-119C POST

Hydrograph type = SCS Runoff Peak discharge = 4.263 cfsStorm frequency = 10 yrsTime to peak = 723 min Time interval = 1 min Hyd. volume = 12.544 cuft Curve number Drainage area = 2.430 ac= 56* Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 13.60 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inStorm duration = 484 = 24 hrs Shape factor

^{*} Composite (Area/CN) = $[(0.070 \times 48) + (0.040 \times 82) + (0.900 \times 58) + (1.420 \times 55)] / 2.430$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

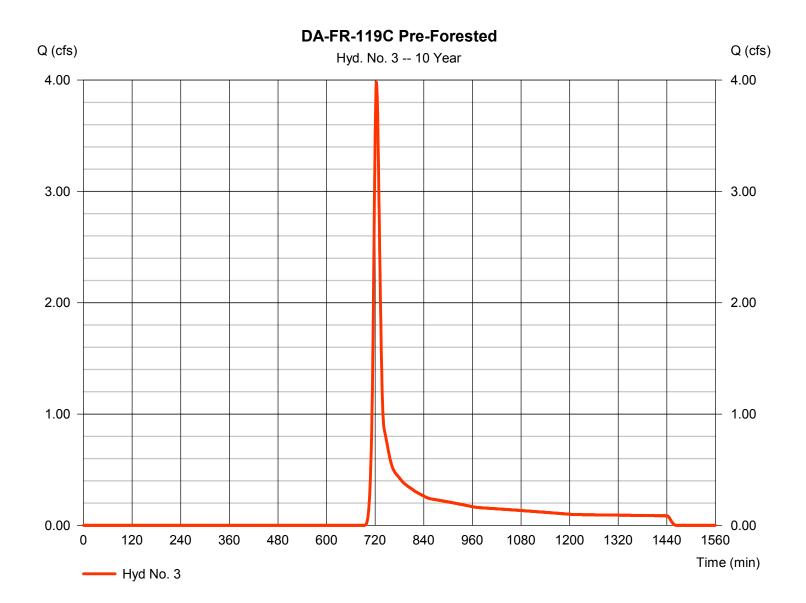
Wednesday, 08 / 16 / 2017

Hyd. No. 3

DA-FR-119C Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 3.980 cfsStorm frequency = 10 yrsTime to peak = 723 min Time interval = 1 min Hyd. volume = 11.895 cuft Curve number Drainage area = 2.430 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 13.50 min = TR55 Total precip. Distribution = Type II = 5.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(2.430 x 55)] / 2.430



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

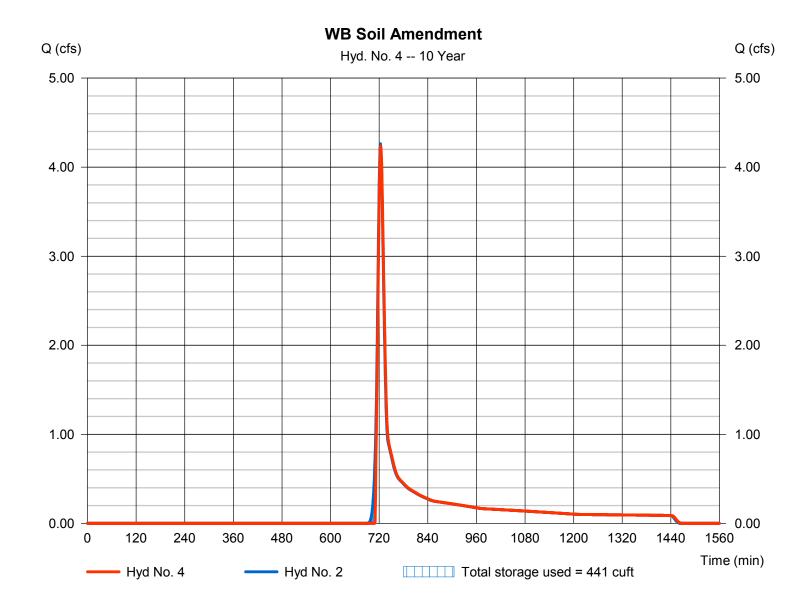
Wednesday, 08 / 16 / 2017

Hyd. No. 4

WB Soil Amendment

Hydrograph type Peak discharge = 4.237 cfs= Reservoir Storm frequency = 10 yrsTime to peak = 724 min Time interval = 1 min Hyd. volume = 12,351 cuftInflow hyd. No. Max. Elevation = 2 - DA-FR-119C POST = 907.97 ftReservoir name = Waterbar Soil Amendment Max. Storage = 441 cuft

Storage Indication method used.



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 16 / 2017

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)								
(Yrs)	В	D	E	(N/A)					
1	0.0000	0.0000	0.0000						
2	69.8703	13.1000	0.8658						
3	0.0000	0.0000	0.0000						
5	79.2597	14.6000	0.8369						
10	88.2351	15.5000	0.8279						
25	102.6072	16.5000	0.8217						
50	114.8193	17.2000	0.8199						
100	127.1596	17.8000	0.8186						

File name: SampleFHA.idf

Intensity = B / (Tc + D)^E

Return	Intensity Values (in/hr)											
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

Precip. file name: P:\MVP ESC&SWM Review\Engineering\SW Calculations\Franklin County.pcp

		F	Rainfall Precipitation Table (in)									
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr				
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00				
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				



ENERGY BALANCE METHOD

ın	

	1-Yr Event					
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)				
Pre-Developed Condition	3.833	13619				
Developed Condition	3.833	13619				
Pre-Developed (Forest) Condition	2.694 10904					

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 1

Calculations:	Check #1:	$Q_{developed} \le IF \times [(Q_{pre-developed} \times RV_{pre-developed}) / RV_{developed}]$ >	<u>Q (cfs)</u> 3.833	≤ OK	Q (cfs) 3.833
	Check #2:	$Q_{developed} \le Q_{pre-developed}$ >	3.833	≤ OK	3.833
	Check #3:	$Q_{developed}$ <u>shall not</u> be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed}$ >	3.833	<u>shall not</u> be required to be ≤	2.157

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 – Manning's n Values for Open Channel Flow

hanr	nel T	уре	Manning n			
			Min.	Normal	Max.	
1.	Exc	avated or Dredged Channels ¹				
	a.	Earth, Straight, and Uniform:				
		Clean, recently completed	0.016	0.018	0.020	
		Clean, after weathering	0.018	0.022	0.025	
		Gravel, uniform section, clean	0.022	0.025	0.030	
		With short grass, few weeds	0.022	0.027	0.033	
	b.	Earth Winding and Sluggish:				
		No vegetation	0.023	0.025	0.030	
		Grass, some weeds	0.025	0.030	0.033	
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040	
		Earth bottom and rubble sides	0.028	0.030	0.035	
		Stony bottom and weedy banks	0.025	0.035	0.040	
		Cobble bottom and clean sides	0.030	0.040	0.050	
	c.	Dragline-Excavated or Dredged:		•		
		No vegetation	0.025	0.028	0.033	
		Light brush on banks	0.035	0.050	0.060	
	d.	Rock Cuts:				
		Smooth and uniform	0.025	0.035	0.040	
		Jagged and irregular	0.035	0.040	0.050	
	e.	Channels not Maintained, Weeds and Brush Uncut:		•		
		Dense weeds, high as flow depth	0.050	0.080	0.120	
		Clean bottom, brush on sides	0.040	0.050	0.080	
		Same as above, highest stage of flow	0.045	0.070	0.110	
		Dense brush, high stage	0.080	0.100	0.140	
2.	Mai	n Channels²				
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.033	
	b.	Same as above, but more stones and weeds	0.030	0.035	0.040	
	c.	Clean, winding, some pools and shoals	0.033	0.040	0.04	
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050	
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.055	
	f.	Same as (d) with more stones	0.045	0.050	0.060	
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080	
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150	

Notes:

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

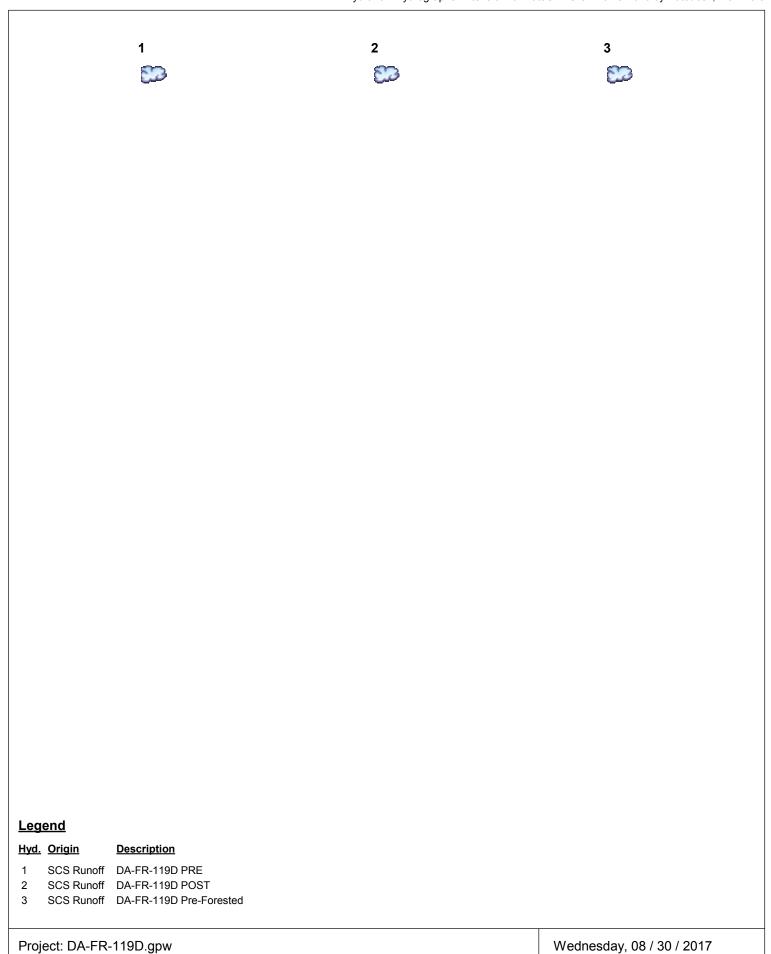
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 30 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-119D PRE	
TR-55 Tc Worksheet	
Hydrograph No. 2, SCS Runoff, DA-FR-119D POST	6
TR-55 Tc Worksheet	
Hydrograph No. 3, SCS Runoff, DA-FR-119D Pre-Forested	
TR-55 Tc Worksheet	9
2 - Year	
Summary Report	. 10
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-119D PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-119D POST	. 12
Hydrograph No. 3, SCS Runoff, DA-FR-119D Pre-Forested	. 13
10 - Year	
Summary Report	. 14
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-119D PRE	. 15
Hydrograph No. 2, SCS Runoff, DA-FR-119D POST	
Hydrograph No. 3, SCS Runoff, DA-FR-119D Pre-Forested	. 17
IDE Donort	40

Watershed Model Schematic

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5



Hydrograph Return Period Recap Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

lyd.	Hydrograph	Inflow				Hydrograph						
No.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description	
1	SCS Runoff		3.833	5.503			15.73				DA-FR-119D PRE	
2	SCS Runoff		3.833	5.503			15.73				DA-FR-119D POST	
3	SCS Runoff		2.694	4.157			13.60				DA-FR-119D Pre-Forested	

Proj. file: DA-FR-119D.gpw

Wednesday, 08 / 30 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

					Hydranow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, inc					
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	3.833	1	725	13,619				DA-FR-119D PRE	
2	SCS Runoff	3.833	1	725	13,619				DA-FR-119D POST	
DA	ED 4405 -				Detroy 5			Wadasad	. 00 / 20 / 2047	
DA	FR-119D.gpv	W			Return P	eriod: 1 Ye	ear	Wednesday, 08 / 30 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

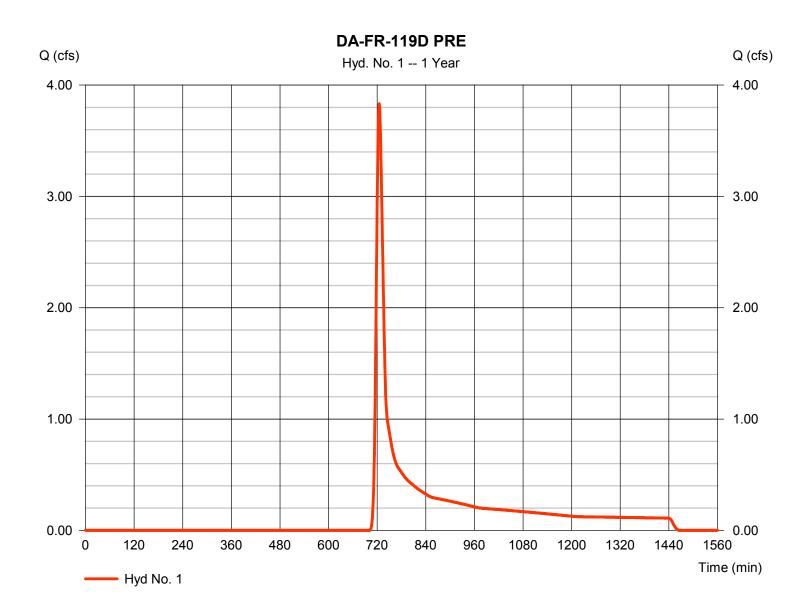
Wednesday, 08 / 30 / 2017

Hyd. No. 1

DA-FR-119D PRE

Hydrograph type = SCS Runoff Peak discharge = 3.833 cfsStorm frequency = 1 yrsTime to peak = 725 min Time interval = 1 min Hyd. volume = 13.619 cuft Drainage area Curve number = 6.110 ac= 64* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.70 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(6.110 x 64)] / 6.110



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1DA-FR-119D PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 5.00 = 13.84	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	13.84
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 631.50 = 11.70 = Unpaved =5.52	i	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.91	+	0.00	+	0.00	=	1.91
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.040 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							15.70 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

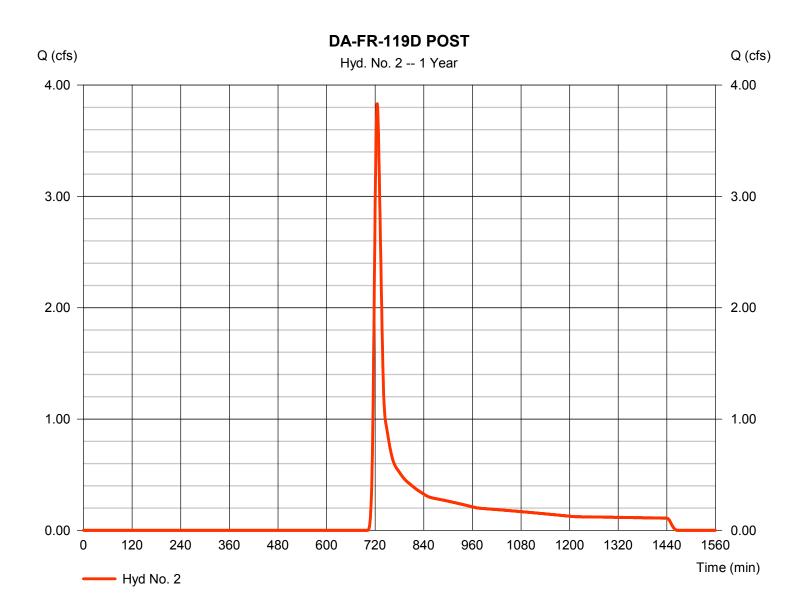
Wednesday, 08 / 30 / 2017

Hyd. No. 2

DA-FR-119D POST

Hydrograph type = SCS Runoff Peak discharge = 3.833 cfsStorm frequency = 1 yrsTime to peak = 725 min Time interval = 1 min Hyd. volume = 13.619 cuft Drainage area Curve number = 6.110 ac= 64*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.80 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(6.110 x 64)] / 6.110



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2DA-FR-119D POST

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>			
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 5.00		0.800 0.0 0.00 0.00		0.011 0.0 0.00 0.00					
Travel Time (min)	= 13.84	+	0.00	+	0.00	=	13.84			
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 491.80 = 12.00 = Unpaved =5.59	d	102.80 11.40 Unpave 5.45	d	0.00 0.00 Paved 0.00					
Travel Time (min)	= 1.47	+	0.31	+	0.00	=	1.78			
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015					
Flow length (ft)	({0})63.9		0.0		0.0					
Travel Time (min)	= 0.22	+	0.00	+	0.00	=	0.22			
Total Travel Time, Tc										

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

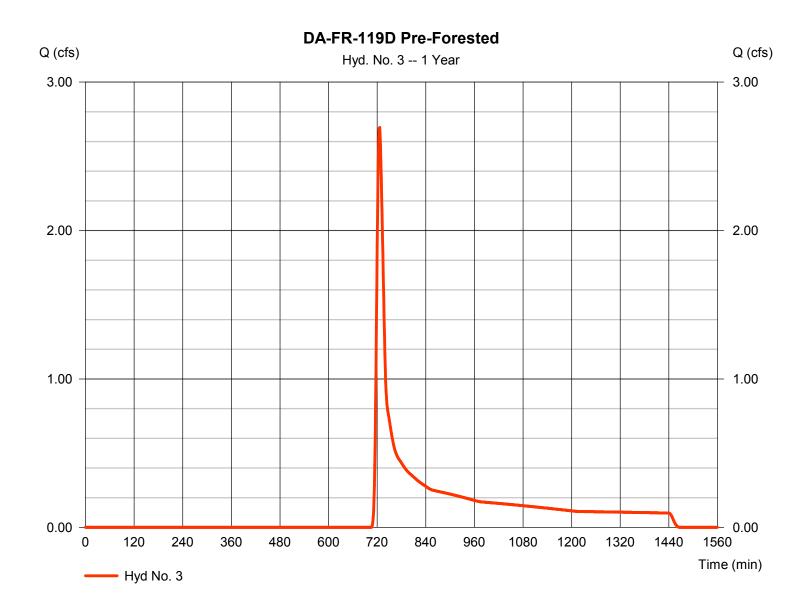
Wednesday, 08 / 30 / 2017

Hyd. No. 3

DA-FR-119D Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 2.694 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 min Hyd. volume = 10.904 cuft Curve number Drainage area = 6.110 ac= 61* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.70 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(3.690 x 55) + (2.420 x 70)] / 6.110



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3DA-FR-119D Pre-Forested

<u>Description</u>	<u>A</u>		<u>B</u>		<u>c</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 5.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 13.84	+	0.00	+	0.00	=	13.84
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 631.50 = 11.70 = Unpaved =5.52	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.91	+	0.00	+	0.00	=	1.91
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.030 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Flow length (ft) Travel Time (min)	({0})0.0 = 0.00	+	0.0 0.00	+	0.0 0.00	=	0.00

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

		Hydraflow Hydrographs E				ydrograpns Exte	graphs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.				
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description		
1	SCS Runoff	5.503	1	724	18,158				DA-FR-119D PRE		
2	SCS Runoff	5.503	1	724	18,158				DA-FR-119D POST		
2 3											
DA-FR-119D.gpw					Return F	Period: 2 Ye	ear	Wednesday	y, 08 / 30 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

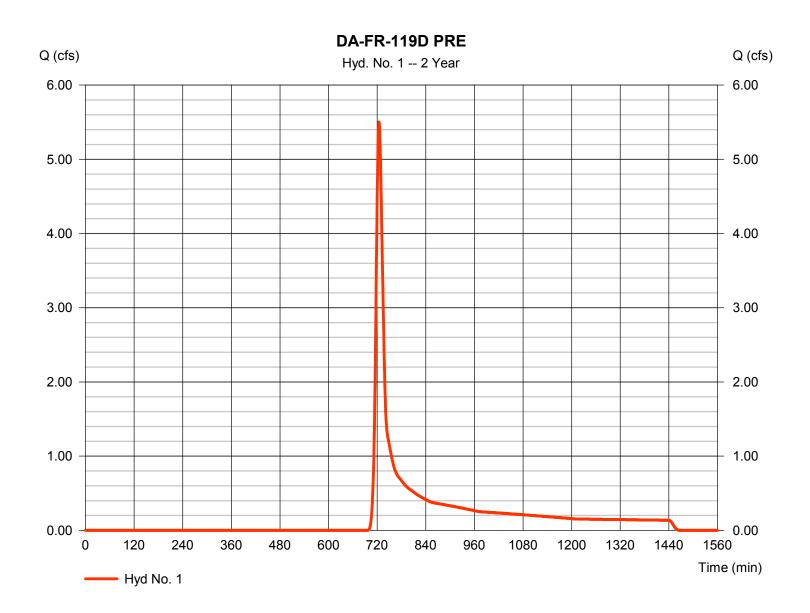
Wednesday, 08 / 30 / 2017

Hyd. No. 1

DA-FR-119D PRE

Hydrograph type = SCS Runoff Peak discharge = 5.503 cfsStorm frequency = 2 yrsTime to peak = 724 min Time interval = 1 min Hyd. volume = 18,158 cuft Curve number Drainage area = 6.110 ac= 64* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.70 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = [(6.110 x 64)] / 6.110



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

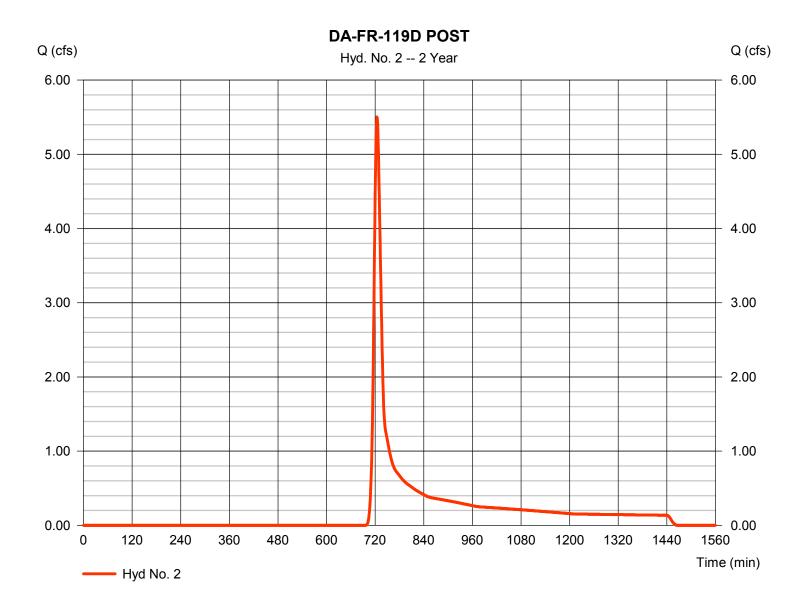
Wednesday, 08 / 30 / 2017

Hyd. No. 2

DA-FR-119D POST

Hydrograph type = SCS Runoff Peak discharge = 5.503 cfsStorm frequency = 2 yrsTime to peak = 724 min Time interval = 1 min Hyd. volume = 18,158 cuft Curve number Drainage area = 6.110 ac= 64* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.80 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = [(6.110 x 64)] / 6.110



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

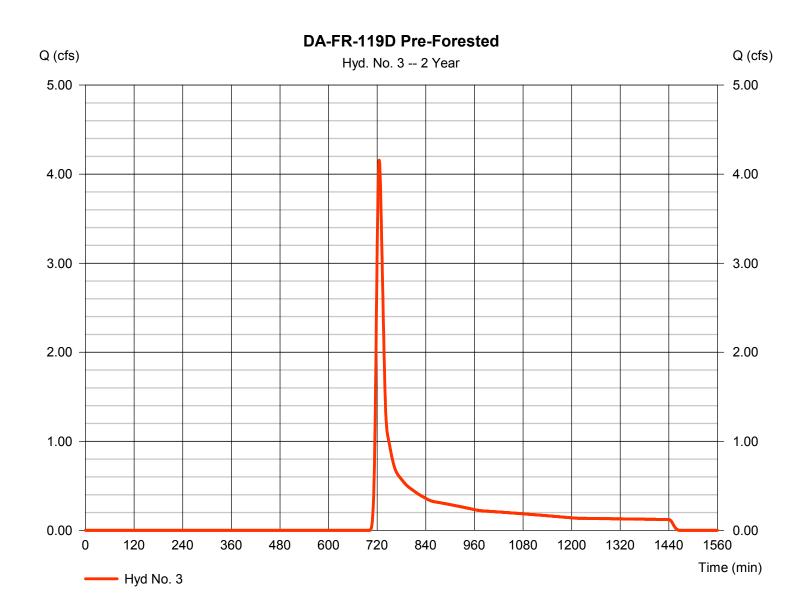
Wednesday, 08 / 30 / 2017

Hyd. No. 3

DA-FR-119D Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 4.157 cfsStorm frequency = 2 yrsTime to peak = 725 min Time interval = 1 min Hyd. volume = 14.936 cuft Curve number Drainage area = 6.110 ac= 61* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.70 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484 = 24 hrs

^{*} Composite (Area/CN) = [(3.690 x 55) + (2.420 x 70)] / 6.110



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

lyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
	(origin)	(0.0)	()	()	(ourt)		(14)	(out)	
1	SCS Runoff	15.73	1	724	46,081				DA-FR-119D PRE
2	SCS Runoff	15.73	1	724	46,081				DA-FR-119D POST
3	SCS Runoff	13.60	1	724	40,591				DA-FR-119D Pre-Forested
DA	-FR-119D.gp	W			Return F	Period: 10 Y	/ear	Wednesda	y, 08 / 30 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

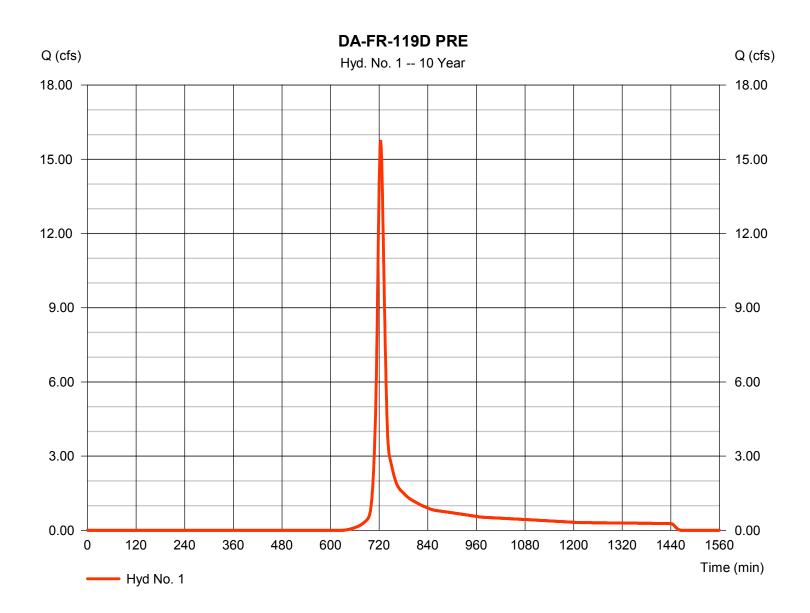
Wednesday, 08 / 30 / 2017

Hyd. No. 1

DA-FR-119D PRE

Hydrograph type = SCS Runoff Peak discharge = 15.73 cfsStorm frequency = 10 yrsTime to peak = 724 min Time interval = 1 min Hyd. volume = 46.081 cuft Drainage area Curve number = 6.110 ac= 64* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.70 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inStorm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(6.110 x 64)] / 6.110



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

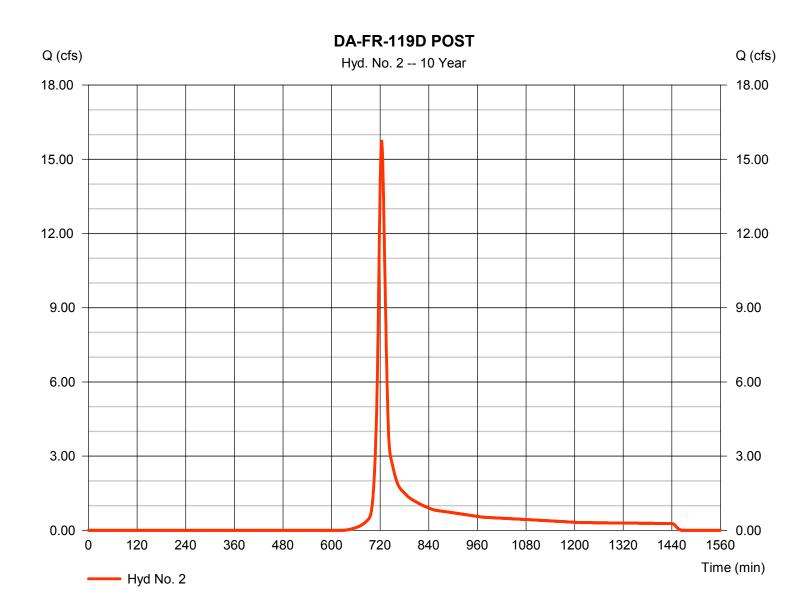
Wednesday, 08 / 30 / 2017

Hyd. No. 2

DA-FR-119D POST

Hydrograph type = SCS Runoff Peak discharge = 15.73 cfsStorm frequency = 10 yrsTime to peak = 724 min Time interval = 1 min Hyd. volume = 46.081 cuft Drainage area Curve number = 6.110 ac= 64* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.80 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(6.110 x 64)] / 6.110



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

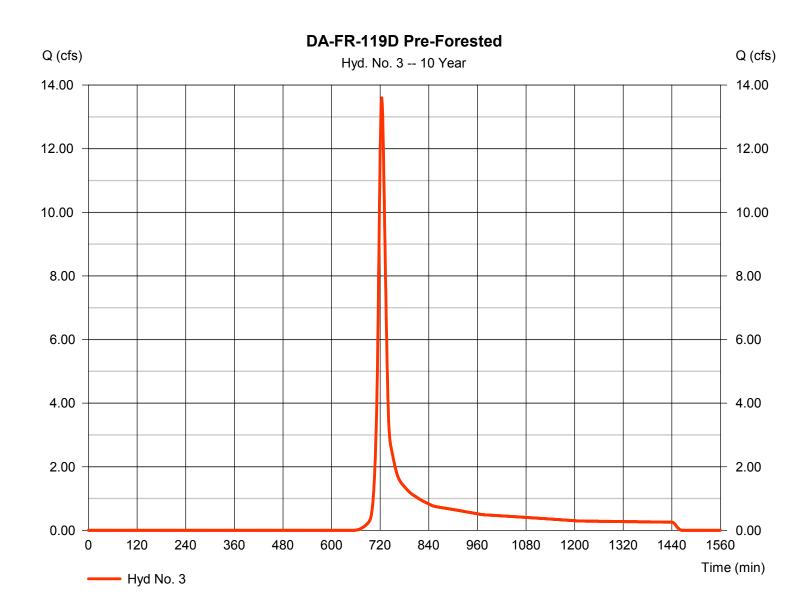
Wednesday, 08 / 30 / 2017

Hyd. No. 3

DA-FR-119D Pre-Forested

Hydrograph type = SCS Runoff Peak discharge = 13.60 cfsStorm frequency = 10 yrsTime to peak = 724 min Time interval = 1 min Hyd. volume = 40.591 cuft Drainage area Curve number = 6.110 ac= 61* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.70 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inStorm duration Shape factor = 484 = 24 hrs

^{*} Composite (Area/CN) = [(3.690 x 55) + (2.420 x 70)] / 6.110



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 30 / 2017

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)										
(Yrs)	В	D	E	(N/A)							
1	0.0000	0.0000	0.0000								
2	69.8703	13.1000	0.8658								
3	0.0000	0.0000	0.0000								
5	79.2597	14.6000	0.8369								
10	88.2351	15.5000	0.8279								
25	102.6072	16.5000	0.8217								
50	114.8193	17.2000	0.8199								
100	127.1596	17.8000	0.8186								

File name: SampleFHA.idf

Intensity = B / (Tc + D)^E

Return		Intensity Values (in/hr)												
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60		
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70		
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15		
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46		
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91		
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25		
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60		

Tc = time in minutes. Values may exceed 60.

Precip. file name: P:\MVP ESC&SWM Review\Engineering\SW Calculations\Franklin County.pcp

		Rainfall Precipitation Table (in)									
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr			
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00			
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			

DA-FR-120

DA-FR-120 is located in a meadow and forested areas with rolling slopes and contains agricultural land, existing dirt road and gravel road. A stream crosses the drainage area. No new impervious area is proposed within DA-FR-120. The total phosphorus load reduction required for DA-FR-120 is -0.64 lb/yr. Multiple points of analysis were evaluated within DA-FR-120 to evaluate the effects on each receiving stream/channel following construction. Specifically, DA-FR-120 was sub-divided into six sub-drainage areas (sub-areas A through F).

Sub-areas 120A and 120F contain both agricultural and non-agricultural areas within the limits of disturbance (LOD). Pre-construction agricultural areas will be returned to agricultural land use (i.e., returned to crop production, in identical condition) following construction. In non-agricultural areas, land use will be restored following construction as noted in the Stormwater Management (SWM) Narrative and the Annual Standards and Specifications. Agricultural areas within the LOD are included in the SWM quality analysis and the total permanent Right of Way (ROW) is analyzed via VRRM; in these calculations agricultural areas are considered "Forest/Open Space".

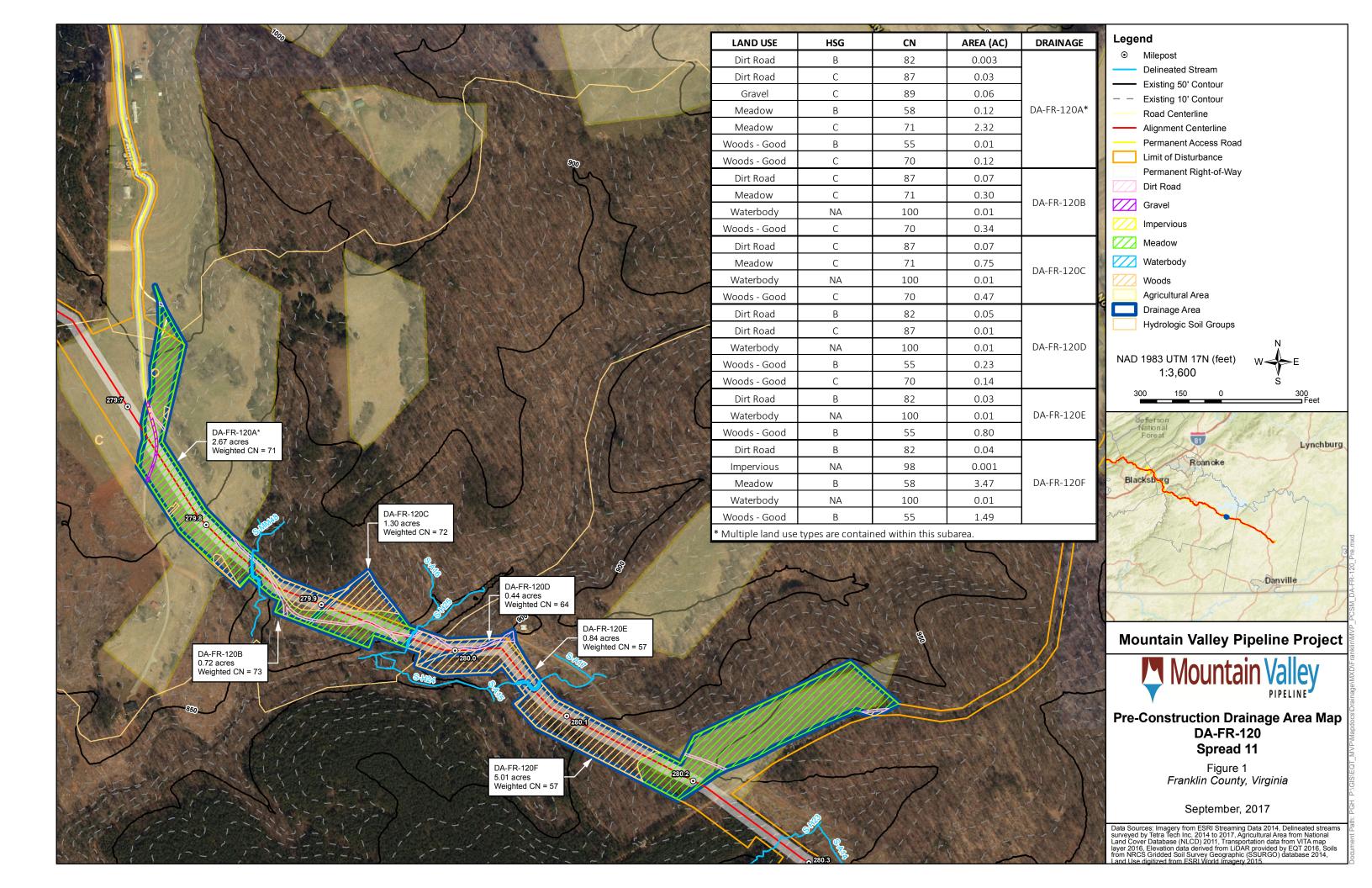
Stormwater quantity is met via the energy balance method for each of the six sub-areas DA-FR-120A through DA-FR-120F. Agricultural areas within the study area are included in the SWM analysis, but an Improvement Factor (IF) of 1.0 is used when applying the Energy Balance Method. This improvement factor is used to account for the exemption of agricultural areas (§ 62.1-44.15:34 and 9VAC25-870-300) since such areas will be returned to agricultural land use (i.e., returned to crop production, in identical condition) following construction. BMPs utilized in the sub-areas included four water bars for 120B, five water bars for 120C and five water bars for 120F.

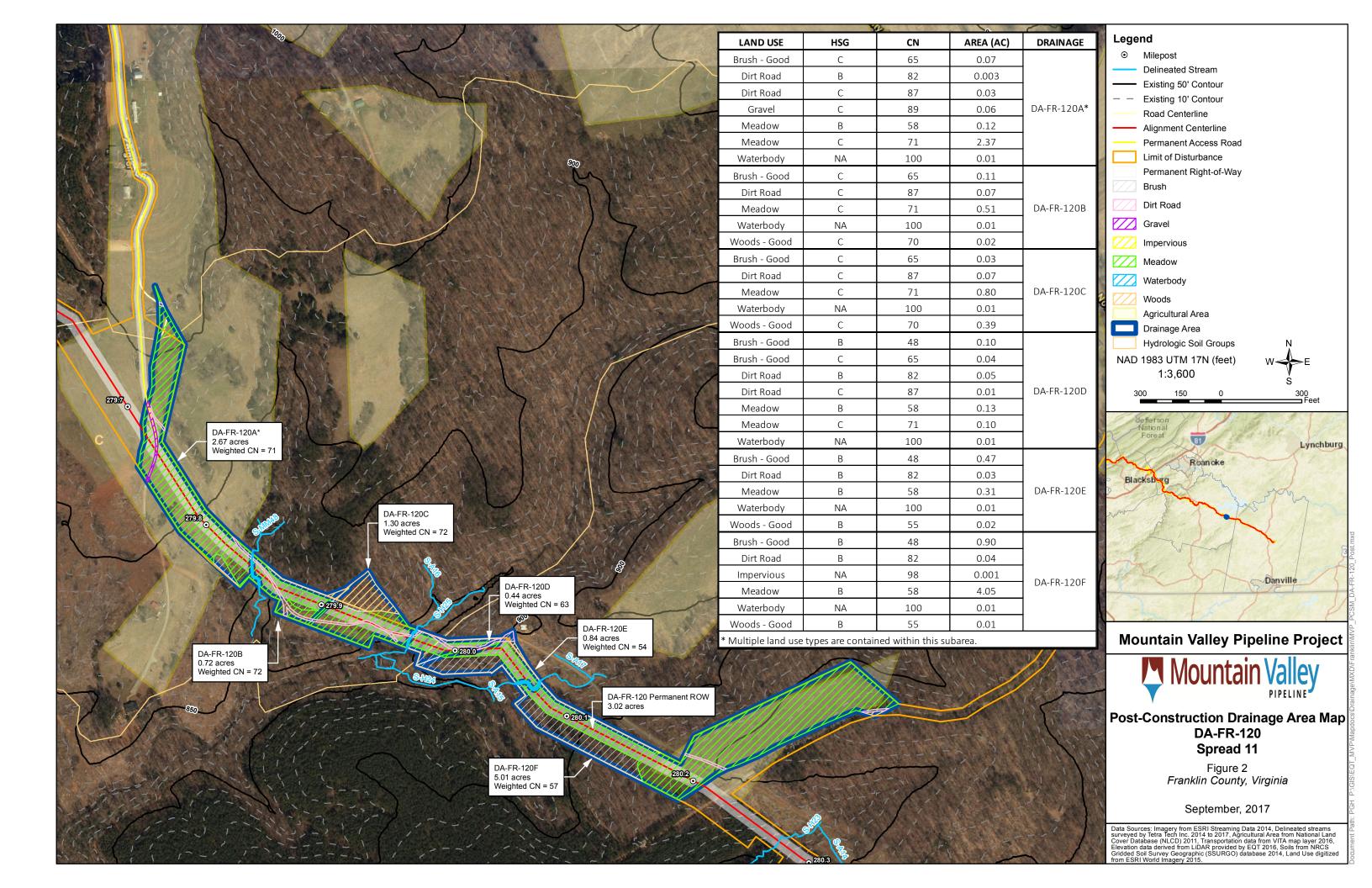
In addition, the Hydraflow Hydrograph's 10-year 24-hour peak discharge results indicate a reduction in flows ranging from 0.00 to 0.56 cfs for all of the sub-drainage areas (as seen in table below).

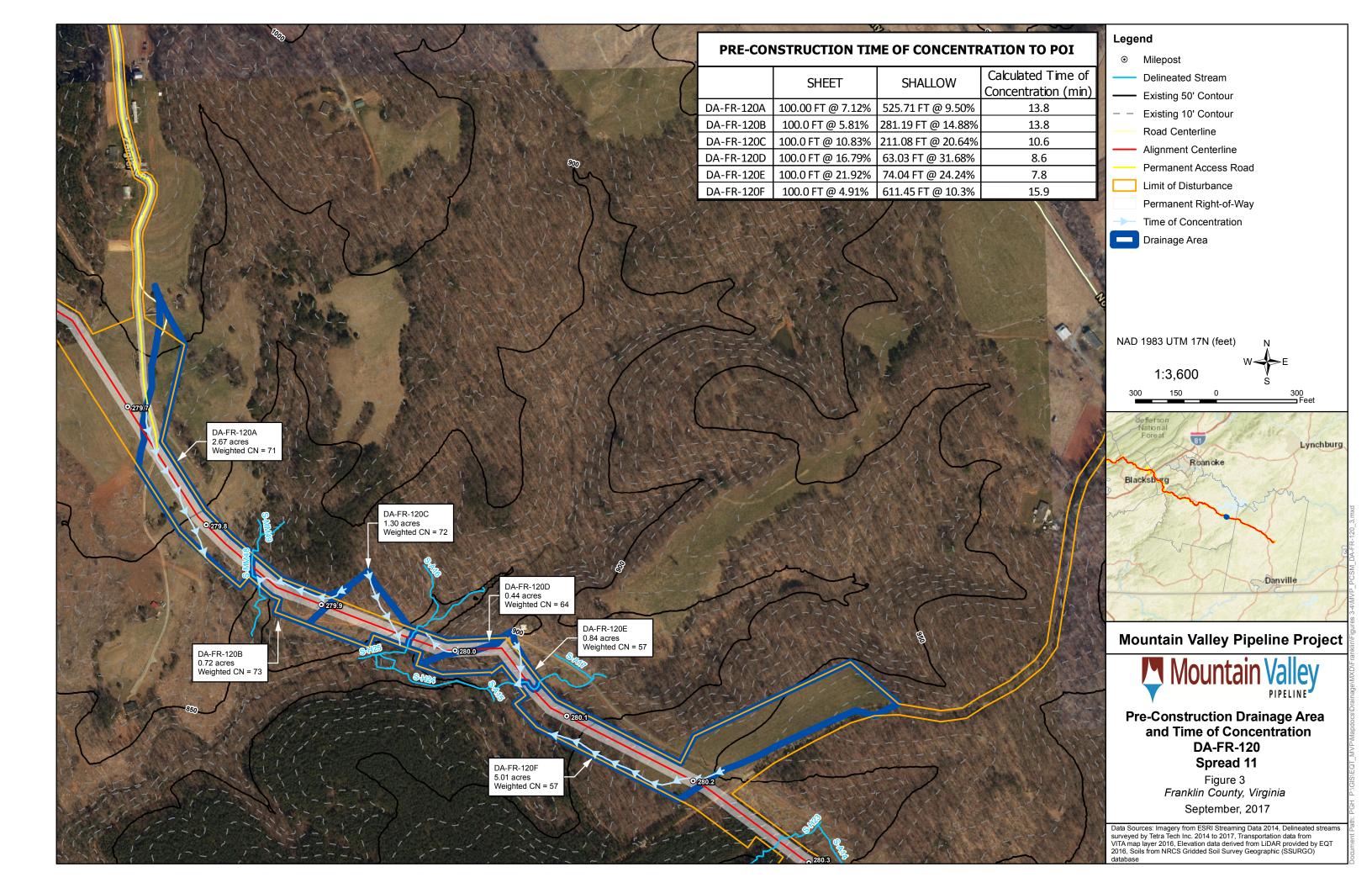
Sub Area	Pre Peak Flow,	Post Peak Flow, Q	Flow differential
	10-yr Q (cfs)	10-yr (cfs)	
DA-FR-120A	9.57	9.57	0.00
DA-FR-120B	2.67	2.65	-0.02
DA-FR-120C	5.46	5.39	-0.07
DA-FR-120D	1.44	1.38	-0.06
DA-FR-120E	2.04	1.48	-0.56
DA-FR-120F	8.87	8.80	-0.07

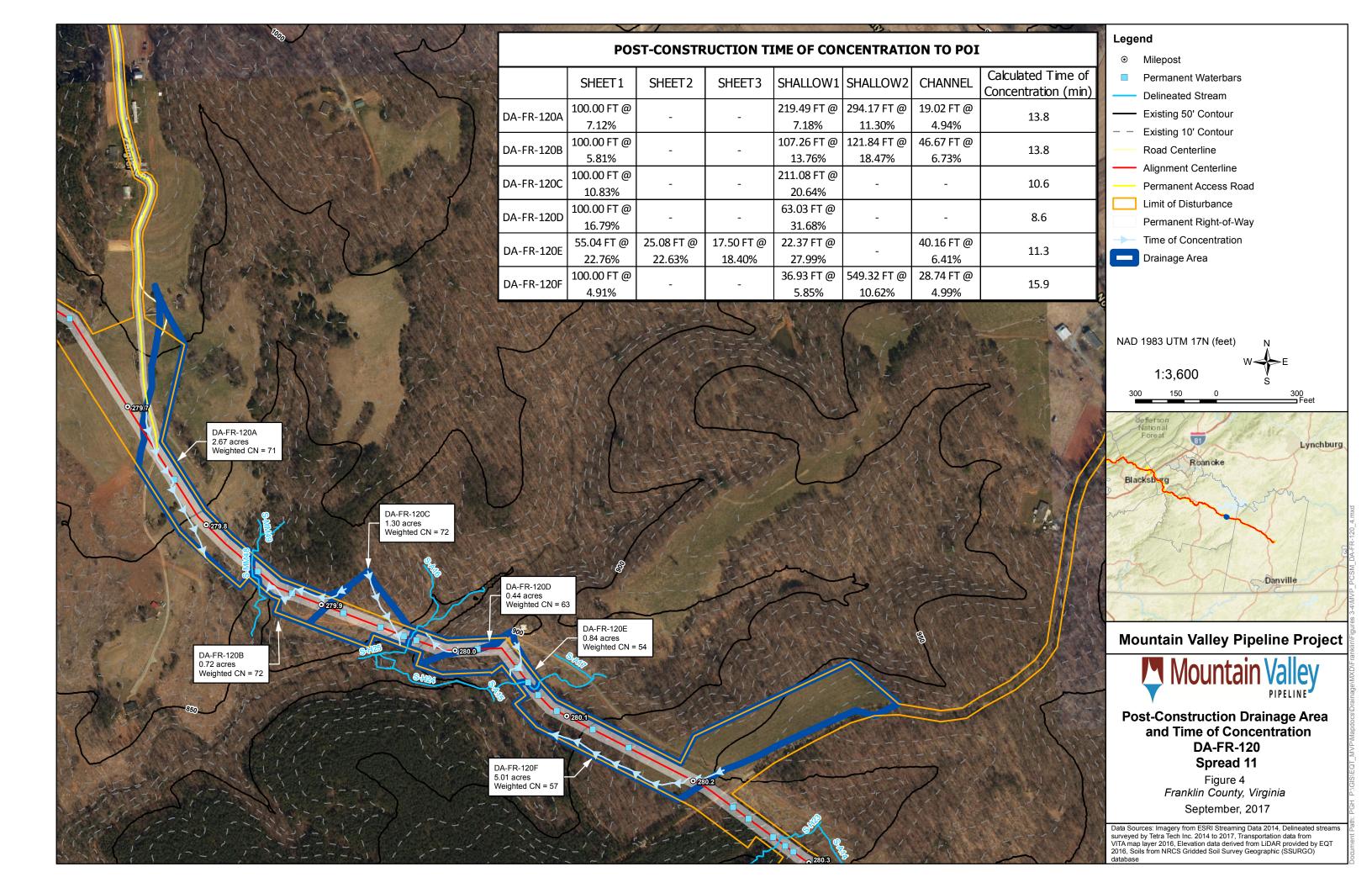
Figures and calculations for each of the sub-areas for DA-FR-120 follow. See Appendix D of the Annual Standards and Specifications for further detail on stormwater methodology.

Note that the sub-area 120A had a time of concentration flow path for the Post-Construction condition that accounted for a permanent water bar that is not cited within the drainage area. This permanent water bar was removed because it was cited in an agricultural area, and resulting changes to the stormwater calculations were flagged during the QA/QC review process. This change was considered to be inconsequential because no stormwater BMPs are sited in DA-FR-120A, so the resulting changes were not made to the stormwater calculations prior to submittal.









DEQ Virginia Runoff Reduction Method Re-Development Compliance Spreadsheet - Version 3.0

BMP Design Specifications List: 2013 Draft Stds & Specs

Site Summary - Linear Development Project***

Total Rainfall (in):	43
Total Disturbed Acreage:	3.02

Site Land Cover Summary

Pre-ReDevelopment Land Cover (acres)

c c = c · c · c p · · · c · · c · c · c · c ·	,					
\circ	A soils	B Soils	C Soils	D Soils	Totals	% of Total
Forest/Open (acres)	0.00	1.30	1.54	0.00	2.84	94
Managed Turf (acres)	0.00	0.00	0.00	0.00	0.00	0
Impervious Cover (acres)	0.00	0.04	0.13	0.00	0.17	6
_			<u> </u>		3.01	100

Post-ReDevelopment Land Cover (acres)

<i>00000000000000000000000000000000000</i>	A soils	B Soils	C Soils	D Soils	Totals	% of Total
Forest/Open (acres)	0.00	1.30	1.54	0.00	2.84	94
Managed Turf (acres)	0.00	0.00	0.00	0.00	0.00	0
Impervious Cover (acres)	0.00	0.04	0.13	0.00	0.17	6
* Forest/Open Space areas must be protected in	3.01	100				

Site Tv and Land Cover Nutrient Loads

	Final Post-Development (Post-ReDevelopment & New Impervious)	Post- ReDevelopment	Post- Development (New Impervious)	Adjusted Pre- ReDevelopment
Site Rv	0.09	0.09		0.09
Treatment Volume (ft ³)	951	951		951
TP Load (lb/yr)	0.60	0.60		0.60
	Describes TD Least (II. /c.u.).	1 22 44 *	*Daduction balance	dayalanmant laad limita

TP Load per acre (lb/acre/yr)	TP Load per acre	Load per acre (lb/acre/yr)
0.20	0.20	0.20

Final Post-Development | Post-ReDevelopment TP

Baseline TP Load (lb/yr):

1.2341*

*Reduction below new development load limitation not required

Total TP Load Reduction Required (lb/yr)	-0.64	N/A***	N/A***
--	-------	--------	--------

***This is a linear development project

	Final Post-Development Load (Post-ReDevelopment & New Impervious)	Pre- ReDevelopment
TN Load (lb/yr)	4.28	4.28

Site Compliance Summary - ***Linear Development Project

Maximum % Reduction Required Below	20%	* Note: % Reduction will reduce post-development TP load to less than or equal to baseline load of 1.23 lb/yr (0.41 lb/ac/yr)
Pre-ReDevelopment Load	20%	[Post-Dev Reduction Requirement = Post-Dev TP load - baseline load of 1.23 lb/yr], baseline load = site area x 0.41 lb/ac/yr

Total Runoff Volume Reduction (ft ³)

Total TP Load Reduction Achieved (lb/yr)	0.00
Total TN Load Reduction Achieved (lb/yr)	0.00
Remaining Post Development TP Load (lb/yr)	0.60
Remaining TP Load Reduction (lb/yr) Required	0.00

** TARGET TP REDUCTION EXCEEDED BY 0.64 LB/YEAR **

.._..

^{*}Reduction below new development load limitation not required



ENERGY BALANCE METHOD

	n		

	1-Y	r Event
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)
Pre-Developed Condition	3.190	9099
Developed Condition	3.190	9099
Pre-Developed (Forest) Condition	0.440	2724

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF =

Calculations:	Check #1:	$Q_{developed} \le IF \times \left[\left(Q_{pre-developed} \times RV_{pre-developed} \right) / RV_{developed} \right]>$	3.190	≤ OK	3.190
	Check #2:	Q _{developed} ≤ Q _{pre-developed} >	3.190	≤ OK	3.190
	Check #3:	$Q_{\text{developed } shall \ not}$ be required to be $\leq (Q_{\text{forest}} \times RV_{\text{forest}}) / RV_{\text{developed}} \longrightarrow$	3.190	shall not be required to be ≤	0.132

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 – Manning's n Values for Open Channel Flow

nan	nel T	уре		Manning n	
			Min.	Normal	Max.
1.	Exc	avated or Dredged Channels ¹			
	a.	Earth, Straight, and Uniform:			
		Clean, recently completed	0.016	0.018	0.020
		Clean, after weathering	0.018	0.022	0.025
		Gravel, uniform section, clean	0.022	0.025	0.030
		With short grass, few weeds	0.022	0.027	0.033
	b.	Earth Winding and Sluggish:			
		No vegetation	0.023	0.025	0.030
		Grass, some weeds	0.025	0.030	0.033
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040
		Earth bottom and rubble sides	0.028	0.030	0.03
		Stony bottom and weedy banks	0.025	0.035	0.040
		Cobble bottom and clean sides	0.030	<mark>0.040</mark>	0.050
	c.	Dragline-Excavated or Dredged:			
		No vegetation	0.025	0.028	0.03
		Light brush on banks	0.035	0.050	0.060
	d.	Rock Cuts:			
		Smooth and uniform	0.025	0.035	0.040
		Jagged and irregular	0.035	0.040	0.050
	e.	Channels not Maintained, Weeds and Brush Uncut:			
		Dense weeds, high as flow depth	0.050	0.080	0.120
		Clean bottom, brush on sides	0.040	0.050	0.080
		Same as above, highest stage of flow	0.045	0.070	0.110
		Dense brush, high stage	0.080	0.100	0.140
2.	Mai	n Channels²			
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.033
	b.	Same as above, but more stones and weeds	0.030	0.035	0.040
	C.	Clean, winding, some pools and shoals	0.033	0.040	0.04
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.05
	f.	Same as (d) with more stones	0.045	0.050	0.060
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

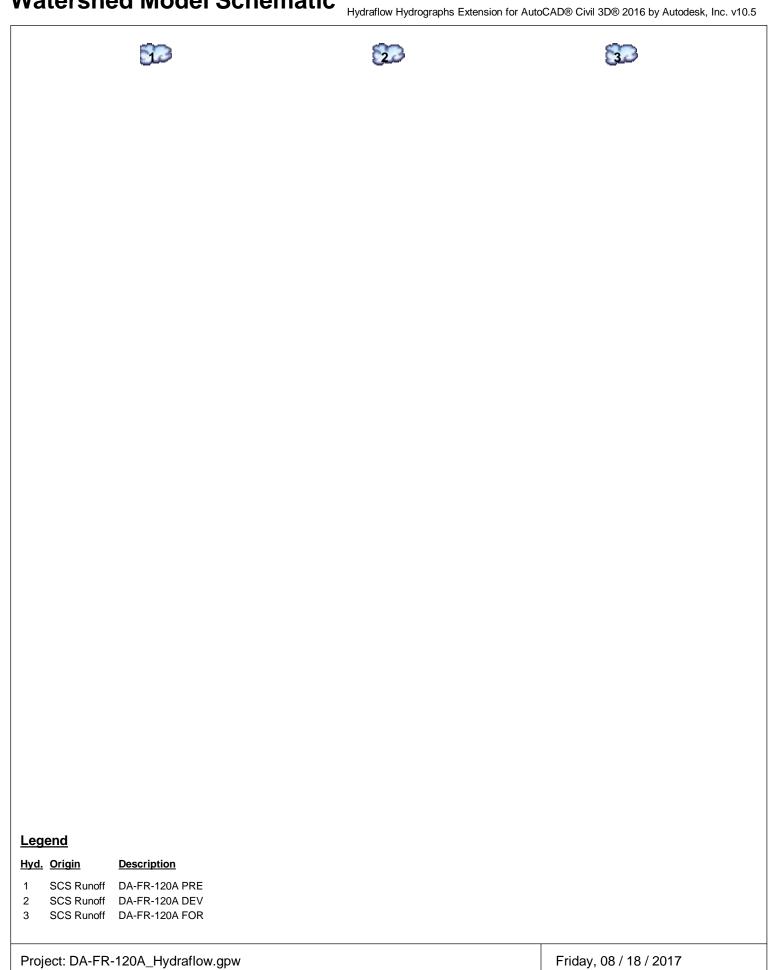
²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-120A PRE	
TR-55 Tc Worksheet	
Hydrograph No. 2, SCS Runoff, DA-FR-120A DEV	
TR-55 Tc Worksheet	
Hydrograph No. 3, SCS Runoff, DA-FR-120A FOR	
TR-55 Tc Worksheet	9
2 - Year	
Summary Report	10
Hydrograph Reports	11
Hydrograph No. 1, SCS Runoff, DA-FR-120A PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-120A DEV	12
Hydrograph No. 3, SCS Runoff, DA-FR-120A FOR	13
10 - Year	
	4.4
Summary ReportHydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-120A PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-120A PRE	
Hydrograph No. 3, SCS Runoff, DA-FR-120A DEV	
Tydrograph No. 5, 500 Nahon, DA-1 N-120A 1 ON	17
IDE Papart	10

Watershed Model Schematic



Hydrograph Return Period Recap Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	Hydrograph	Inflow				Hydrograph					
lo.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		3.190	4.144			9.565				DA-FR-120A PRE
2	SCS Runoff		3.190	4.144			9.565				DA-FR-120A DEV
3	SCS Runoff		0.440	0.875			4.374				DA-FR-120A FOR

Proj. file: DA-FR-120A_Hydraflow.gpw

Friday, 08 / 18 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	3.190	1	723	9,099				DA-FR-120A PRE
2	SCS Runoff	3.190	1	723	9,099				DA-FR-120A DEV
2									
DA	-FR-120A_H _y	/draflow.g	jpw	1	Return F	Return Period: 1 Year			18 / 2017

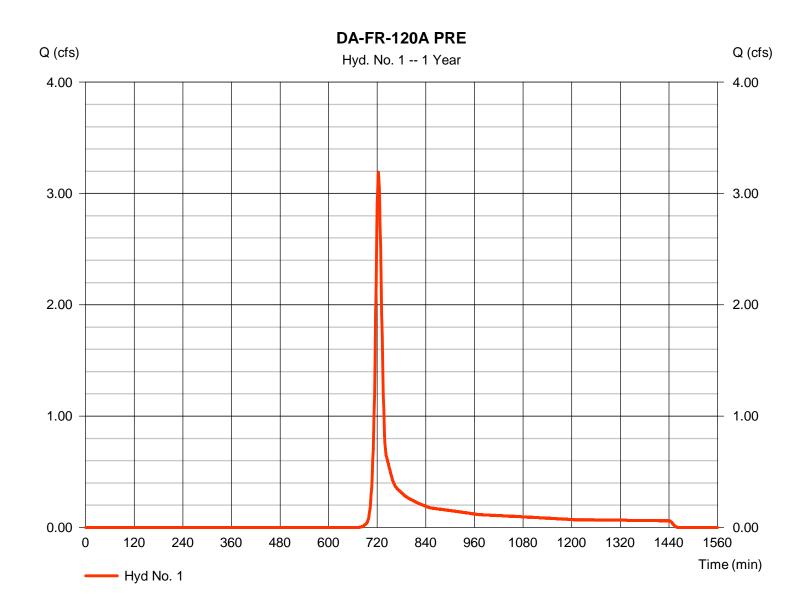
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 1

DA-FR-120A PRE

= SCS Runoff Hydrograph type Peak discharge = 3.190 cfsStorm frequency Time to peak = 723 min = 1 yrsTime interval = 1 min Hyd. volume = 9.099 cuftDrainage area Curve number = 2.670 ac= 71 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 13.80 \, \text{min}$ Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1

DA-FR-120A PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 7.12 = 12.02	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	12.02
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 525.71 = 9.50 = Unpaved =4.97	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.76	+	0.00	+	0.00	=	1.76
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							13.80 min

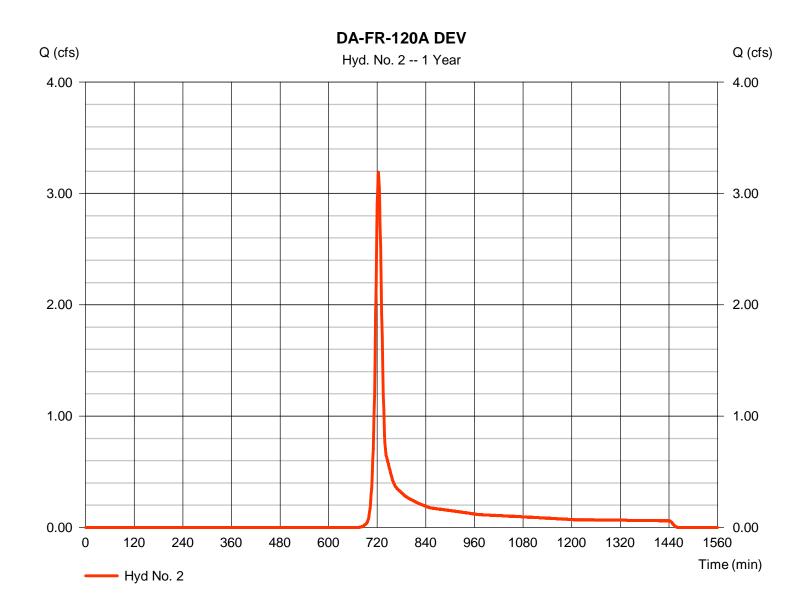
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 2

DA-FR-120A DEV

= SCS Runoff Hydrograph type Peak discharge = 3.190 cfsStorm frequency Time to peak = 723 min = 1 yrsTime interval = 1 min Hyd. volume = 9.099 cuftDrainage area Curve number = 2.670 ac= 71 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 13.80 \, \text{min}$ Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2

DA-FR-120A DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 7.12 = 12.02	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	=	12.02
, ,	- 12.02	•	0.00	•	0.00	_	12.02
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 219.49 = 7.18 = Unpaved =4.32	t	294.17 11.30 Unpave 5.42	ed	0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.85	+	0.90	+	0.00	=	1.75
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value	= 4.47 = 5.00 = 0.040		0.00 0.00 0.015		0.00 0.00 0.015		
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 4.47 = 5.00 = 0.040 =4.86	+	0.00 0.00 0.015 0.00	+	0.00 0.00 0.015 0.00	=	0.07

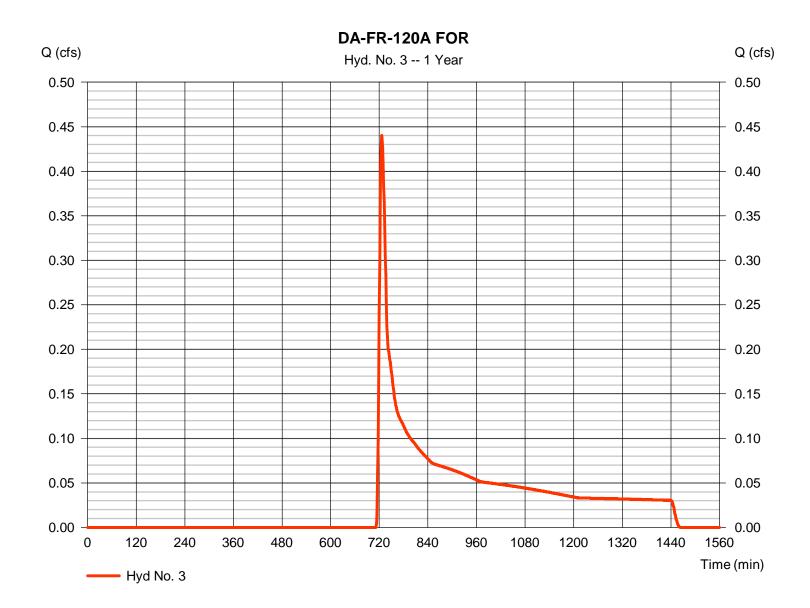
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 3

DA-FR-120A FOR

Hydrograph type = SCS Runoff Peak discharge = 0.440 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 min Hyd. volume = 2.724 cuftDrainage area Curve number = 2.670 ac= 55 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 13.80 \, \text{min}$ Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3

DA-FR-120A FOR

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>			
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 7.12 = 12.02		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		12.02			
Travel Time (min)	= 12.02	+	0.00	+	0.00	=	12.02			
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 525.71 = 9.50 = Unpave =4.97	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00					
Travel Time (min)	= 1.76	+	0.00	+	0.00	=	1.76			
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.400 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015					
Flow length (ft)	({0})0.0		0.0		0.0					
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00			
Total Travel Time, Tc										

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

lyd. lo.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	4.144	1	723	11,563				DA-FR-120A PRE
2	SCS Runoff	4.144	1	723	11,563				DA-FR-120A DEV
2 3	SCS Runoff	0.875	1 1	723	11,563 4,028				DA-FR-120A DEV DA-FR-120A FOR
)A	-FR-120A_H	vdraflow o	wapw		Return F	Period: 2 Ye	 ear	Friday, 08 /	18 / 2017

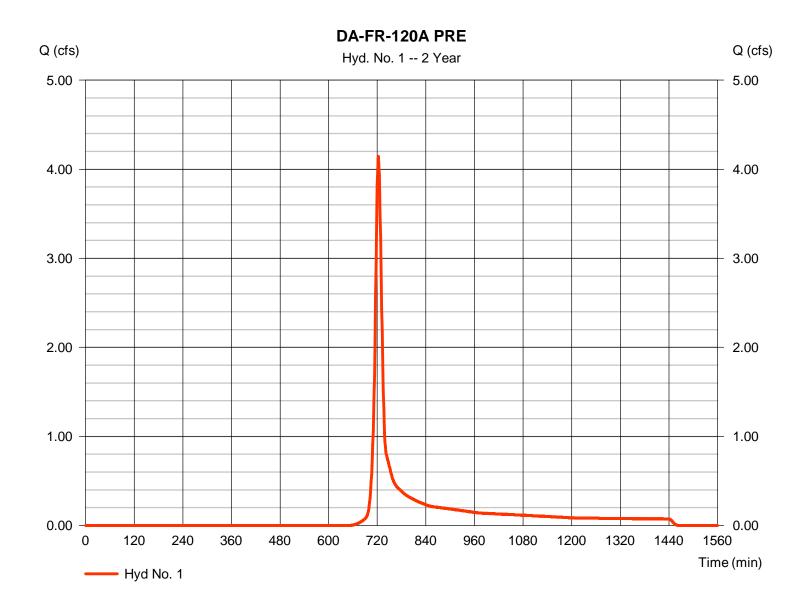
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 1

DA-FR-120A PRE

= SCS Runoff Hydrograph type Peak discharge = 4.144 cfsStorm frequency Time to peak = 723 min = 2 yrsTime interval = 1 min Hyd. volume = 11,563 cuftDrainage area Curve number = 2.670 ac= 71 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 13.80 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



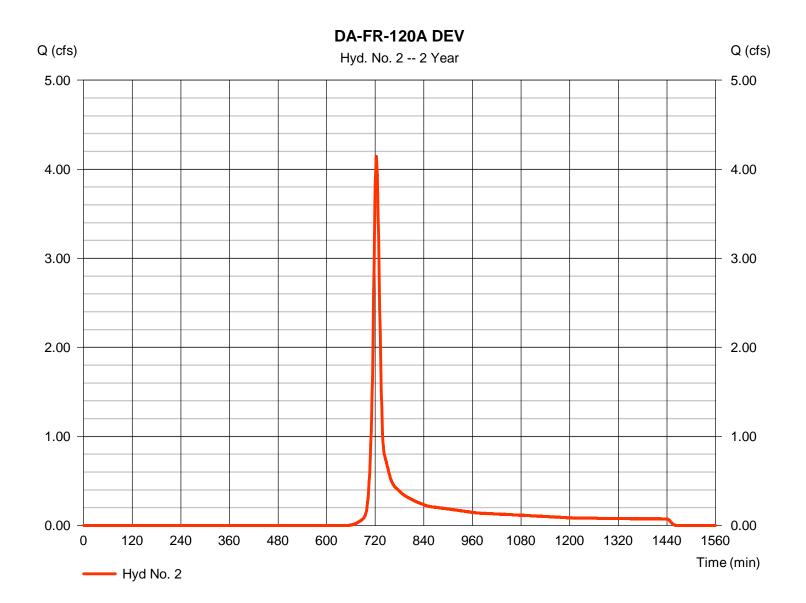
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 2

DA-FR-120A DEV

= SCS Runoff Hydrograph type Peak discharge = 4.144 cfsStorm frequency Time to peak = 723 min = 2 yrsTime interval = 1 min Hyd. volume = 11,563 cuftDrainage area Curve number = 2.670 ac= 71 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 13.80 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



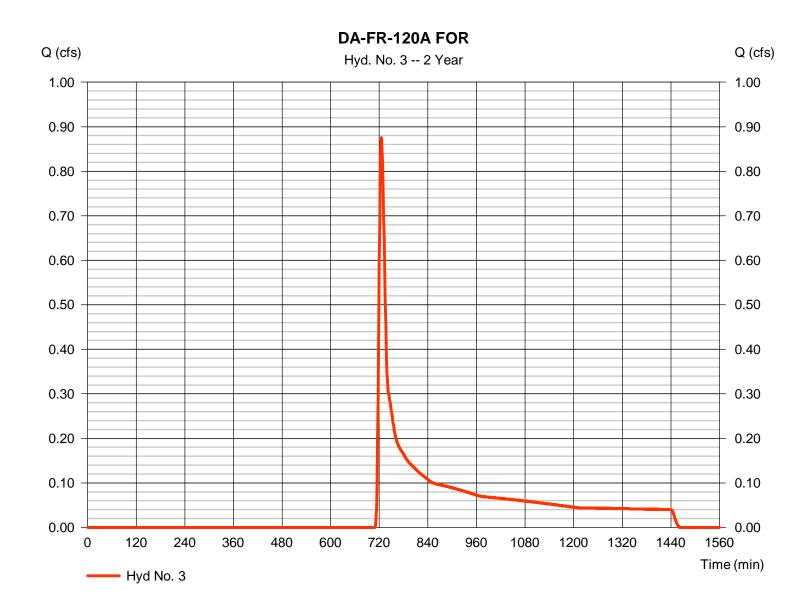
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 3

DA-FR-120A FOR

Hydrograph type = SCS Runoff Peak discharge = 0.875 cfsStorm frequency Time to peak = 725 min = 2 yrsTime interval = 1 min Hyd. volume = 4,028 cuftDrainage area Curve number = 2.670 ac= 55 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 13.80 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	9.565	1	722	25,771				DA-FR-120A PRE
2	SCS Runoff	9.565	1	722	25,771				DA-FR-120A DEV
3	SCS Runoff	4.374	1	723	13,070				DA-FR-120A FOR
DA	-FR-120A_H	ydraflow.ç	gpw		Return F	Period: 10 \	/ear	Friday, 08 /	18 / 2017

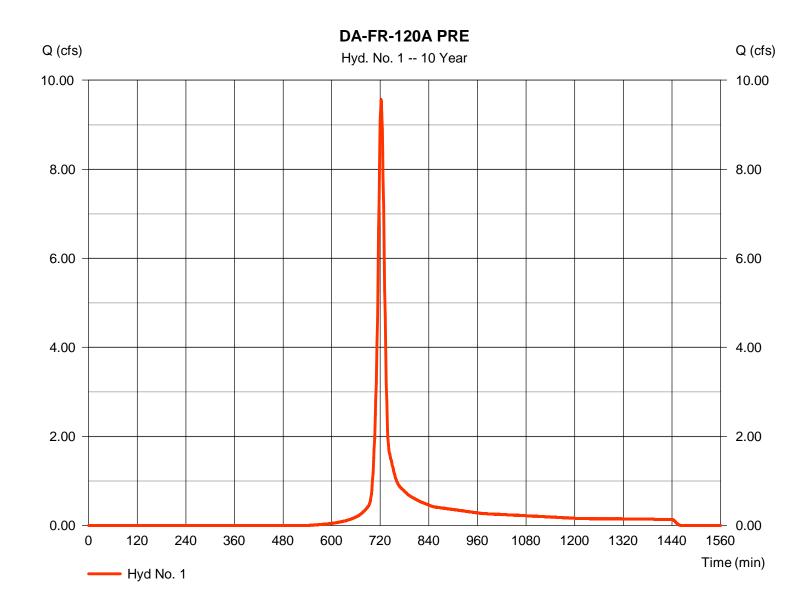
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 1

DA-FR-120A PRE

= SCS Runoff Hydrograph type Peak discharge = 9.565 cfsStorm frequency Time to peak = 722 min = 10 yrsTime interval = 1 min Hyd. volume = 25,771 cuftDrainage area Curve number = 2.670 ac= 71 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 13.80 \, \text{min}$ Total precip. Distribution = 5.70 in= Type II Storm duration = 24 hrs Shape factor = 484



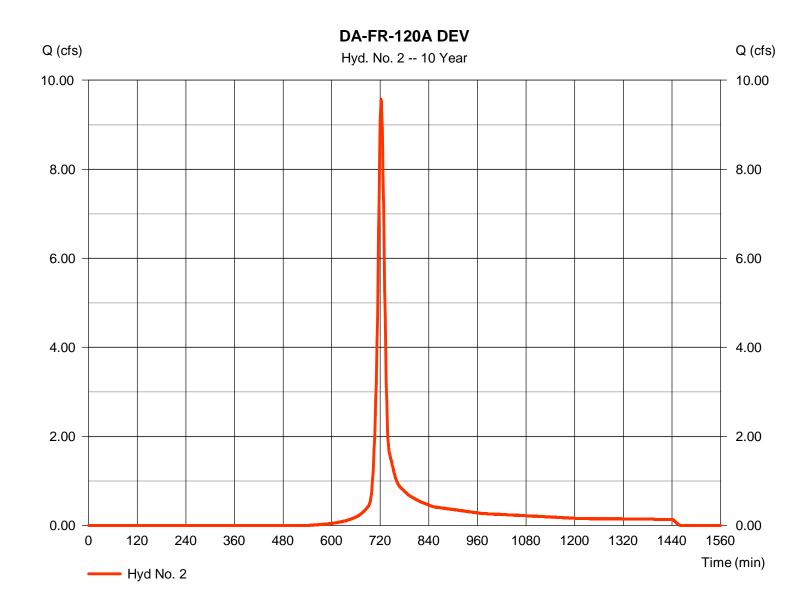
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 2

DA-FR-120A DEV

= SCS Runoff Hydrograph type Peak discharge = 9.565 cfsStorm frequency Time to peak = 722 min = 10 yrsTime interval = 1 min Hyd. volume = 25,771 cuftDrainage area Curve number = 2.670 ac= 71 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 13.80 \, \text{min}$ Total precip. Distribution = 5.70 in= Type II Storm duration = 24 hrs Shape factor = 484



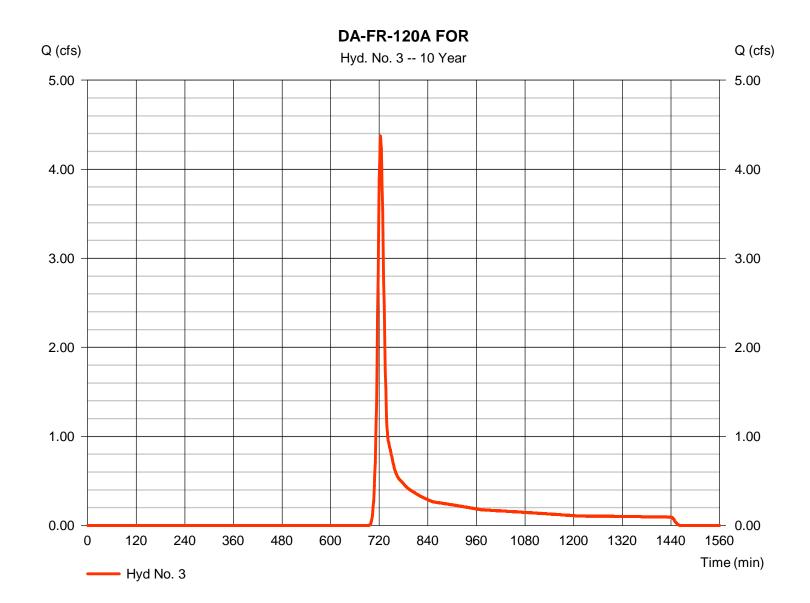
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 3

DA-FR-120A FOR

= SCS Runoff Hydrograph type Peak discharge = 4.374 cfsStorm frequency Time to peak = 723 min = 10 yrsTime interval = 1 min Hyd. volume = 13,070 cuftDrainage area Curve number = 2.670 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 13.80 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Return Period	Intensity-Du	Intensity-Duration-Frequency Equation Coefficients (FHA)										
(Yrs)	В	D	E	(N/A)								
1	0.0000	0.0000	0.0000									
2	69.8703	13.1000	0.8658									
3	0.0000	0.0000	0.0000									
5	79.2597	14.6000	0.8369									
10	88.2351	15.5000	0.8279									
25	102.6072	16.5000	0.8217									
50	114.8193	17.2000	0.8199									
100	127.1596	17.8000	0.8186									

File name: SampleFHA.idf

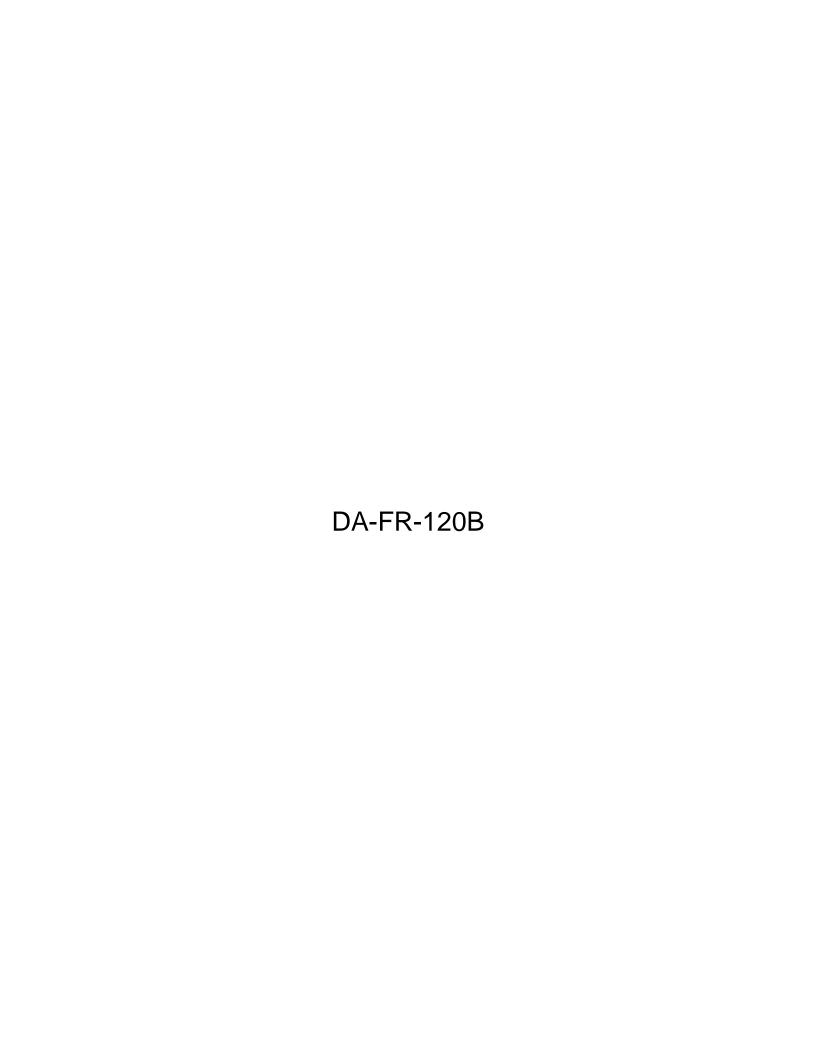
Intensity = $B/(Tc + D)^E$

Return		Intensity Values (in/hr)												
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60		
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70		
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15		
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46		
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91		
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25		
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60		

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc

	Rainfall Precipitation Table (in)									
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr		
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00		
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		



STORAGE VOLUME OF WATERBAR WITH SOIL COMPOST AMENDMENT AREA

Equations Used:

¹Vgravel storage = L*W*D_{gravel}*(40/100) ²Vsoil storage = L*W*D_{soil}*(20/100)

 3 V surface storage = [W*S*D^2]+[L*S*D^2]+[W*L*D]+[((2*S*D)^2*D)/3]

¹Equation #2b under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, assuming that gravel is made up of 40% voids.

²Equation #2b under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, assuming that soil compost amendment is made up of 20% voids.

³Equation #1 under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, but calculation also takes into account surface side slopes.

Inputs:	Depth	n of Gravel Layer, D _{gravel} (ft) =	0	
	Depth of Soil	Amendment Area, D _{soil} (ft) =	1	Refer to Table 4.3 in VA DEQ Stormwater Design Specification No. 4; Note that compost amendment may not be necessary for HSG A/B soils
	Length of Waterbar S	oil Amendment Area, L (ft) =	40	Assume max. length of 50' for waterbar soil amendment areas (i.e., limited to permanent ROW)
	Width of Waterbar So	il Amendment Area, W (ft) =	3	
		nkment Side Slopes, S (H:V) =	2	Assume 2H:1V surface side slopes for waterbars
	Number of Perm. Wat	terbars in Drainage Area, n =	4	
	Design	Infiltration Rate, IR (in/hr) =	0.2	Min. rate of 0.30 in/hr for HSG A soils and 0.15-0.30 in/hr for HSG B soils (see Chap. 4, p. 4-30 in VA Stormwater Management Handbook Volume II (First Edition, 1999)
	Sui	rface Ponding Depth, D (ft) =	0.5	Assume 0.5' CFS height at the end of waterbars
Calculations:		Storage Depth per BMP (ft) =	1.5	
	Surface Sto	orage Volume per BMP (cf) =	82	
	Subsurface Sto	orage Volume per BMP (cf) =	24	
	Total Sto	orage Volume per BMP (cf) =	106	
	Total BMP Storage Vo	olume in Drainage Area (cf) =	425	
	Calculated Infilt	ration Period per BMP (hr) =	53	
		Depth-Storage D		
Depth (ft)	Width (ft)	Length (ft)	Storage Volume per BMP (cf)	Storage Volume in Drainage Area (cf)
0	3	40	0	0
0.5	3	40	12	48
1	3	40	24	96
1.5	5	42	106	425
2	7	44	235	941

ENERGY BALANCE METHOD

Inputs:

	1-	-Yr Event
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)
Pre-Developed Condition	0.919	2593
Developed Condition	0.835	2168
Pre-Developed (Forest) Condition	0.119	735

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.9

Calculations:	Check #1:	$Q_{\text{developed}} \leq \text{IF } x \left[\left(Q_{\text{pre-developed}} \times \text{RV}_{\text{pre-developed}} \right) / \text{RV}_{\text{developed}} \right] >$	0.835	≤ OK	0.989
	Check #2:	Q _{developed} ≤ Q _{pre-developed} →	0.835	≤ OK	0.919
	Check #3:	$Q_{developed} \frac{shall\ not}{not}$ be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed}$ >	0.835	shall not be required to be ≤	0.040

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	·
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 – Manning's n Values for Open Channel Flow

nan	nel T	уре		Manning n	
			Min.	Normal	Max.
1.	Exc	avated or Dredged Channels ¹			
	a.	Earth, Straight, and Uniform:			
		Clean, recently completed	0.016	0.018	0.020
		Clean, after weathering	0.018	0.022	0.025
		Gravel, uniform section, clean	0.022	0.025	0.030
		With short grass, few weeds	0.022	0.027	0.033
	b.	Earth Winding and Sluggish:			
		No vegetation	0.023	0.025	0.030
		Grass, some weeds	0.025	0.030	0.033
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040
		Earth bottom and rubble sides	0.028	0.030	0.03
		Stony bottom and weedy banks	0.025	0.035	0.040
		Cobble bottom and clean sides	0.030	<mark>0.040</mark>	0.050
	c.	Dragline-Excavated or Dredged:			
		No vegetation	0.025	0.028	0.03
		Light brush on banks	0.035	0.050	0.060
	d.	Rock Cuts:			
		Smooth and uniform	0.025	0.035	0.040
		Jagged and irregular	0.035	0.040	0.050
	e.	Channels not Maintained, Weeds and Brush Uncut:			
		Dense weeds, high as flow depth	0.050	0.080	0.120
		Clean bottom, brush on sides	0.040	0.050	0.080
		Same as above, highest stage of flow	0.045	0.070	0.110
		Dense brush, high stage	0.080	0.100	0.140
2.	Mai	n Channels²			
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.033
	b.	Same as above, but more stones and weeds	0.030	0.035	0.040
	C.	Clean, winding, some pools and shoals	0.033	0.040	0.04
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.05
	f.	Same as (d) with more stones	0.045	0.050	0.060
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

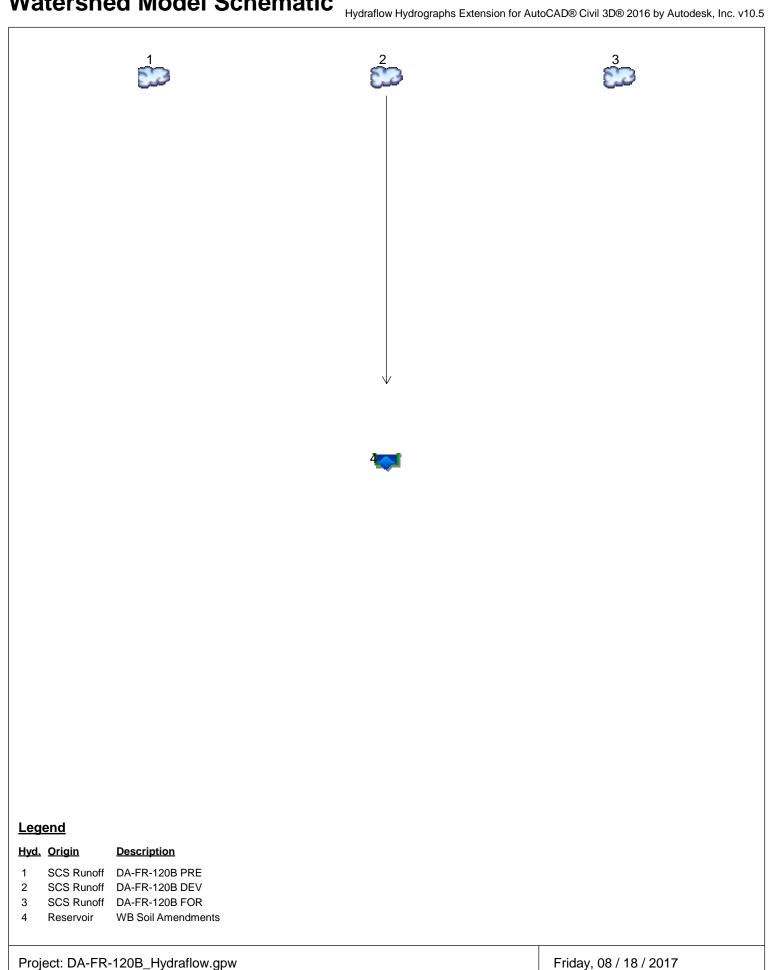
-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	. 3
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-120B PRE	. 4
TR-55 Tc Worksheet	. 5
Hydrograph No. 2, SCS Runoff, DA-FR-120B DEV	. 6
TR-55 Tc Worksheet	. 7
Hydrograph No. 3, SCS Runoff, DA-FR-120B FOR	. 8
TR-55 Tc Worksheet	
Hydrograph No. 4, Reservoir, WB Soil Amendments	10
Pond Report - Waterbar Soil Amendments	11
2 - Year	
	42
Summary Report	13
Hydrograph Reports	14
Hydrograph No. 2, SCS Runoff, DA-FR-120B DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-120B FOR	
Hydrograph No. 4, Reservoir, WB Soil Amendments	
Trydrograpit No. 4, Neservoll, WB Soll Amendments	17
10 - Year	
Summary Report	18
Hydrograph Reports	19
Hydrograph No. 1, SCS Runoff, DA-FR-120B PRE	19
Hydrograph No. 2, SCS Runoff, DA-FR-120B DEV	20
Hydrograph No. 3, SCS Runoff, DA-FR-120B FOR	
Hydrograph No. 4, Reservoir, WB Soil Amendments	22
IDE December 1	^^



Project: DA-FR-120B_Hydraflow.gpw

Hydrograph Return Period Recap Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	Hydrograph	Inflow				Hydrograph							
No.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description		
1	SCS Runoff		0.919	1.183			2.671				DA-FR-120B PRE		
2	SCS Runoff		0.919	1.183			2.671				DA-FR-120B DEV		
3	SCS Runoff		0.119	0.236			1.179				DA-FR-120B FOR		
4	Reservoir	2	0.835	1.153			2.650				WB Soil Amendments		

Proj. file: DA-FR-120B_Hydraflow.gpw

Friday, 08 / 18 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.919	1	723	2,593				DA-FR-120B PRE
2	SCS Runoff	0.919	1	723	2,593				DA-FR-120B DEV
3	SCS Runoff	0.119	1	726	735				DA-FR-120B FOR
4	Reservoir	0.835	1	726	2,168	2	101.60	528	WB Soil Amendments
DA-	-FR-120B_Hy	draflow.g	pw		Return P	eriod: 1 Ye	ear	Friday, 08 /	18 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

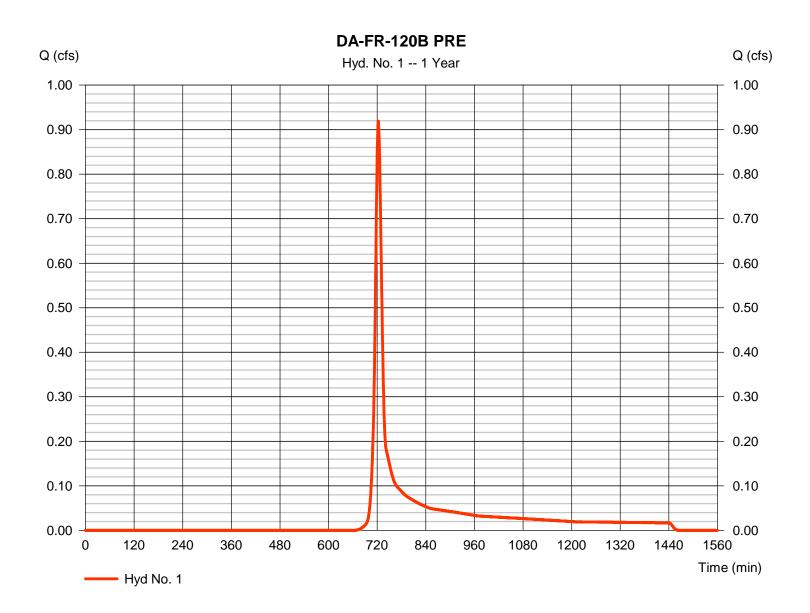
Friday, 08 / 18 / 2017

Hyd. No. 1

DA-FR-120B PRE

Hydrograph type = SCS Runoff Peak discharge = 0.919 cfsStorm frequency Time to peak = 723 min = 1 yrsTime interval = 1 minHyd. volume = 2.593 cuftCurve number Drainage area = 0.720 ac= 72*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 13.80 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.070 \times 87) + (0.300 \times 71) + (0.010 \times 100) + (0.340 \times 70)] / 0.720$



Hyd. No. 1

DA-FR-120B PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 5.81		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 13.04	+	0.00	+	0.00	=	13.04
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 281.19 = 14.88 = Unpaved =6.22	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.75	+	0.00	+	0.00	=	0.75
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							13.80 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

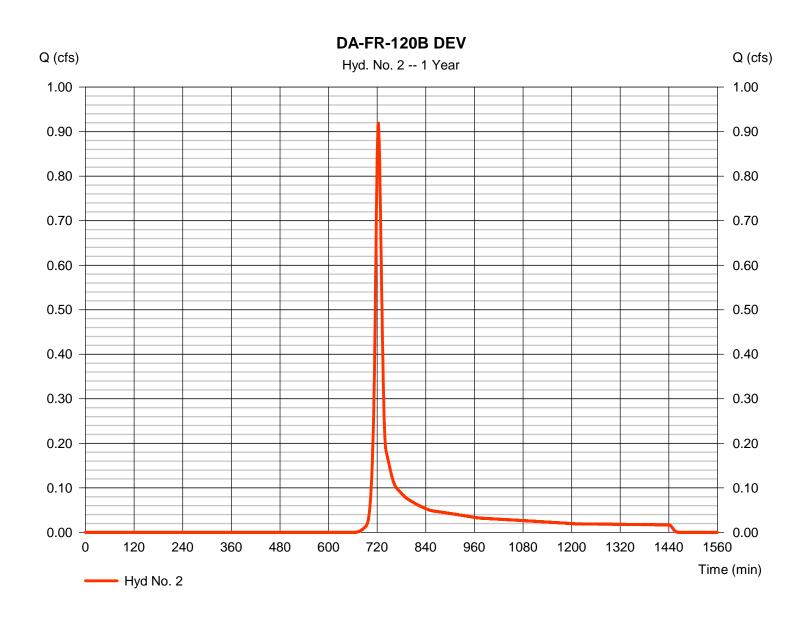
Friday, 08 / 18 / 2017

Hyd. No. 2

DA-FR-120B DEV

Hydrograph type = SCS Runoff Peak discharge = 0.919 cfsStorm frequency Time to peak = 723 min = 1 yrsTime interval = 1 minHyd. volume = 2.593 cuftCurve number Drainage area = 0.720 ac= 72*Basin Slope = 0.0 % Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 13.80 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.110 \times 65) + (0.070 \times 87) + (0.510 \times 71) + (0.010 \times 100) + (0.020 \times 70)] / 0.720$



Hyd. No. 2

DA-FR-120B DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 5.81		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		40.04
Travel Time (min)	= 13.04	+	0.00	+	0.00	=	13.04
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 107.26 = 13.76 = Unpaved =5.99	t	121.84 18.47 Unpave 6.93	d	0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.30	+	0.29	+	0.00	=	0.59
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})46.7		0.0		0.0		
Travel Time (min)	= 0.16	+	0.00	+	0.00	=	0.16
Total Travel Time, Tc							13.80 min

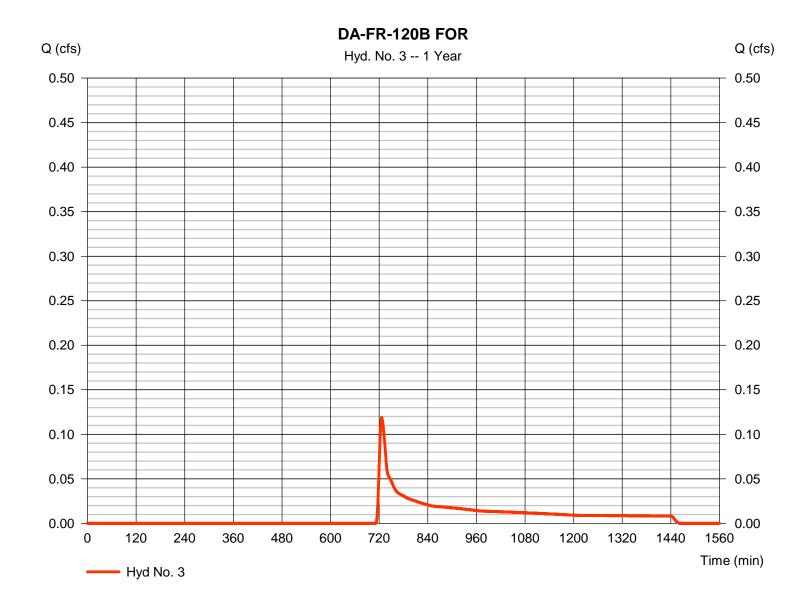
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 3

DA-FR-120B FOR

Hydrograph type = SCS Runoff Peak discharge = 0.119 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 min Hyd. volume = 735 cuft Drainage area Curve number = 0.720 ac= 55Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 13.80 \, \text{min}$ = TR55 Total precip. = 3.30 inDistribution = Type II Storm duration Shape factor = 24 hrs = 484



Hyd. No. 3

DA-FR-120B FOR

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 5.81		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 13.04	+	0.00	+	0.00	=	13.04
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 281.19 = 14.88 = Unpaved =6.22	I	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.75	+	0.00	+	0.00	=	0.75
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							13.80 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

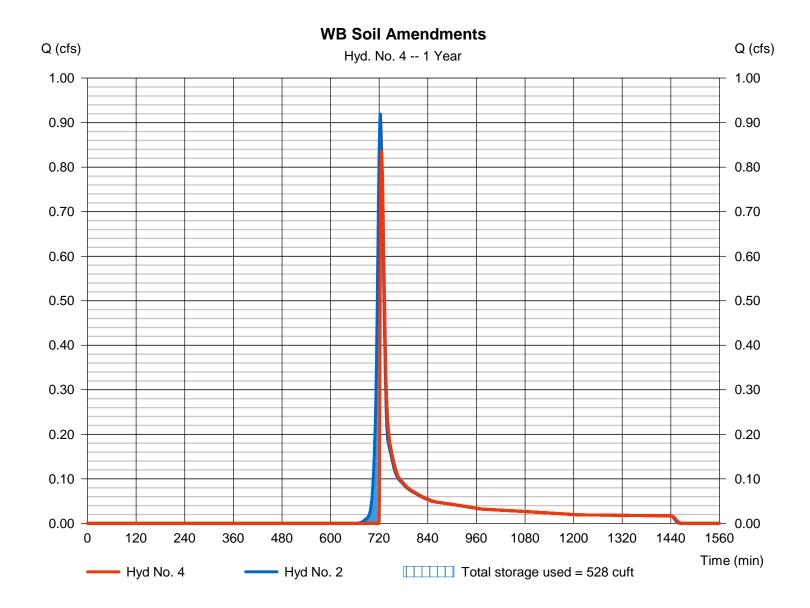
Friday, 08 / 18 / 2017

Hyd. No. 4

WB Soil Amendments

Hydrograph type Peak discharge = 0.835 cfs= Reservoir Storm frequency Time to peak = 726 min = 1 yrsTime interval = 1 minHyd. volume = 2,168 cuftInflow hyd. No. Max. Elevation = 2 - DA-FR-120B DEV $= 101.60 \, \text{ft}$ = Waterbar Soil Amendments Reservoir name Max. Storage = 528 cuft

Storage Indication method used.



Friday, 08 / 18 / 2017

Pond No. 1 - Waterbar Soil Amendments

Pond Data

Pond storage is based on user-defined values.

Stage / Storage Table

Culvert / Orifice Structures

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	100.00	n/a	0	0
0.50	100.50	n/a	48	48
1.00	101.00	n/a	48	96
1.50	101.50	n/a	329	425
2.00	102.00	n/a	516	941

[D] [A] [B] [C] [PrfRsr] [A] [B] [C] Rise (in) = 0.000.00 0.00 0.00 Crest Len (ft) = 8.00 0.00 0.00 0.00 Span (in) = 0.00 0.00 0.00 0.00 Crest El. (ft) = 101.500.00 0.00 0.00 No. Barrels = 00 0 Weir Coeff. = 3.333.33 3.33 3.33 Invert El. (ft) = 0.000.00 0.00 0.00 Weir Type = Rect Length (ft) = 0.000.00 0.00 0.00 Multi-Stage = No No No No Slope (%) = 0.000.00 0.00 n/a

Weir Structures

N-Value = .013 .013 .013 n/a = 0.600.60 0.60 0.60 = 0.000 (by Wet area) Orifice Coeff. Exfil.(in/hr) Multi-Stage = n/aNo TW Elev. (ft) = 0.00

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage	:/Discharge	Table
-----------------	-------------	-------

Stage ft	Storage cuft	Elevation ft	CIv A cfs	CIv B cfs	CIv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	100.00					0.00						0.000
0.05	5	100.05					0.00						0.000
0.10	10	100.10					0.00						0.000
0.15	14	100.15					0.00						0.000
0.20	19	100.20					0.00						0.000
0.25	24	100.25					0.00						0.000
0.30	29	100.30					0.00						0.000
0.35	34	100.35					0.00						0.000
0.40	38	100.40					0.00						0.000
0.45	43	100.45					0.00						0.000
0.50	48	100.50					0.00						0.000
0.55	53	100.55					0.00						0.000
0.60	58	100.60					0.00						0.000
0.65	62	100.65					0.00						0.000
0.70	67	100.70					0.00						0.000
0.75	72	100.75					0.00						0.000
0.80	77	100.80					0.00						0.000
0.85	82	100.85					0.00						0.000
0.90	86	100.90					0.00						0.000
0.95	91	100.95					0.00						0.000
1.00	96	101.00					0.00						0.000
1.05	129	101.05					0.00						0.000
1.10	162	101.10					0.00						0.000
1.15	195	101.15					0.00						0.000
1.20	228	101.20					0.00						0.000
1.25	260	101.25					0.00						0.000
1.30	293	101.30					0.00						0.000
1.35	326	101.35					0.00						0.000
1.40	359	101.40					0.00						0.000
1.45	392	101.45					0.00						0.000
1.50	425	101.50					0.00						0.000
1.55	477	101.55					0.30						0.298
1.60	528	101.60					0.84						0.843
1.65	580	101.65					1.55						1.548
1.70	631	101.70					2.38						2.383
1.75	683	101.75					3.33						3.330
1.80	735	101.80					4.38						4.378
1.85	786	101.85					5.52						5.517
1.90	838	101.90					6.74						6.740
											Continue	es on nex	t page

Waterbar Soil Amendments Stage / Storage / Discharge Table

Stage	Storage	Elevation	CIv A	Clv B	CIv C	PrfRsr	Wr A	Wr B	Wr C	Wr D	Exfil	User	Total
ft	cuft	ft	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs
1.95 2.00	889 941	101.95 102.00					8.04 9.42						8.043 9.419

...End

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

						- Trydranow Tr	yarograpno zate		D® Civil 3D® 2016 by Autodesk, Inc. v10.
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	1.183	1	723	3,277				DA-FR-120B PRE
2	SCS Runoff	1.183	1	723	3,277				DA-FR-120B DEV
3	SCS Runoff	0.236	1	725	1,086				DA-FR-120B FOR
4	Reservoir	1.153	1	724	2,852	2	101.62	551	WB Soil Amendments
DA	 -FR-120B_Hy	draflow.g	lpw		Return F	Period: 2 Ye	 ear	Friday, 08 /	18 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

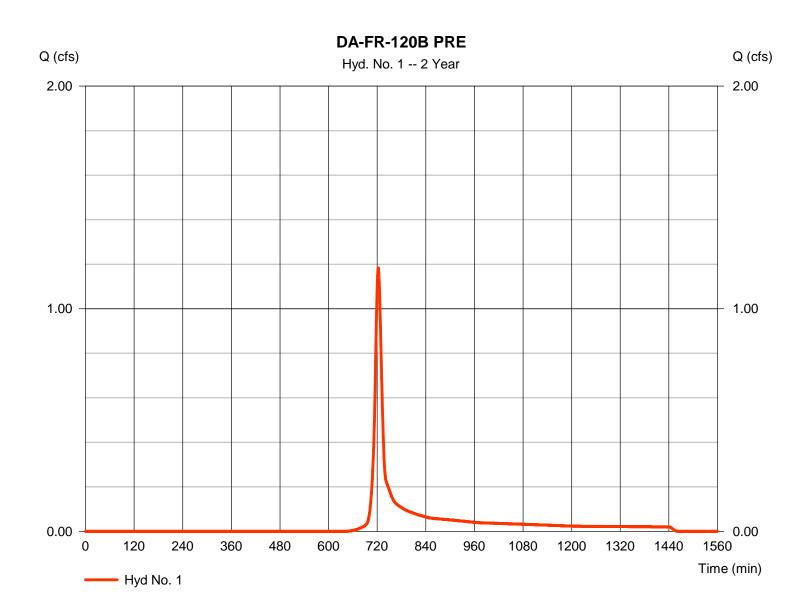
Friday, 08 / 18 / 2017

Hyd. No. 1

DA-FR-120B PRE

Hydrograph type = SCS Runoff Peak discharge = 1.183 cfsStorm frequency = 2 yrsTime to peak = 723 min Time interval = 1 minHyd. volume = 3.277 cuftCurve number Drainage area = 0.720 ac= 72*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 13.80 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.070 \times 87) + (0.300 \times 71) + (0.010 \times 100) + (0.340 \times 70)] / 0.720$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

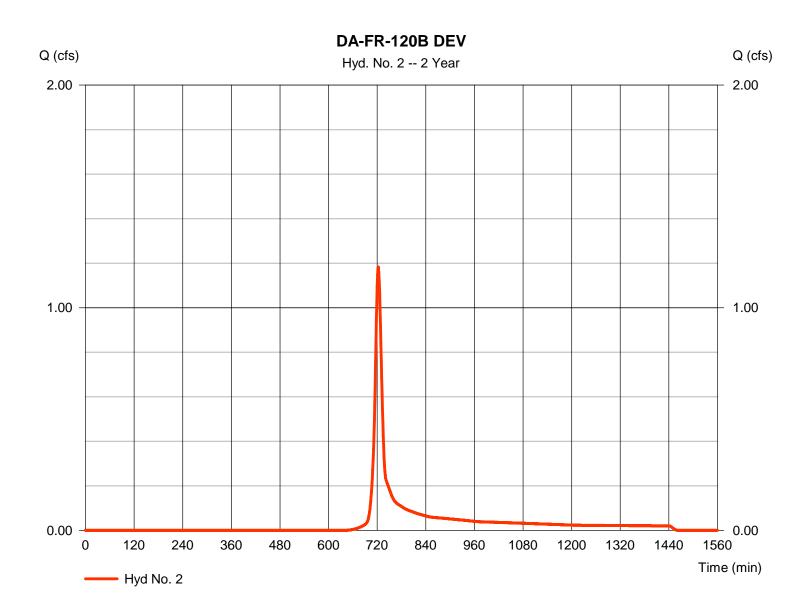
Friday, 08 / 18 / 2017

Hyd. No. 2

DA-FR-120B DEV

Hydrograph type = SCS Runoff Peak discharge = 1.183 cfsStorm frequency = 2 yrsTime to peak = 723 min Time interval = 1 minHyd. volume = 3.277 cuftCurve number Drainage area = 0.720 ac= 72*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 13.80 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.110 \times 65) + (0.070 \times 87) + (0.510 \times 71) + (0.010 \times 100) + (0.020 \times 70)] / 0.720$



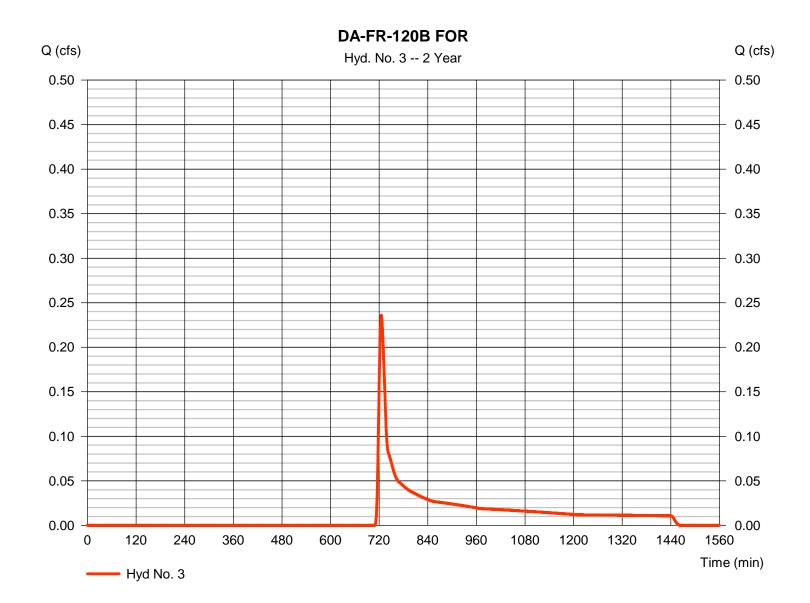
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 3

DA-FR-120B FOR

Hydrograph type = SCS Runoff Peak discharge = 0.236 cfsStorm frequency = 2 yrsTime to peak = 725 min Time interval = 1 min Hyd. volume = 1.086 cuftDrainage area Curve number = 0.720 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 13.80 \, \text{min}$ = TR55 Total precip. = 3.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

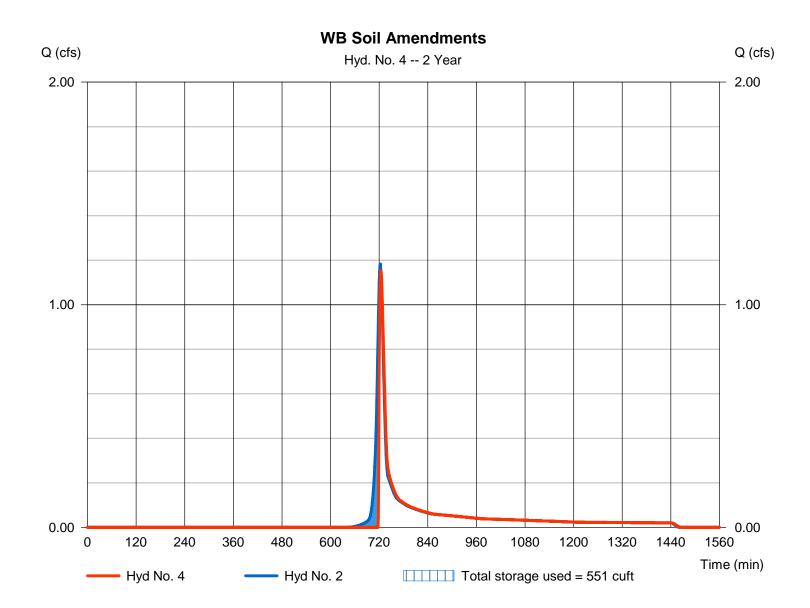
Friday, 08 / 18 / 2017

Hyd. No. 4

WB Soil Amendments

Hydrograph type = Reservoir Peak discharge = 1.153 cfsStorm frequency = 2 yrsTime to peak = 724 min Time interval = 1 minHyd. volume = 2.852 cuftInflow hyd. No. Max. Elevation = 2 - DA-FR-120B DEV = 101.62 ft= Waterbar Soil Amendments Reservoir name Max. Storage = 551 cuft

Storage Indication method used.



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	<u> </u>		utoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.
Hyd. Hydrograph Peak Time Time Interval (cfs) (min)	o Hyd. Inflow volume (cuft)	Maximum Total strge use (ft) (cuft)	Hydrograph Description
1 SCS Runoff 2.671 1 7	2 7,187		DA-FR-120B PRE
2 SCS Runoff 2.671 1 7	? 7,187		DA-FR-120B DEV
3 SCS Runoff 1.179 1 7	3,524		DA-FR-120B FOR
4 Reservoir 2.650 1 7	6,762 2	101.71 646	WB Soil Amendments
DA-FR-120B_Hydraflow.gpw	Return Period: 10	Year Friday	, 08 / 18 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

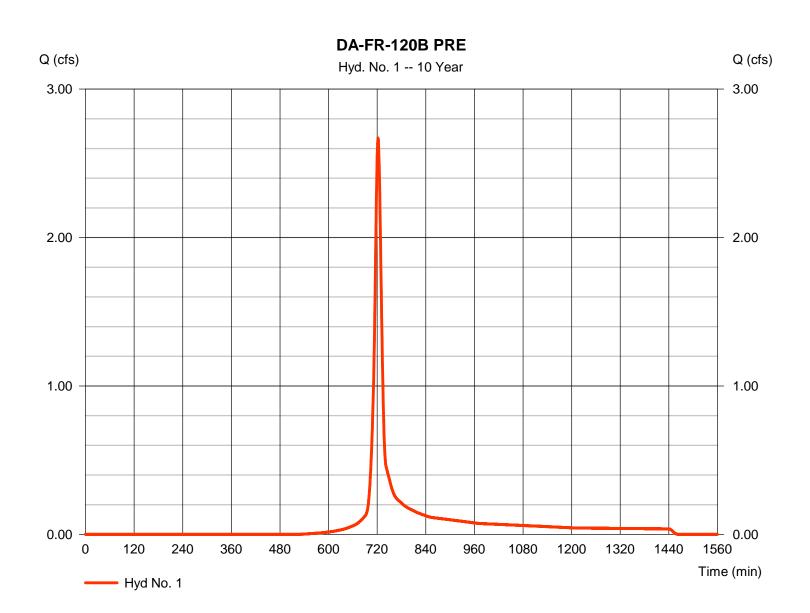
Friday, 08 / 18 / 2017

Hyd. No. 1

DA-FR-120B PRE

Hydrograph type = SCS Runoff Peak discharge = 2.671 cfsStorm frequency = 10 yrsTime to peak = 722 min Time interval = 1 minHyd. volume = 7,187 cuftCurve number Drainage area = 0.720 ac= 72*Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) = 13.80 min Tc method = TR55 Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs = 484Shape factor

^{*} Composite (Area/CN) = $[(0.070 \times 87) + (0.300 \times 71) + (0.010 \times 100) + (0.340 \times 70)] / 0.720$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

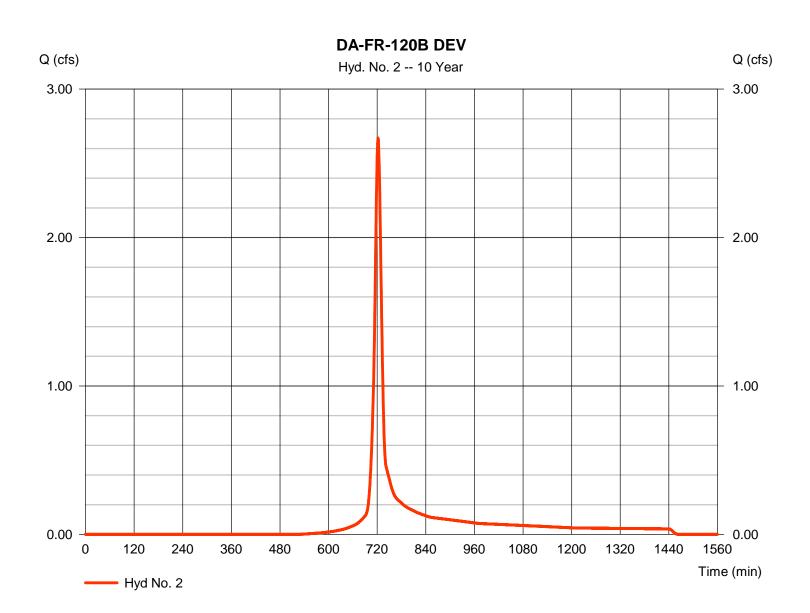
Friday, 08 / 18 / 2017

Hyd. No. 2

DA-FR-120B DEV

Hydrograph type = SCS Runoff Peak discharge = 2.671 cfsStorm frequency = 10 yrsTime to peak = 722 min Time interval = 1 minHyd. volume = 7,187 cuftDrainage area = 0.720 acCurve number = 72*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 13.80 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inStorm duration = 24 hrs = 484Shape factor

^{*} Composite (Area/CN) = $[(0.110 \times 65) + (0.070 \times 87) + (0.510 \times 71) + (0.010 \times 100) + (0.020 \times 70)] / 0.720$



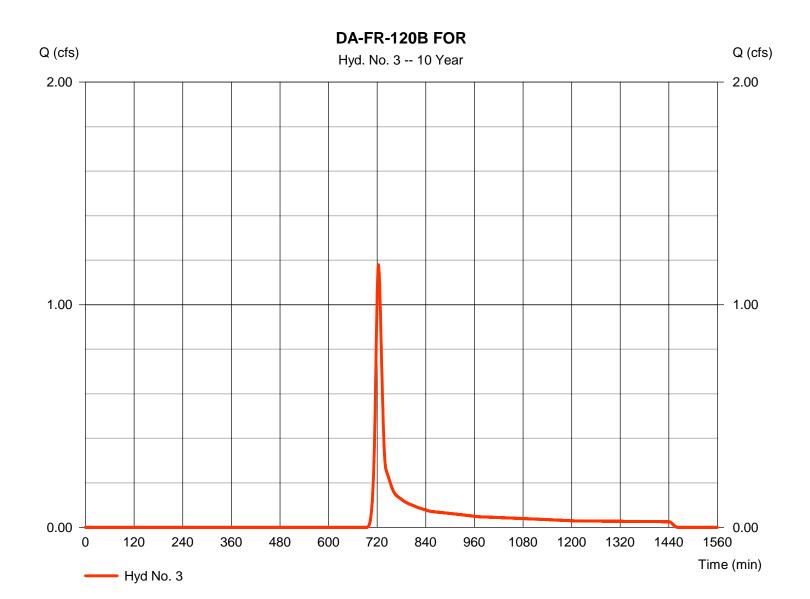
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 3

DA-FR-120B FOR

= SCS Runoff Hydrograph type Peak discharge = 1.179 cfsStorm frequency = 10 yrsTime to peak = 723 min Time interval = 1 minHyd. volume = 3.524 cuftDrainage area Curve number = 0.720 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 13.80 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

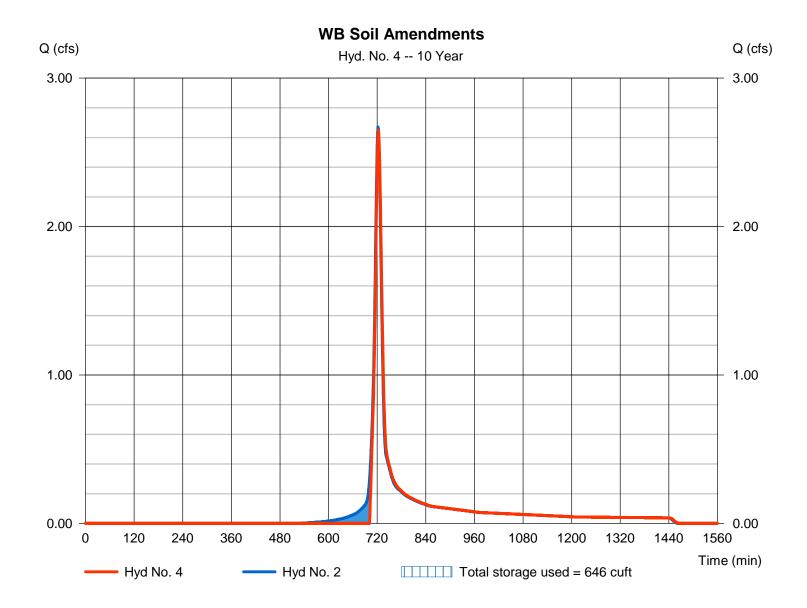
Friday, 08 / 18 / 2017

Hyd. No. 4

WB Soil Amendments

Hydrograph type = Reservoir Peak discharge = 2.650 cfsStorm frequency = 10 yrsTime to peak = 723 min Time interval = 1 minHyd. volume = 6,762 cuftInflow hyd. No. Max. Elevation = 2 - DA-FR-120B DEV = 101.71 ft= Waterbar Soil Amendments = 646 cuft Reservoir name Max. Storage

Storage Indication method used.



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Return Period	Intensity-Du	ıration-Frequency E	quation Coefficients	(FHA)
(Yrs)	В	D	E	(N/A)
1	0.0000	0.0000	0.0000	
2	69.8703	13.1000	0.8658	
3	0.0000	0.0000	0.0000	
5	79.2597	14.6000	0.8369	
10	88.2351	15.5000	0.8279	
25	102.6072	16.5000	0.8217	
50	114.8193	17.2000	0.8199	
100	127.1596	17.8000	0.8186	

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return		Intensity Values (in/hr)											
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60	
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70	
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15	
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46	
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91	
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25	
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60	

Tc = time in minutes. Values may exceed 60.

ling\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.p

Rainfall Precipitation Table (in)								
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00



STORAGE VOLUME OF WATERBAR WITH SOIL COMPOST AMENDMENT AREA

Equations Used:

¹Vgravel storage = L*W*D_{gravel}*(40/100) ²Vsoil storage = L*W*D_{soil}*(20/100)

 3 V surface storage = [W*S*D^2]+[L*S*D^2]+[W*L*D]+[((2*S*D)^2*D)/3]

¹Equation #2b under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, assuming that gravel is made up of 40% voids.

²Equation #2b under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, assuming that soil compost amendment is made up of 20% voids.

³Equation #1 under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, but calculation also takes into account surface side slopes.

Inputs:	Depth	of Gravel Layer, D _{gravel} (ft) =	0	
	Depth of Soil	Amendment Area, D _{soil} (ft) =	1	Refer to Table 4.3 in VA DEQ Stormwater Design Specification No. 4; Note that compost amendment may not be necessary for HSG A/B soils
	Length of Waterbar S	oil Amendment Area, L (ft) =	50	Assume max. length of 50' for waterbar soil amendment areas (i.e., limited to permanent ROW)
	Width of Waterbar So	I Amendment Area, W (ft) =	3	
		kment Side Slopes, S (H:V) =		Assume 2H:1V surface side slopes for waterbars
	Number of Perm. Wat	erbars in Drainage Area, n =	5	
	Design	Infiltration Rate, IR (in/hr) =	0.2	Min. rate of 0.30 in/hr for HSG A soils and 0.15-0.30 in/hr for HSG B soils (see Chap. 4, p. 4-30 in VA Stormwater Management Handbook Volume II (First Edition, 1999)
	Sur	face Ponding Depth, D (ft) =	0.5	Assume 0.5' CFS height at the end of waterbars
Calculations:		torage Depth per BMP (ft) =		
	Surface Sto	rage Volume per BMP (cf) =	102	
	Subsurface Sto	rage Volume per BMP (cf) =	30	
	Total Sto	rage Volume per BMP (cf) =	132	
	Total BMP Storage Vo	lume in Drainage Area (cf) =	661	
	Calculated Infilt	ation Period per BMP (hr) =	53	
		Depth-Storage I	Data	
Depth (ft)	Width (ft)	Length (ft)	Storage Volume per BMP (cf)	Storage Volume in Drainage Area (cf)
0	3	50	0	0
0.5	3	50	15	75
1	3	50	30	150
1.5	5	52	132	661
2	7	54	291	1457

ENERGY BALANCE METHOD

Inputs:

	1-	Yr Event
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)
Pre-Developed Condition	1.904	4766
Developed Condition	1.764	4105
Pre-Developed (Forest) Condition	0.254	1350

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.8

<u>Calculations:</u>	Check #1:	$Q_{developed} \le IF \times \{ \{Q_{gre-developed} \times RV_{pre-developed} \} / RV_{developed} \}$	1.764	≤ OK	1.768
	Check #2:	Q _{developed} ≤ Q _{pre-developed} >	1.764	≤ OK	1.904
	Check #3:	$Q_{\text{developed }} \underline{shall \ not}$ be required to be $\leq (Q_{\text{forest}} \times RV_{\text{forest}}) / RV_{\text{developed}} \longrightarrow$	1.764	shall not be required to be ≤	0.084

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	·
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

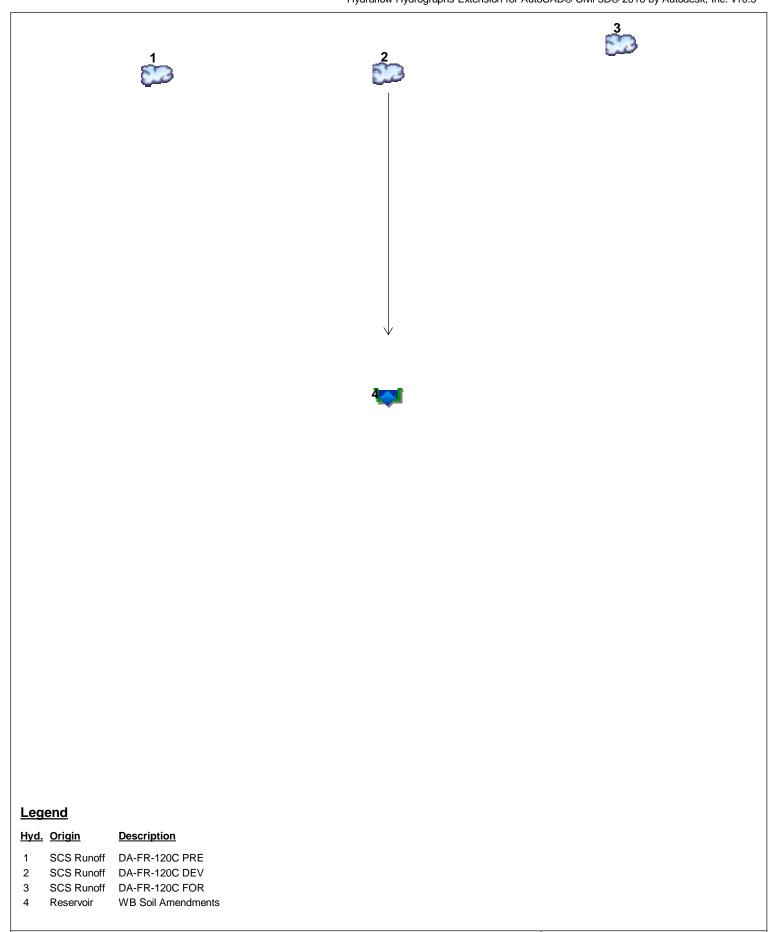
-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Friday, 08 / 18 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	4
Hydrograph No. 1, SCS Runoff, DA-FR-120C PRE	4
TR-55 Tc Worksheet	5
Hydrograph No. 2, SCS Runoff, DA-FR-120C DEV	6
TR-55 Tc Worksheet	
Hydrograph No. 3, SCS Runoff, DA-FR-120C FOR	
TR-55 Tc Worksheet	9
Hydrograph No. 4, Reservoir, WB Soil Amendments	10
Pond Report - Waterbar Soil Amendments	11
2 - Year Summary Report	13
Hydrograph Reports	14
Hydrograph No. 1, SCS Runoff, DA-FR-120C PRE	14
Hydrograph No. 2, SCS Runoff, DA-FR-120C DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-120C FOR	16
Hydrograph No. 4, Reservoir, WB Soil Amendments	17
10 - Year	
Summary Report	
Hydrograph Reports	19
Hydrograph No. 1, SCS Runoff, DA-FR-120C PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-120C DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-120C FOR	
Hydrograph No. 4, Reservoir, WB Soil Amendments	22
IDE Daw and	00

Project: DA-FR-120C_Hydraflow.gpw

Friday, 08 / 18 / 2017



Hydrograph Return Period Recap Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	Hydrograph	Inflow				Hydrograph					
lo.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		1.904	2.443			5.461				DA-FR-120C PRE
2	SCS Runoff		1.904	2.443			5.461				DA-FR-120C DEV
3	SCS Runoff		0.254	0.506			2.461				DA-FR-120C FOR
4	Reservoir	2	1.764	2.373			5.389				WB Soil Amendments

Proj. file: DA-FR-120C_Hydraflow.gpw

Friday, 08 / 18 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	1.904	1	721	4,766				DA-FR-120C PRE	
2	SCS Runoff	1.904	1	721	4,766				DA-FR-120C DEV	
3	SCS Runoff	0.254	1	724	1,350				DA-FR-120C FOR	
	SCS Runoff Reservoir	0.254 1.764	1 1	724 723	1,350 4,105	2	101.64	884	DA-FR-120C FOR WB Soil Amendments	
DA-FR-120C_Hydraflow.gpw				Return F	Period: 1 Ye	ear	Friday, 08 /	Friday, 08 / 18 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

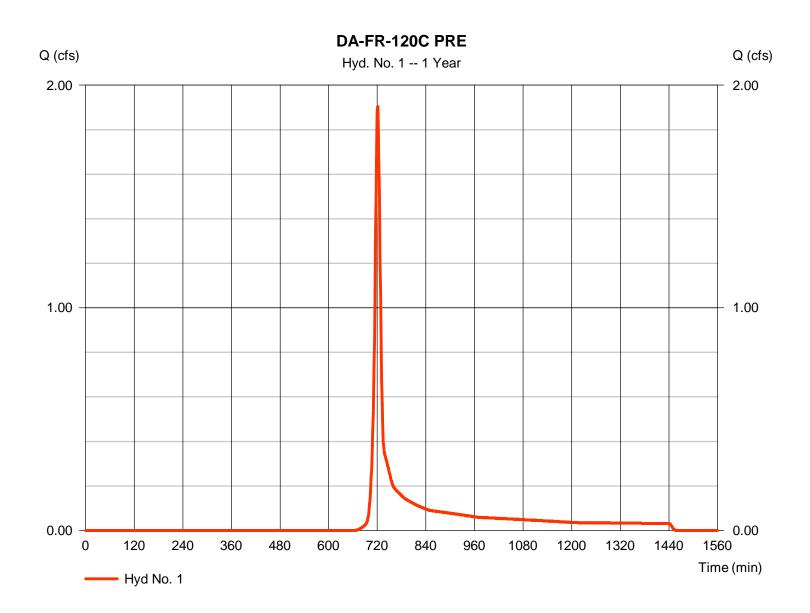
Friday, 08 / 18 / 2017

Hyd. No. 1

DA-FR-120C PRE

= SCS Runoff Hydrograph type Peak discharge = 1.904 cfsStorm frequency Time to peak = 721 min = 1 yrsTime interval = 1 minHyd. volume = 4,766 cuftDrainage area Curve number = 1.300 ac= 72*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 10.60 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.072 \times 87) + (0.748 \times 71) + (0.007 \times 100) + (0.471 \times 70)] / 1.300$



Hyd. No. 1

DA-FR-120C PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 10.83 = 10.16	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	10.16
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 211.08 = 20.64 = Unpave =7.33	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.48	+	0.00	+	0.00	=	0.48
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							10.60 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

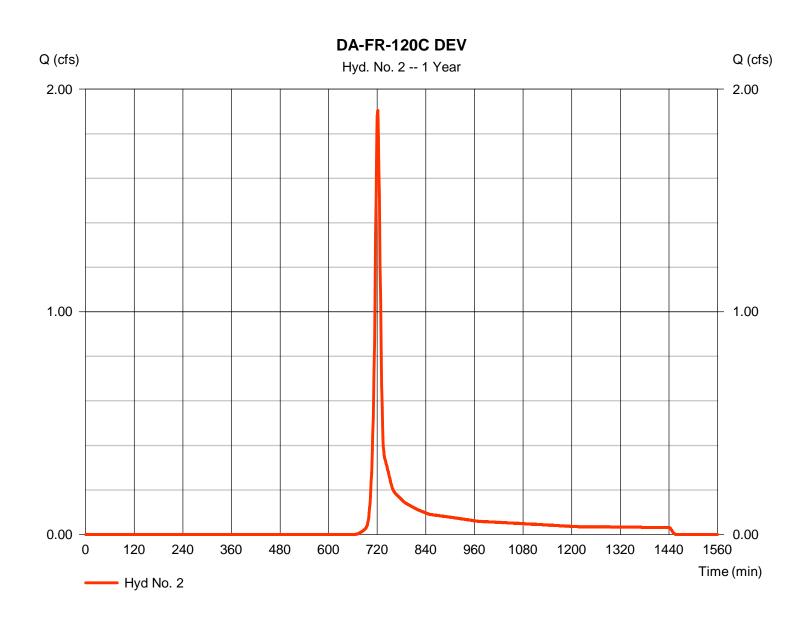
Friday, 08 / 18 / 2017

Hyd. No. 2

DA-FR-120C DEV

= SCS Runoff Hydrograph type Peak discharge = 1.904 cfsStorm frequency Time to peak = 721 min = 1 yrsTime interval = 1 minHyd. volume = 4,766 cuftDrainage area Curve number = 1.300 ac= 72*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 10.60 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.033 \times 65) + (0.072 \times 87) + (0.796 \times 71) + (0.007 \times 100) + (0.391 \times 70)] / 1.300$



Hyd. No. 2

DA-FR-120C DEV

Total Travel Time, Tc						10.60 min	
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Flow length (ft)	({0})0.0		0.0		0.0		
			0.00		0.00		
Manning's n-value Velocity (ft/s)	= 0.015 =0.00		0.015		0.015		
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%)	= 0.00 $= 0.00$ $= 0.00$		0.00 0.00 0.00		0.00 0.00 0.00		
Channel Flow		-		-			
Travel Time (min)	= 0.48	+	0.00	+	0.00	=	0.48
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 211.08 = 20.64 = Unpaved =7.33	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 10.16	+	0.00	+	0.00	=	10.16
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 10.83		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>

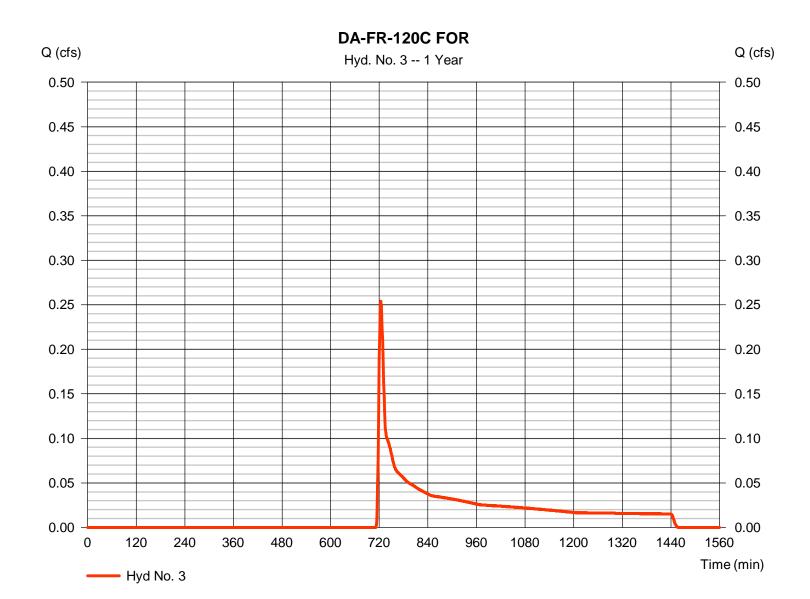
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 3

DA-FR-120C FOR

Hydrograph type = SCS Runoff Peak discharge = 0.254 cfsStorm frequency Time to peak = 724 min = 1 yrsTime interval = 1 min Hyd. volume = 1,350 cuftDrainage area Curve number = 1.300 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 10.60 \, \text{min}$ Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hyd. No. 3

DA-FR-120C FOR

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 10.83	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	10.16
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 211.08 = 20.64 = Unpaved =7.33	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.48	+	0.00	+	0.00	=	0.48
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							10.60 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

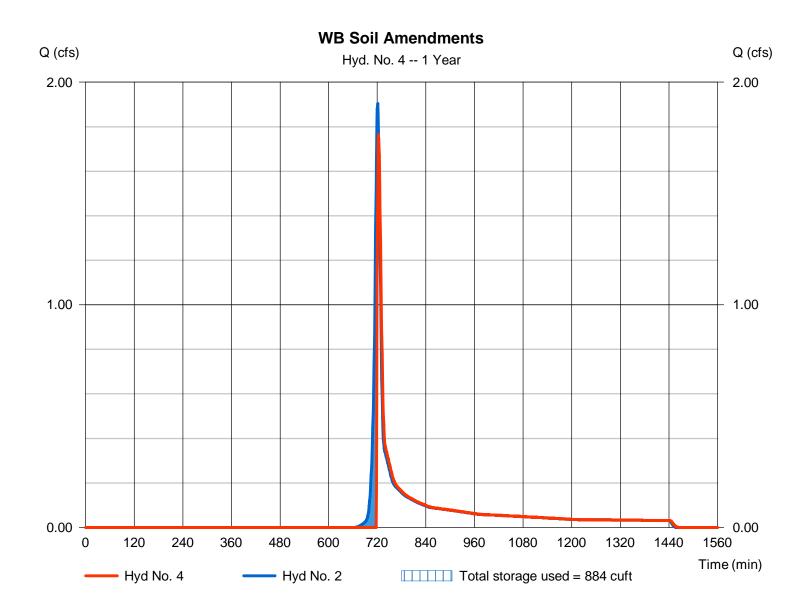
Friday, 08 / 18 / 2017

Hyd. No. 4

WB Soil Amendments

= 1.764 cfsHydrograph type = Reservoir Peak discharge Storm frequency Time to peak = 723 min = 1 yrsTime interval = 1 minHyd. volume = 4,105 cuftInflow hyd. No. Max. Elevation = 2 - DA-FR-120C DEV = 101.64 ft= Waterbar Soil Amendments Reservoir name Max. Storage = 884 cuft

Storage Indication method used.



Friday, 08 / 18 / 2017

Pond No. 1 - Waterbar Soil Amendments

Pond Data

Pond storage is based on user-defined values.

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	100.00	n/a	0	0
0.50	100.50	n/a	75	75
1.00	101.00	n/a	75	150
1.50	101.50	n/a	511	661
2.00	102.00	n/a	796	1,457

Culvert / Orifice Structures Weir Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 0.00	0.00	0.00	0.00	Crest Len (ft)	= 10.00	0.00	0.00	0.00
Span (in)	= 0.00	0.00	0.00	0.00	Crest El. (ft)	= 101.50	0.00	0.00	0.00
No. Barrels	= 0	0	0	0	Weir Coeff.	= 3.33	3.33	3.33	3.33
Invert El. (ft)	= 0.00	0.00	0.00	0.00	Weir Type	= Rect			
Length (ft)	= 0.00	0.00	0.00	0.00	Multi-Stage	= No	No	No	No
Slope (%)	= 0.00	0.00	0.00	n/a					
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by			
Multi-Stage	= n/a	No	No	No	TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage	/ Discharge	Table
-----------------	-------------	-------

Stage ft	Storage cuft	Elevation ft	CIv A cfs	CIv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	100.00					0.00						0.000
0.05	8	100.05					0.00						0.000
0.10	15	100.10					0.00						0.000
0.15	23	100.15					0.00						0.000
0.20	30	100.20					0.00						0.000
0.25	38	100.25					0.00						0.000
0.30	45	100.30					0.00						0.000
0.35	53	100.35					0.00						0.000
0.40	60	100.40					0.00						0.000
0.45	68	100.45					0.00						0.000
0.50	75	100.50					0.00						0.000
0.55	83	100.55					0.00						0.000
0.60	90	100.60					0.00						0.000
0.65	98	100.65					0.00						0.000
0.70	105	100.70					0.00						0.000
0.75	113	100.75					0.00						0.000
0.80	120	100.80					0.00						0.000
0.85	128	100.85					0.00						0.000
0.90	135	100.90					0.00						0.000
0.95	143	100.95					0.00						0.000
1.00	150	101.00					0.00						0.000
1.05	201	101.05					0.00						0.000
1.10	252	101.10					0.00						0.000
1.15	303	101.15					0.00						0.000
1.20	354	101.20					0.00						0.000
1.25	406	101.25					0.00						0.000
1.30	457	101.30					0.00						0.000
1.35	508	101.35					0.00						0.000
1.40	559	101.40					0.00						0.000
1.45	610	101.45					0.00						0.000
1.50	661	101.50					0.00						0.000
1.55	741	101.55					0.37						0.372
1.60	820	101.60					1.05						1.053
1.65	900	101.65					1.93						1.935
1.70	979	101.70					2.98						2.979
1.75	1,059	101.75					4.16						4.163
1.80	1,139	101.80					5.47						5.472
1.85	1,218	101.85					6.90						6.896
1.90	1,298	101.90					8.43						8.425
	Continues on next page										t page		

Continues on next page...

Waterbar Soil Amendments Stage / Storage / Discharge Table

Stage	Storage cuft	Elevation	Clv A	Clv B	CIv C	PrfRsr	Wr A	Wr B	Wr C	Wr D	Exfil	User	Total
ft		ft	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs
1.95 2.00	1,377 1,457	101.95 102.00					10.05 11.77						10.05 11.77

...End

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

		_			Hydranow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. V10.						
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description		
1	SCS Runoff	2.443	1	721	6,022				DA-FR-120C PRE		
2	SCS Runoff	2.443	1	721	6,022				DA-FR-120C DEV		
3	SCS Runoff	0.506	1	723	1,996				DA-FR-120C FOR		
4	Reservoir	2.373	1	722	5,361	2	101.67	933	WB Soil Amendments		
DA:	-FR-120C_Hy	/draflow.g	gpw		Return P	eriod: 2 Ye	ear	Friday, 08 /	18 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

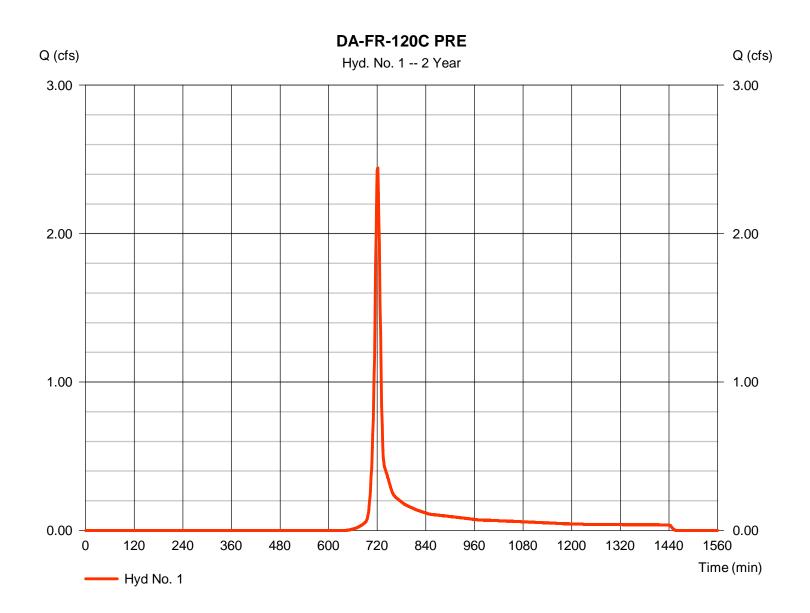
Friday, 08 / 18 / 2017

Hyd. No. 1

DA-FR-120C PRE

Hydrograph type = SCS Runoff Peak discharge = 2.443 cfsStorm frequency Time to peak = 721 min = 2 yrsTime interval = 1 min Hyd. volume = 6.022 cuftCurve number Drainage area = 1.300 ac= 72*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 10.60 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.072 \times 87) + (0.748 \times 71) + (0.007 \times 100) + (0.471 \times 70)] / 1.300$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

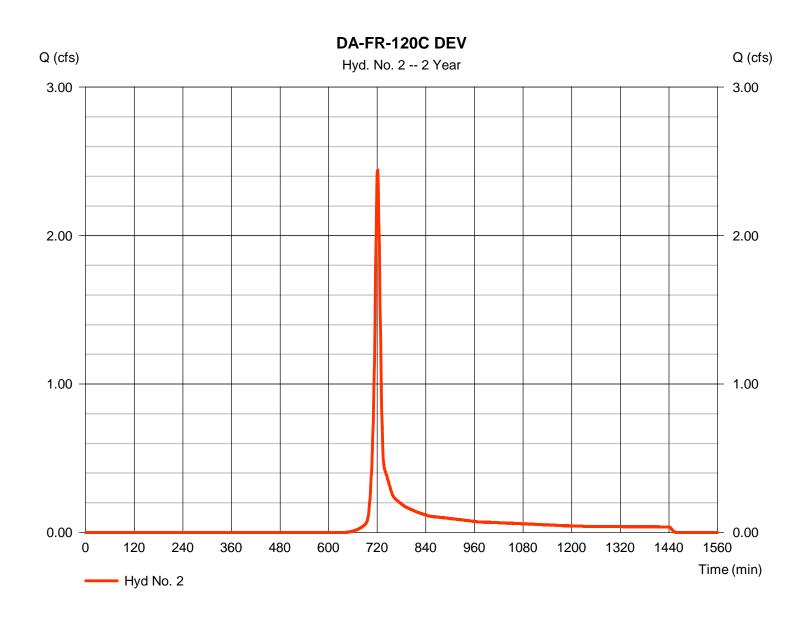
Friday, 08 / 18 / 2017

Hyd. No. 2

DA-FR-120C DEV

Hydrograph type = SCS Runoff Peak discharge = 2.443 cfsStorm frequency Time to peak = 721 min = 2 yrsTime interval = 1 min Hyd. volume = 6.022 cuftCurve number Drainage area = 1.300 ac= 72*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 10.60 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.033 \times 65) + (0.072 \times 87) + (0.796 \times 71) + (0.007 \times 100) + (0.391 \times 70)] / 1.300$



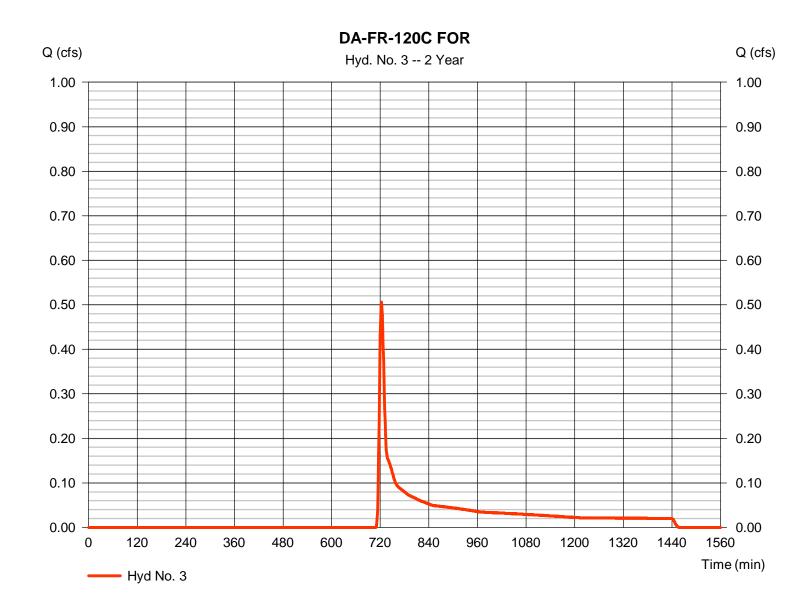
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 3

DA-FR-120C FOR

Hydrograph type = SCS Runoff Peak discharge = 0.506 cfsStorm frequency Time to peak = 723 min = 2 yrsTime interval = 1 min Hyd. volume = 1,996 cuftDrainage area Curve number = 1.300 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 10.60 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

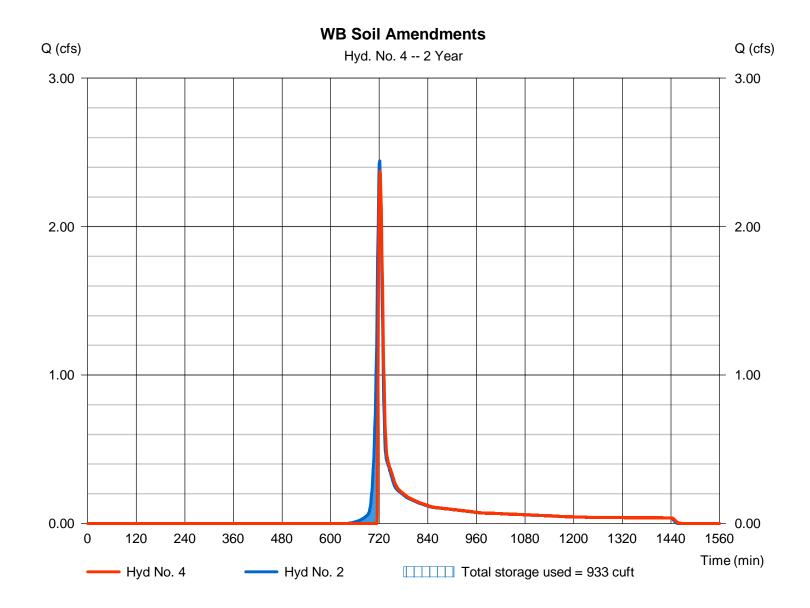
Friday, 08 / 18 / 2017

Hyd. No. 4

WB Soil Amendments

Hydrograph type = Reservoir Peak discharge = 2.373 cfsStorm frequency Time to peak = 722 min = 2 yrsTime interval = 1 min Hyd. volume = 5.361 cuftInflow hyd. No. Max. Elevation = 2 - DA-FR-120C DEV = 101.67 ft= Waterbar Soil Amendments Reservoir name Max. Storage = 933 cuft

Storage Indication method used.



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	5.461	1	720	13,208				DA-FR-120C PRE
2	SCS Runoff	5.461	1	720	13,208				DA-FR-120C DEV
3	SCS Runoff	2.461	1	721	6,477				DA-FR-120C FOR
	SCS Runoff Reservoir	2.461 5.389		721 721	6,477	2	101.80	1,134	DA-FR-120C FOR WB Soil Amendments
DA-	DA-FR-120C_Hydraflow.gpw					Period: 10 \	/ear	Friday, 08 /	18 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

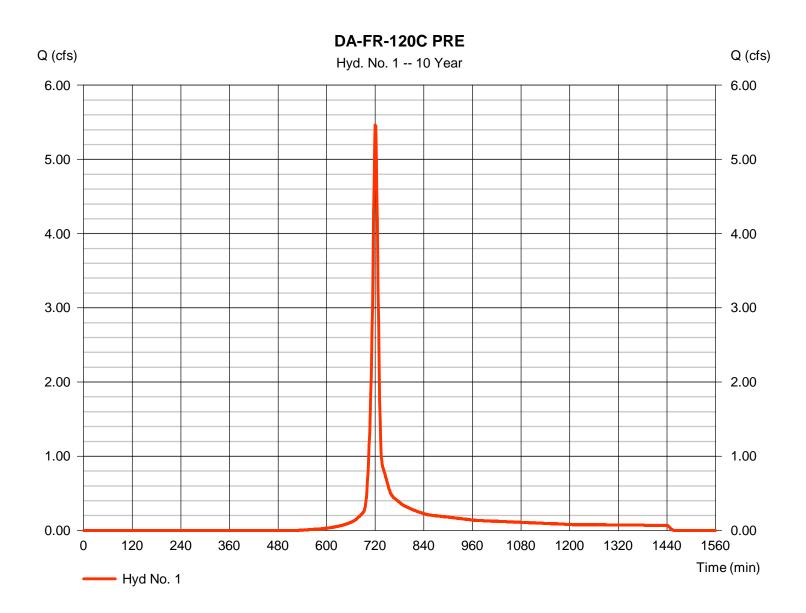
Friday, 08 / 18 / 2017

Hyd. No. 1

DA-FR-120C PRE

= SCS Runoff Hydrograph type Peak discharge = 5.461 cfsStorm frequency Time to peak = 720 min = 10 yrsTime interval = 1 min Hyd. volume = 13.208 cuft Curve number Drainage area = 1.300 ac= 72* Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 10.60 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration Shape factor = 484

 $= 24 \, hrs$



^{*} Composite (Area/CN) = $[(0.072 \times 87) + (0.748 \times 71) + (0.007 \times 100) + (0.471 \times 70)] / 1.300$

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

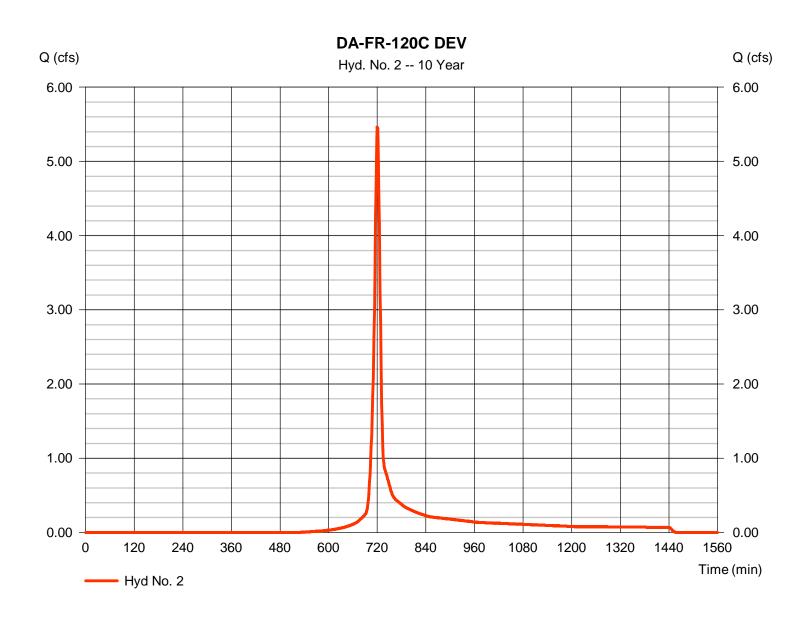
Friday, 08 / 18 / 2017

Hyd. No. 2

DA-FR-120C DEV

Hydrograph type = SCS Runoff Peak discharge = 5.461 cfsStorm frequency Time to peak = 720 min = 10 yrsTime interval = 1 min Hyd. volume = 13.208 cuft Curve number Drainage area = 1.300 ac= 72* Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 10.60 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.033 \times 65) + (0.072 \times 87) + (0.796 \times 71) + (0.007 \times 100) + (0.391 \times 70)] / 1.300$



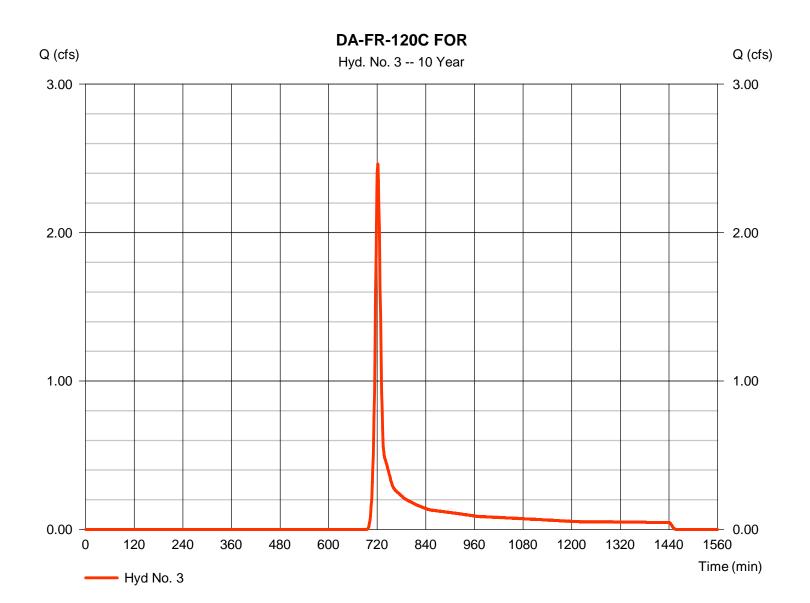
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 3

DA-FR-120C FOR

= SCS Runoff Hydrograph type Peak discharge = 2.461 cfsStorm frequency Time to peak = 721 min = 10 yrsTime interval = 1 min Hyd. volume = 6,477 cuftDrainage area Curve number = 1.300 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 10.60 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

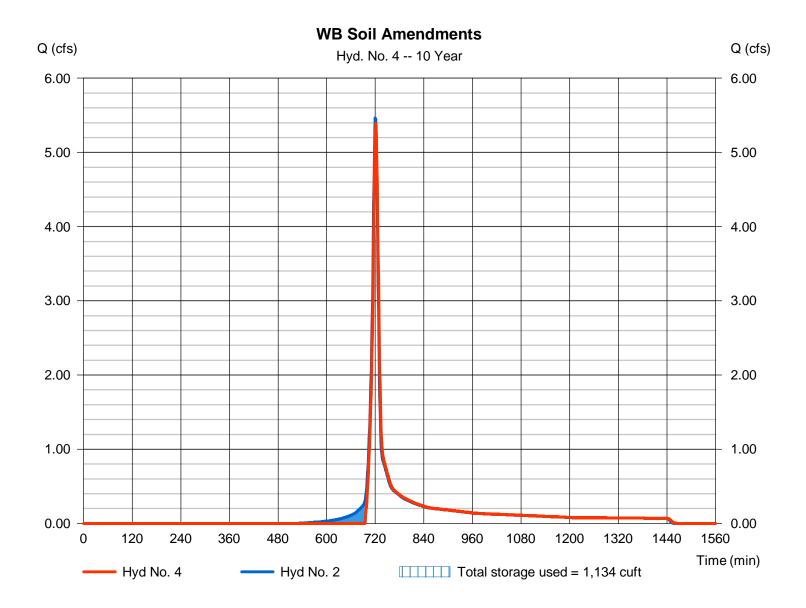
Friday, 08 / 18 / 2017

Hyd. No. 4

WB Soil Amendments

Hydrograph type = Reservoir Peak discharge = 5.389 cfsStorm frequency Time to peak = 721 min = 10 yrsTime interval = 1 min Hyd. volume = 12,547 cuftInflow hyd. No. Max. Elevation = 2 - DA-FR-120C DEV = 101.80 ftReservoir name = Waterbar Soil Amendments Max. Storage = 1,134 cuft

Storage Indication method used.



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Return Period	Intensity-Du	ıration-Frequency Ed	quation Coefficients ((FHA)
(Yrs)	В	D	E	(N/A)
1	0.0000	0.0000	0.0000	
2	69.8703	13.1000	0.8658	
3	0.0000	0.0000	0.0000	
5	79.2597	14.6000	0.8369	
10	88.2351	15.5000	0.8279	
25	102.6072	16.5000	0.8217	
50	114.8193	17.2000	0.8199	
100	127.1596	17.8000	0.8186	

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return		Intensity Values (in/hr)													
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60			
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70			
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15			
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46			
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91			
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25			
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60			

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc|

		R	ainfall P	recipitat	ion Tab	le (in)		
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00



ENERGY BALANCE METHOD

		ts

	1-Y	r Event
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)
Pre-Developed Condition	0.368	969
Developed Condition	0.333	902
Pre-Developed (Forest) Condition	0.094	449

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.9

Calculations:	Check #1:	$Q_{developed} \le IF x [(Q_{pre-developed} \times RV_{pre-developed}) / RV_{developed}]$	0.333	≤ OK	0.356
	Check #2:	Q _{developed} ≤ Q _{pre-developed} >	0.333	≤ OK	0.368
	Check #3:	$Q_{\text{developed } shall \ not}$ be required to be $\leq (Q_{\text{forest}} \times RV_{\text{forest}}) / RV_{\text{developed}} \longrightarrow$	0.333	shall not be required to be ≤	0.047

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

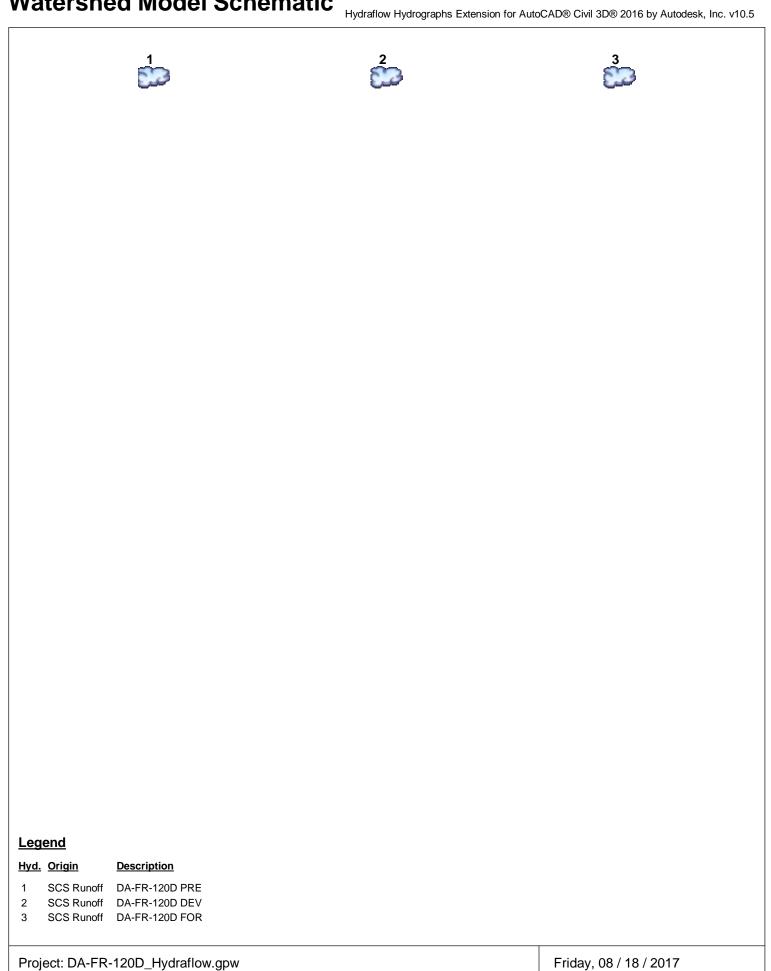
-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	
Hydrograph Reports	4
Hydrograph No. 1, SCS Runoff, DA-FR-120D PRE	4
TR-55 Tc Worksheet	
Hydrograph No. 2, SCS Runoff, DA-FR-120D DEV	6
TR-55 Tc Worksheet	
Hydrograph No. 3, SCS Runoff, DA-FR-120D FOR	
TR-55 Tc Worksheet	9
2 - Year	
Summary Report	10
Hydrograph Reports	11
Hydrograph No. 1, SCS Runoff, DA-FR-120D PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-120D DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-120D FOR	13
10 - Year	
Summary Report	14
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-120D PRE	15
Hydrograph No. 2, SCS Runoff, DA-FR-120D DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-120D FOR	17
IDE Papart	10



Hydrograph Return Period Recap Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	Hydrograph	Inflow				Hydrograph					
lo.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		0.368	0.522			1.439				DA-FR-120D PRE
2	SCS Runoff		0.333	0.481			1.377				DA-FR-120D DEV
3	SCS Runoff		0.094	0.187			0.889				DA-FR-120D FOR

Proj. file: DA-FR-120D_Hydraflow.gpw

Friday, 08 / 18 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.368	1	720	969				DA-FR-120D PRE
2	SCS Runoff	0.333	1	721	902				DA-FR-120D DEV
2									
DA	-FR-120D_Hy	⊣ /draflow.g	Jpw	<u> </u>	Return F	Period: 1 Ye	ear	Friday, 08 /	18 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

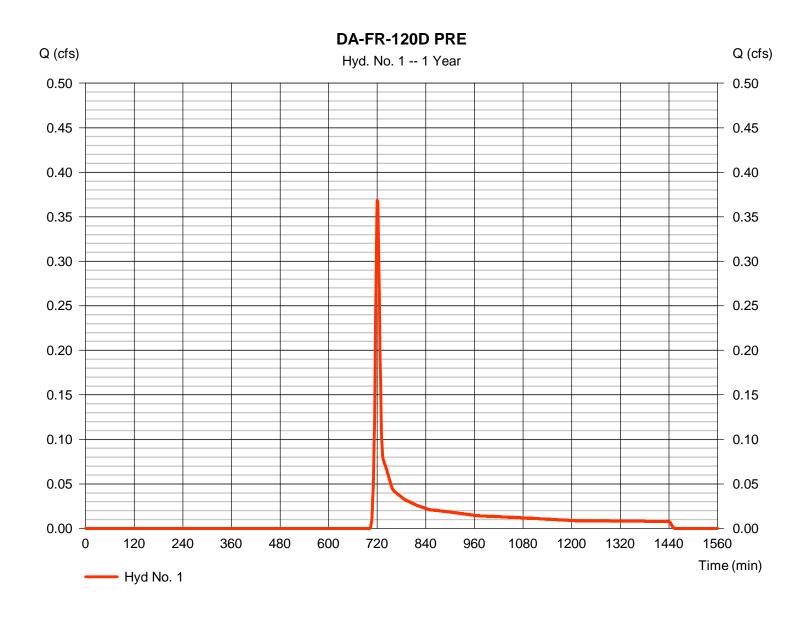
Friday, 08 / 18 / 2017

Hyd. No. 1

DA-FR-120D PRE

Hydrograph type = SCS Runoff Peak discharge = 0.368 cfsStorm frequency Time to peak = 1 yrs= 720 min Time interval = 1 min Hyd. volume = 969 cuft Drainage area Curve number = 0.440 ac= 64*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.60 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.052 \times 82) + (0.012 \times 87) + (0.006 \times 100) + (0.229 \times 55) + (0.136 \times 70)] / 0.440$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1

DA-FR-120D PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 16.79		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 8.53	+	0.00	+	0.00	=	8.53
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 63.03 = 31.68 = Unpaved =9.08	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.12	+	0.00	+	0.00	=	0.12
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							8.60 min

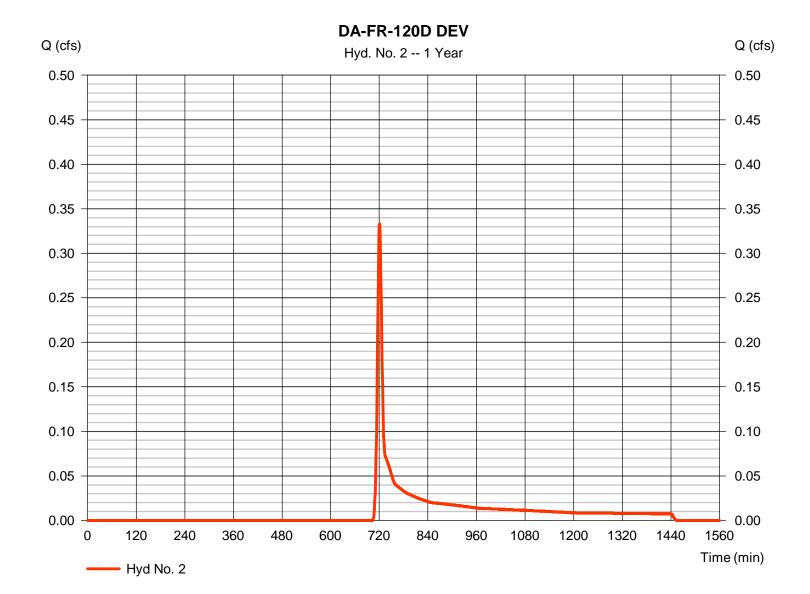
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 2

DA-FR-120D DEV

Hydrograph type = SCS Runoff Peak discharge = 0.333 cfsStorm frequency Time to peak = 721 min = 1 yrsTime interval = 1 min Hyd. volume = 902 cuft Drainage area Curve number = 0.440 ac= 63 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 8.60 \, \text{min}$ Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2

DA-FR-120D DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>				
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 16.79		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00						
Travel Time (min)	= 8.53	+	0.00	+	0.00	=	8.53				
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 63.03 = 31.68 = Unpave =9.08	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00						
Travel Time (min)	= 0.12	+	0.00	+	0.00	=	0.12				
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.400 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015						
Flow length (ft)	({0})0.0		0.0		0.0						
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00				
Total Travel Time, Tc											

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

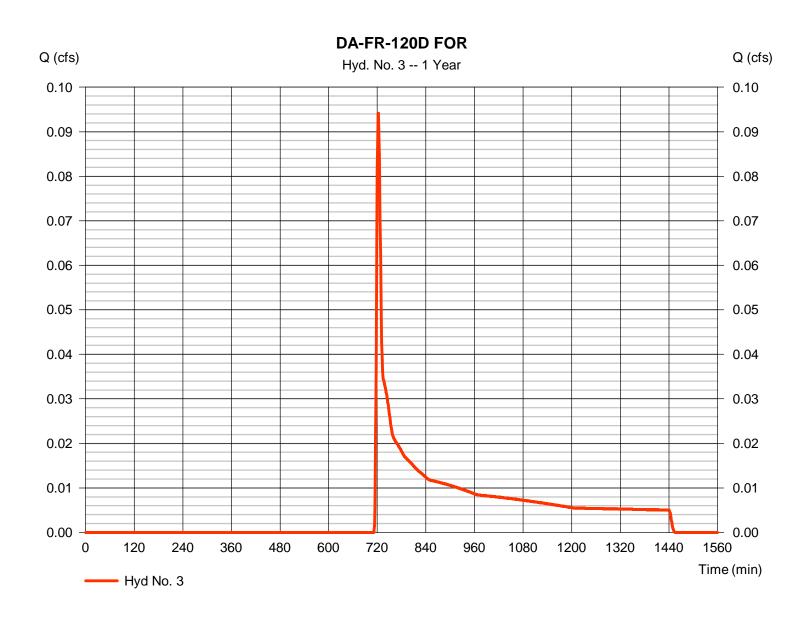
Friday, 08 / 18 / 2017

Hyd. No. 3

DA-FR-120D FOR

Hydrograph type = SCS Runoff Peak discharge = 0.094 cfsStorm frequency Time to peak = 723 min = 1 yrsTime interval = 1 min Hyd. volume = 449 cuft Drainage area Curve number = 0.440 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.60 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.050 \times 82) + (0.010 \times 87) + (0.010 \times 100) + (0.230 \times 55) + (0.140 \times 70)] / 0.440$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3

DA-FR-120D FOR

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 16.79		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 8.53	+	0.00	+	0.00	=	8.53
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 63.03 = 31.68 = Unpave =9.08	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.12	+	0.00	+	0.00	=	0.12
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.400 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							8.60 min

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description		
1	SCS Runoff	0.522	1	720	1,291				DA-FR-120D PRE		
2	SCS Runoff	0.481	1	720	1,213				DA-FR-120D DEV		
DA	-FR-120D_H ₃	/draflow.g	jpw	1	Return F	Return Period: 2 Year			Friday, 08 / 18 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

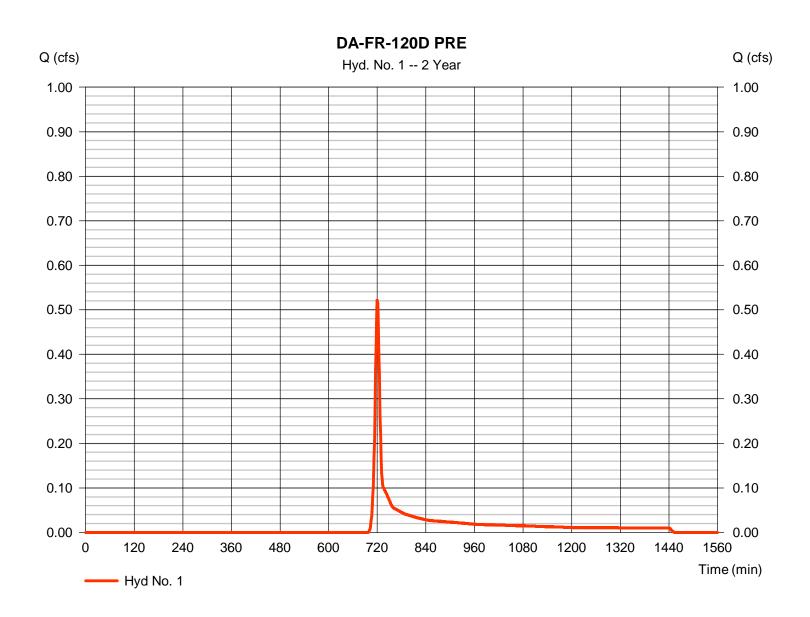
Friday, 08 / 18 / 2017

Hyd. No. 1

DA-FR-120D PRE

Hydrograph type = SCS Runoff Peak discharge = 0.522 cfsStorm frequency Time to peak = 2 yrs= 720 min Time interval = 1 min Hyd. volume = 1.291 cuftDrainage area Curve number = 0.440 ac= 64*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.60 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Shape factor Storm duration = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.052 \times 82) + (0.012 \times 87) + (0.006 \times 100) + (0.229 \times 55) + (0.136 \times 70)] / 0.440$



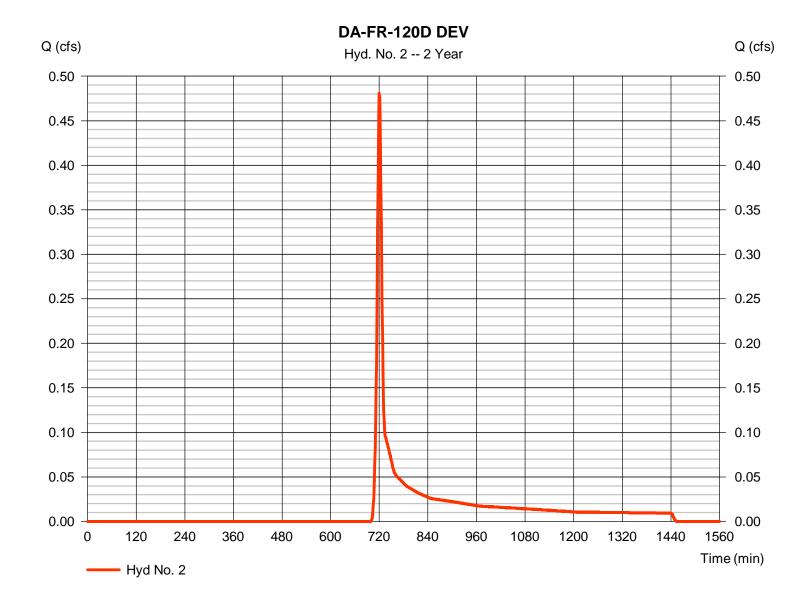
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 2

DA-FR-120D DEV

= SCS Runoff Hydrograph type Peak discharge = 0.481 cfsStorm frequency Time to peak = 720 min = 2 yrsTime interval = 1 min Hyd. volume = 1.213 cuftDrainage area Curve number = 0.440 ac= 63Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 8.60 \, \text{min}$ Total precip. Distribution = 3.70 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

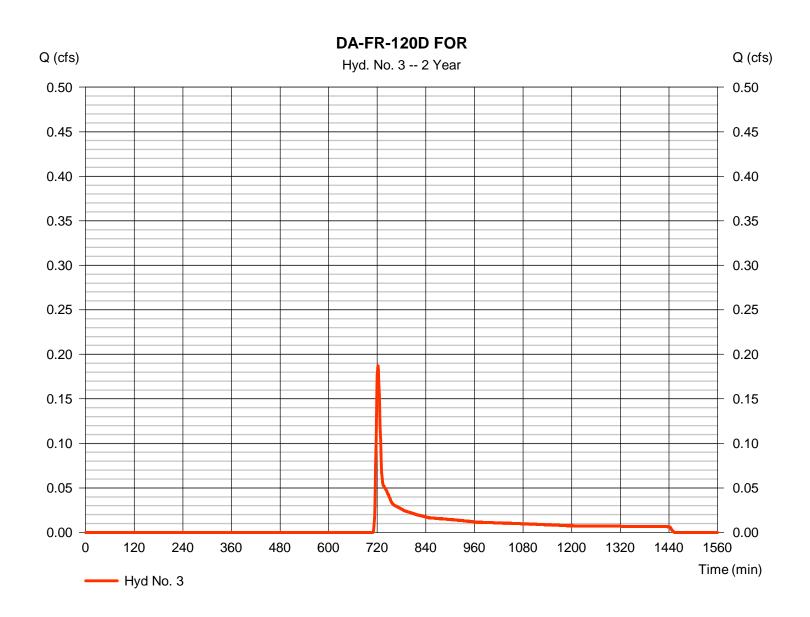
Friday, 08 / 18 / 2017

Hyd. No. 3

DA-FR-120D FOR

Hydrograph type = SCS Runoff Peak discharge = 0.187 cfsStorm frequency Time to peak = 722 min = 2 yrsTime interval = 1 min Hyd. volume = 664 cuft Drainage area Curve number = 0.440 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.60 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Shape factor Storm duration = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.050 \times 82) + (0.010 \times 87) + (0.010 \times 100) + (0.230 \times 55) + (0.140 \times 70)] / 0.440$



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	1.439	1	720	3,277				DA-FR-120D PRE
2	SCS Runoff	1.377	1	720	3,146				DA-FR-120D DEV
DA	 -FR-120D_Hy	⊥ ydraflow.g	J Jpw		Return F	Return Period: 10 Year			18 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

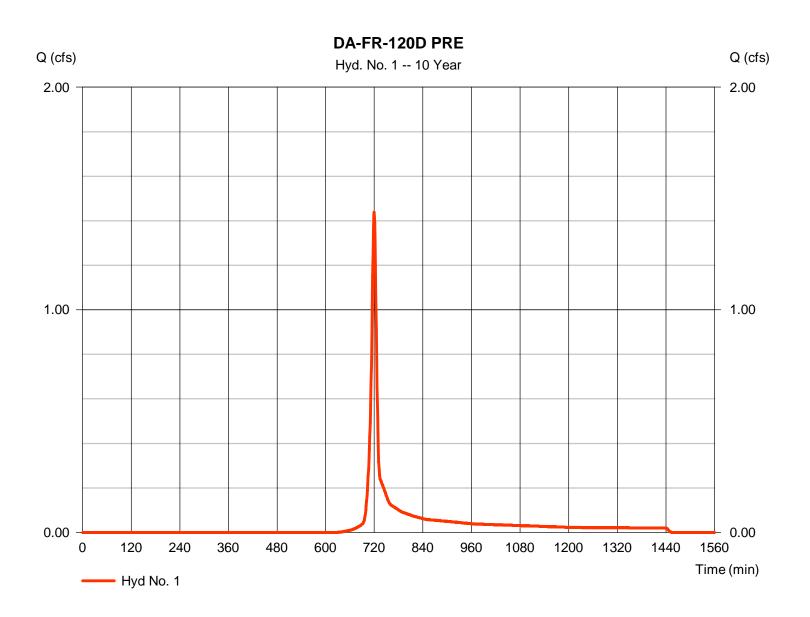
Friday, 08 / 18 / 2017

Hyd. No. 1

DA-FR-120D PRE

= SCS Runoff Hydrograph type Peak discharge = 1.439 cfsStorm frequency Time to peak = 720 min = 10 yrsTime interval = 1 min Hyd. volume = 3.277 cuftDrainage area Curve number = 0.440 ac= 64*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.60 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.052 \times 82) + (0.012 \times 87) + (0.006 \times 100) + (0.229 \times 55) + (0.136 \times 70)] / 0.440$



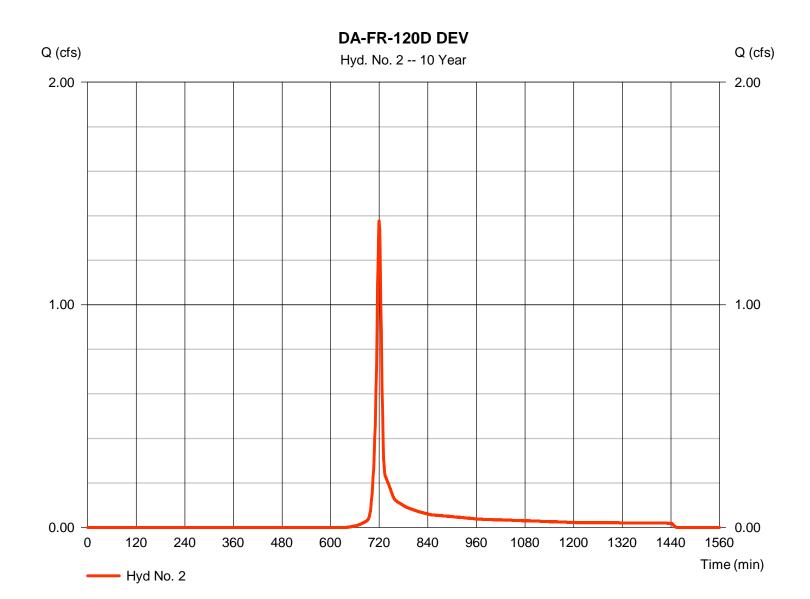
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 2

DA-FR-120D DEV

= SCS Runoff Hydrograph type Peak discharge = 1.377 cfsStorm frequency Time to peak = 720 min = 10 yrsTime interval = 1 min Hyd. volume = 3,146 cuftDrainage area Curve number = 0.440 ac= 63 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.60 \, \text{min}$ Total precip. Distribution = 5.70 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

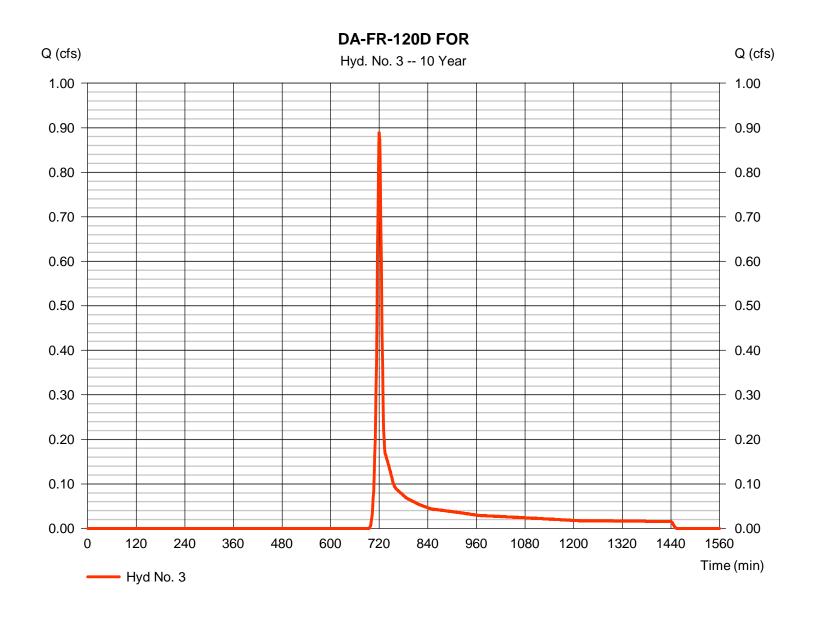
Friday, 08 / 18 / 2017

Hyd. No. 3

DA-FR-120D FOR

Hydrograph type = SCS Runoff Peak discharge = 0.889 cfsStorm frequency Time to peak = 720 min = 10 yrsTime interval = 1 min Hyd. volume = 2,154 cuftDrainage area Curve number = 0.440 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.60 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Shape factor Storm duration = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.050 \times 82) + (0.010 \times 87) + (0.010 \times 100) + (0.230 \times 55) + (0.140 \times 70)] / 0.440$



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Return Period	Intensity-Du	ıration-Frequency Ed	quation Coefficients ((FHA)
(Yrs)	В	D	E	(N/A)
1	0.0000	0.0000	0.0000	
2	69.8703	13.1000	0.8658	
3	0.0000	0.0000	0.0000	
5	79.2597	14.6000	0.8369	
10	88.2351	15.5000	0.8279	
25	102.6072	16.5000	0.8217	
50	114.8193	17.2000	0.8199	
100	127.1596	17.8000	0.8186	

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return		Intensity Values (in/hr)													
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60			
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70			
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15			
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46			
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91			
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25			
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60			

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc

		R	ainfall P	recipitat	ion Tab	le (in)		
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00



ENERGY BALANCE METHOD

		ts

	1-Y	r Event
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)
Pre-Developed Condition	0.303	1022
Developed Condition	0.127	782
Pre-Developed (Forest) Condition	0.200	836

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.9

Calculations:	Check #1:	$Q_{developed} \le IF \times \left[\left(Q_{pre-developed} \times RV_{pre-developed} \right) / RV_{developed} \right]$	0.127	≤ OK	0.356
	Check #2:	$Q_{developed} \le Q_{pre-developed}$	0.127	≤ OK	0.303
	Check #3:	$Q_{developed} \underline{shall \ not}$ be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed} \longrightarrow$	0.127	<u>shall not</u> be required to be ≤	0.214

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	·
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	<mark>0.40</mark>
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 – Manning's n Values for Open Channel Flow

nan	nel T	уре		Manning n	
			Min.	Normal	Max
1.	Exc	avated or Dredged Channels¹			
	a.	Earth, Straight, and Uniform:			
		Clean, recently completed	0.016	0.018	0.020
		Clean, after weathering	0.018	0.022	0.02
		Gravel, uniform section, clean	0.022	0.025	0.030
		With short grass, few weeds	0.022	0.027	0.03
	b.	Earth Winding and Sluggish:			
		No vegetation	0.023	0.025	0.030
		Grass, some weeds	0.025	0.030	0.03
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040
		Earth bottom and rubble sides	0.028	0.030	0.03
		Stony bottom and weedy banks	0.025	0.035	0.040
		Cobble bottom and clean sides	0.030	<mark>0.040</mark>	0.050
	c.	Dragline-Excavated or Dredged:		•	
		No vegetation	0.025	0.028	0.03
		Light brush on banks	0.035	0.050	0.06
	d.	Rock Cuts:			
		Smooth and uniform	0.025	0.035	0.040
		Jagged and irregular	0.035	0.040	0.050
	e.	Channels not Maintained, Weeds and Brush Uncut:			
		Dense weeds, high as flow depth	0.050	0.080	0.120
		Clean bottom, brush on sides	0.040	0.050	0.080
		Same as above, highest stage of flow	0.045	0.070	0.110
		Dense brush, high stage	0.080	0.100	0.140
2.	Mai	n Channels²			
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.03
	b.	Same as above, but more stones and weeds	0.030	0.035	0.04
	c.	Clean, winding, some pools and shoals	0.033	0.040	0.04
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.05
	f.	Same as (d) with more stones	0.045	0.050	0.060
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

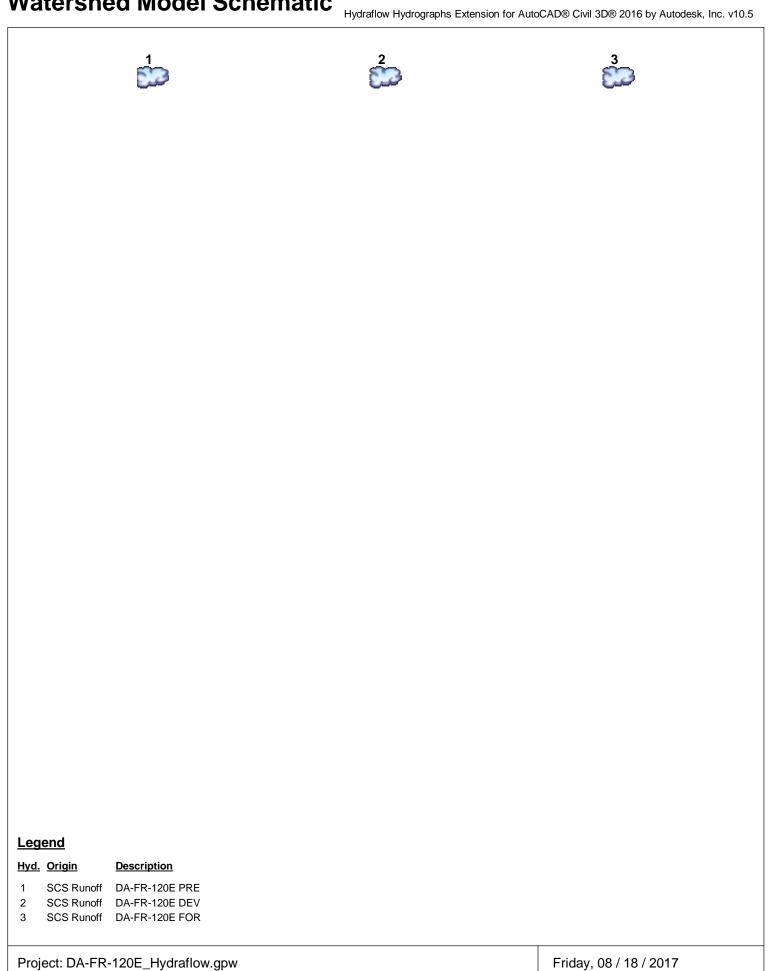
-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

Friday, 08 / 18 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-120E PRE	4
TR-55 Tc Worksheet	. 5
Hydrograph No. 2, SCS Runoff, DA-FR-120E DEV	6
TR-55 Tc Worksheet	
Hydrograph No. 3, SCS Runoff, DA-FR-120E FOR	
TR-55 Tc Worksheet	. 9
2 - Year	
Summary Report	10
Hydrograph Reports	11
Hydrograph No. 1, SCS Runoff, DA-FR-120E PRE	11
Hydrograph No. 2, SCS Runoff, DA-FR-120E DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-120E FOR	
10 - Year	
Summary Report	14
Hydrograph Reports	 15
Hydrograph No. 1, SCS Runoff, DA-FR-120E PRE	15
Hydrograph No. 2, SCS Runoff, DA-FR-120E DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-120E FOR	17
IDF Report	18



Hydrograph Return Period Recap Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	Hydrograph	Inflow		Peak Outflow (cfs)						Hydrograph Description	
lo.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		0.303	0.526			2.036				DA-FR-120E PRE
2	SCS Runoff		0.127	0.273			1.481				DA-FR-120E DEV
3	SCS Runoff		0.200	0.390			1.799				DA-FR-120E FOR

Proj. file: DA-FR-120E_Hydraflow.gpw

Friday, 08 / 18 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.303	1	721	1,022				DA-FR-120E PRE
2	SCS Runoff	0.127	1	724	782				DA-FR-120E DEV
DA	-FR-120E_H	/draflow.g	jpw	1	Return F	Period: 1 Ye	ear	Friday, 08 /	18 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

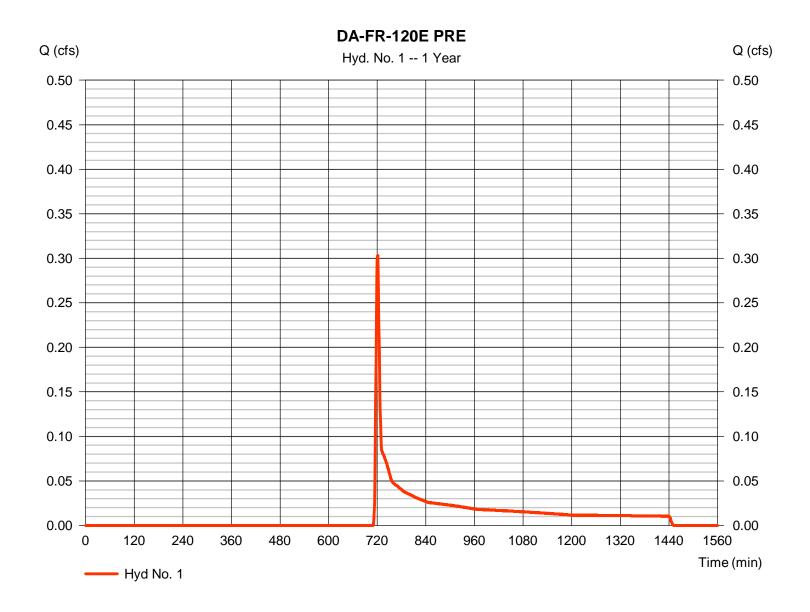
Friday, 08 / 18 / 2017

Hyd. No. 1

DA-FR-120E PRE

Hydrograph type = SCS Runoff Peak discharge = 0.303 cfsStorm frequency Time to peak = 721 min = 1 yrsTime interval = 1 min Hyd. volume = 1.022 cuftDrainage area Curve number = 0.840 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 7.80 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.030 \times 82) + (0.010 \times 100) + (0.800 \times 55)] / 0.840$



Hyd. No. 1

DA-FR-120E PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 21.92		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 7.66	+	0.00	+	0.00	=	7.66
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 74.04 = 24.24 = Unpave =7.94	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.16	+	0.00	+	0.00	=	0.16
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							7.80 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

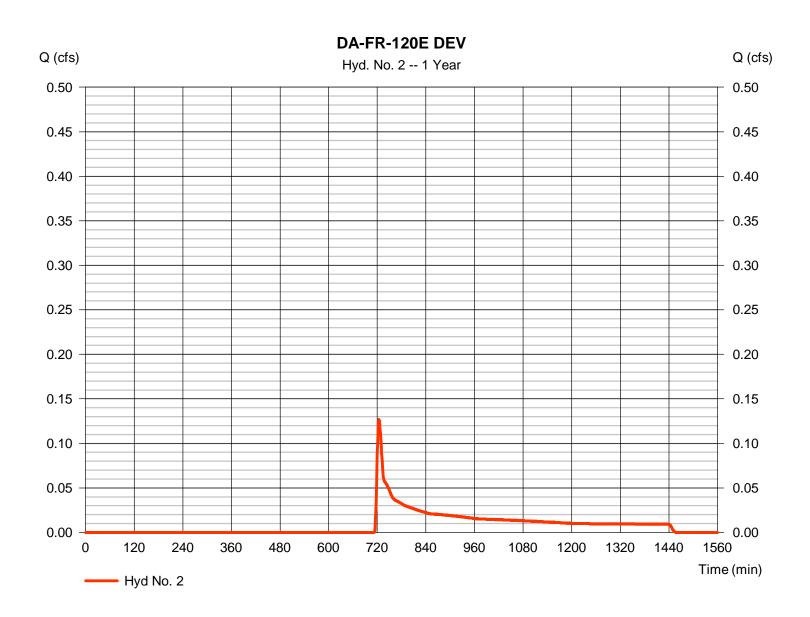
Friday, 08 / 18 / 2017

Hyd. No. 2

DA-FR-120E DEV

Hydrograph type = SCS Runoff Peak discharge = 0.127 cfsStorm frequency Time to peak = 724 min = 1 yrsTime interval = 1 min Hyd. volume = 782 cuft Drainage area Curve number = 0.840 ac= 54*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 11.30 min Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.466 \times 48) + (0.030 \times 82) + (0.314 \times 58) + (0.013 \times 100) + (0.020 \times 55)] / 0.840$



Hyd. No. 2

DA-FR-120E DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 55.0 = 3.70 = 22.76 = 4.68	+	0.800 25.1 3.70 22.63	+	0.400 17.5 3.70 18.40	=	11.08
, ,							
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 22.37 = 27.99 = Unpaved =8.54	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.04	+	0.00	+	0.00	=	0.04
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})40.2		0.0		0.0		
Travel Time (min)	= 0.14	+	0.00	+	0.00	=	0.14
Total Travel Time, Tc							11.30 min

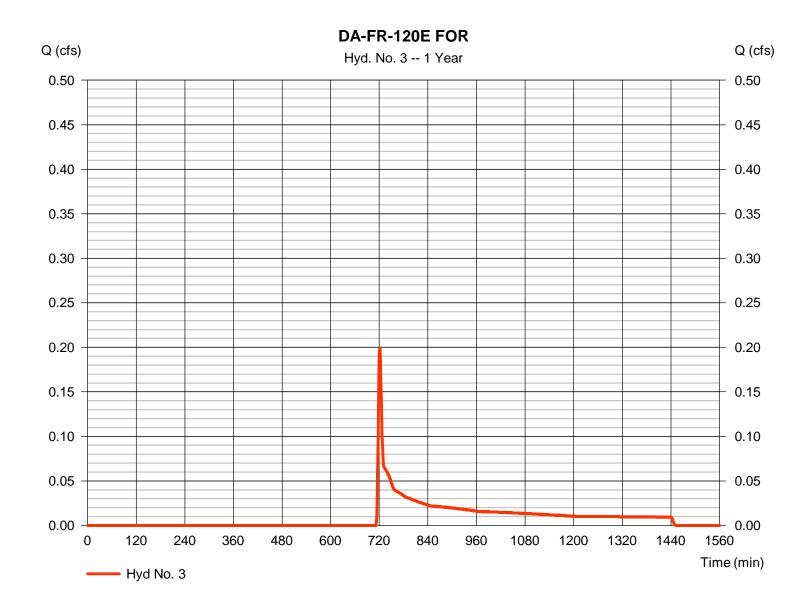
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 3

DA-FR-120E FOR

Hydrograph type = SCS Runoff Peak discharge = 0.200 cfsStorm frequency Time to peak = 721 min = 1 yrsTime interval = 1 min Hyd. volume = 836 cuft Drainage area Curve number = 0.840 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 7.80 \, \text{min}$ Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hyd. No. 3

DA-FR-120E FOR

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 21.92		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 7.66	+	0.00	+	0.00	=	7.66
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 74.04 = 24.24 = Unpave =7.94	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.16	+	0.00	+	0.00	=	0.16
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							7.80 min

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.526	1	720	1,466				DA-FR-120E PRE
2	SCS Runoff	0.273	1	723	1,176				DA-FR-120E DEV
DA	-FR-120E_H ₃	/draflow.g	lbw		Return F	Return Period: 2 Year			18 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

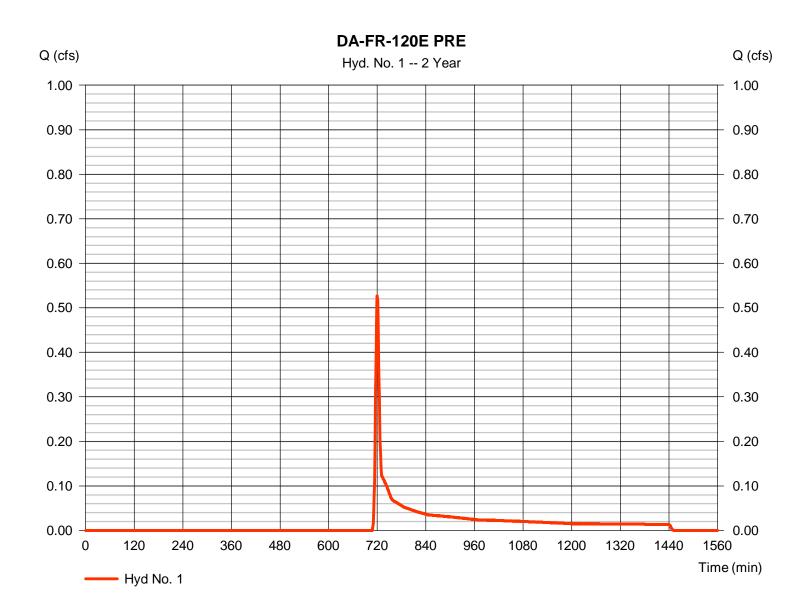
Friday, 08 / 18 / 2017

Hyd. No. 1

DA-FR-120E PRE

Hydrograph type = SCS Runoff Peak discharge = 0.526 cfsStorm frequency Time to peak = 720 min = 2 yrsTime interval = 1 min Hyd. volume = 1,466 cuftDrainage area Curve number = 0.840 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 7.80 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.030 \times 82) + (0.010 \times 100) + (0.800 \times 55)] / 0.840$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

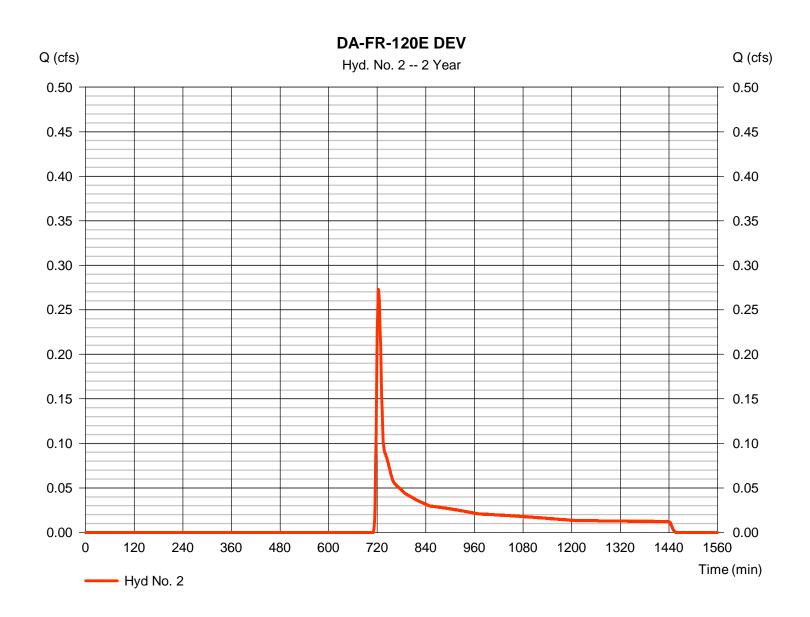
Friday, 08 / 18 / 2017

Hyd. No. 2

DA-FR-120E DEV

Hydrograph type = SCS Runoff Peak discharge = 0.273 cfsStorm frequency Time to peak = 723 min = 2 yrsTime interval = 1 min Hyd. volume = 1,176 cuftCurve number Drainage area = 0.840 ac= 54* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 11.30 min Total precip. = 3.70 inDistribution = Type II Shape factor Storm duration = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.466 \times 48) + (0.030 \times 82) + (0.314 \times 58) + (0.013 \times 100) + (0.020 \times 55)] / 0.840$



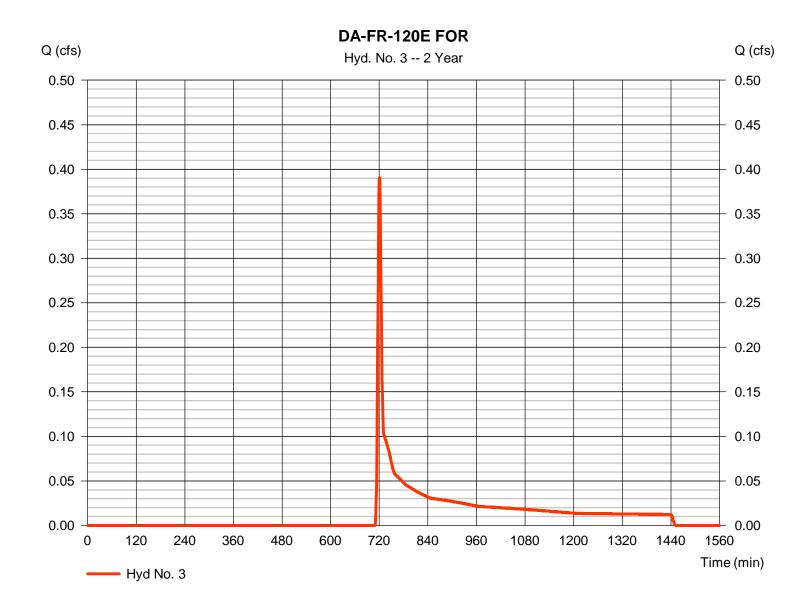
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 3

DA-FR-120E FOR

= SCS Runoff Hydrograph type Peak discharge = 0.390 cfsStorm frequency Time to peak = 721 min = 2 yrsTime interval = 1 min Hyd. volume = 1.236 cuftDrainage area Curve number = 0.840 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 7.80 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

yd. o.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	2.036	1	719	4,450				DA-FR-120E PRE
2	SCS Runoff	1.481	1	721	3,961				DA-FR-120E DEV
2 3									
DΑ	⊥ -FR-120E_H _!	_ ydraflow.¢	gpw	<u> </u>	Return F	Period: 10 \	⊥ ∕ear	Friday, 08 /	′ 18 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

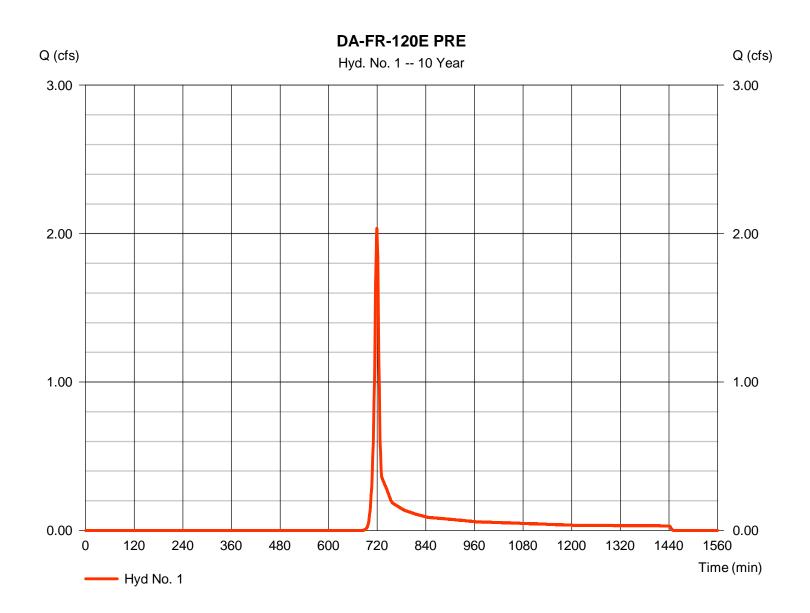
Friday, 08 / 18 / 2017

Hyd. No. 1

DA-FR-120E PRE

= SCS Runoff Hydrograph type Peak discharge = 2.036 cfsStorm frequency Time to peak = 719 min = 10 yrsTime interval = 1 min Hyd. volume = 4,450 cuft= 57* Curve number Drainage area = 0.840 acBasin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 7.80 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.030 \times 82) + (0.010 \times 100) + (0.800 \times 55)] / 0.840$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

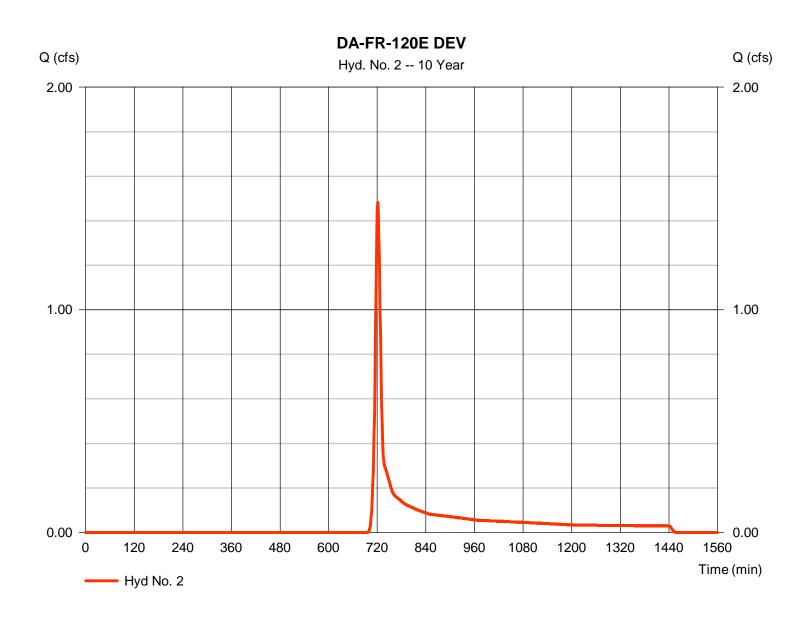
Friday, 08 / 18 / 2017

Hyd. No. 2

DA-FR-120E DEV

= SCS Runoff Hydrograph type Peak discharge = 1.481 cfsStorm frequency Time to peak = 721 min = 10 yrsTime interval = 1 min Hyd. volume = 3.961 cuftDrainage area Curve number = 0.840 ac= 54*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 11.30 min Total precip. = 5.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.466 \times 48) + (0.030 \times 82) + (0.314 \times 58) + (0.013 \times 100) + (0.020 \times 55)] / 0.840$



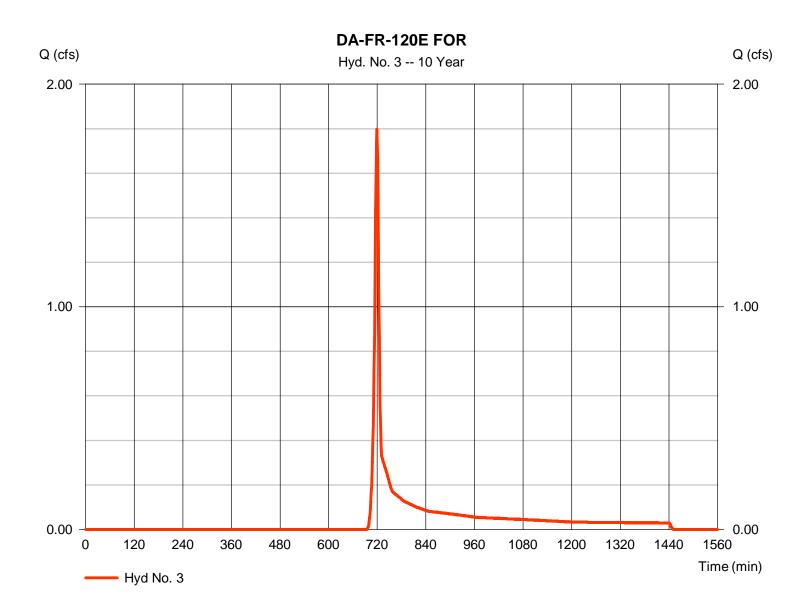
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Hyd. No. 3

DA-FR-120E FOR

= SCS Runoff = 1.799 cfsHydrograph type Peak discharge Storm frequency Time to peak = 719 min = 10 yrsTime interval = 1 min Hyd. volume = 4,009 cuftDrainage area Curve number = 0.840 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 7.80 \, \text{min}$ Total precip. Distribution = 5.70 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Friday, 08 / 18 / 2017

Return Period	Intensity-Du	ensity-Duration-Frequency Equation Coefficients (FHA)								
(Yrs)	В	D	E	(N/A)						
1	0.0000	0.0000	0.0000							
2	69.8703	13.1000	0.8658							
3	0.0000	0.0000	0.0000							
5	79.2597	14.6000	0.8369							
10	88.2351	15.5000	0.8279							
25	102.6072	16.5000	0.8217							
50	114.8193	17.2000	0.8199							
100	127.1596	17.8000	0.8186							

File name: SampleFHA.idf

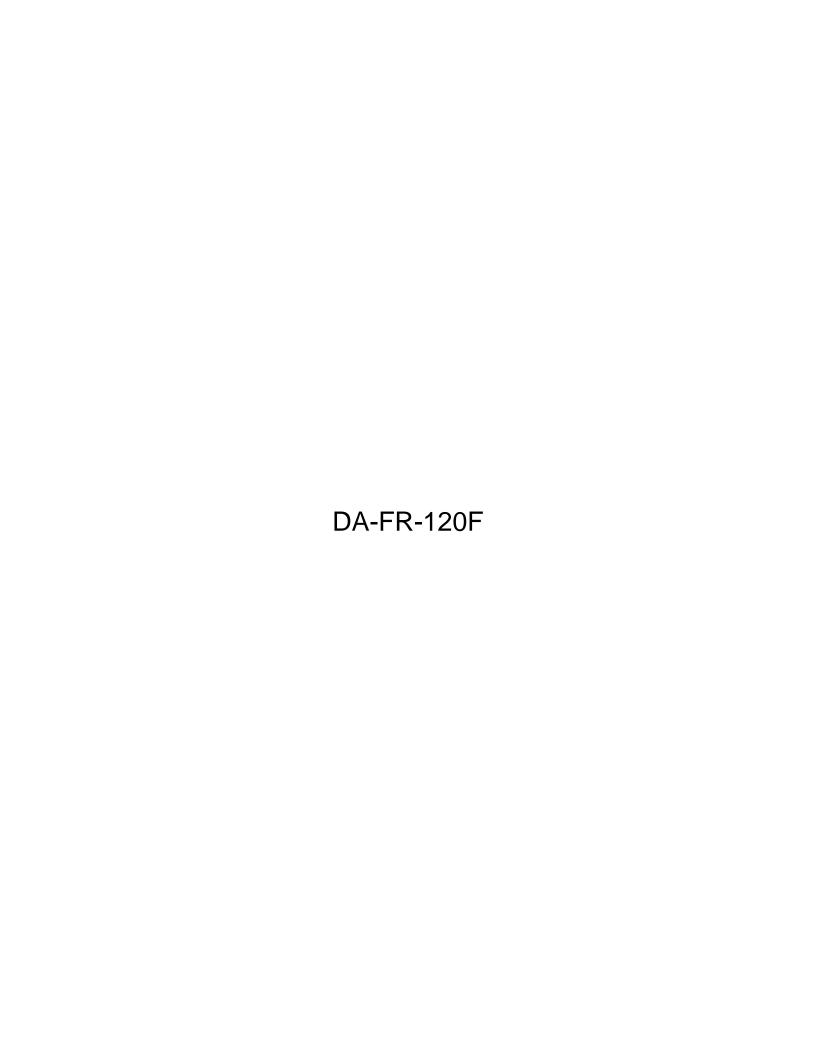
Intensity = $B/(Tc + D)^E$

Return					Intens	sity Values	(in/hr)					
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc

	Rainfall Precipitation Table (in)										
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr			
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00			
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			



STORAGE VOLUME OF WATERBAR WITH SOIL COMPOST AMENDMENT AREA

Equations Used:

¹Vgravel storage = L*W*D_{gravel}*(40/100) ²Vsoil storage = L*W*D_{soil}*(20/100)

³Vsurface storage = [W*S*D^2]+[L*S*D^2]+[W*L*D]+[((2*S*D)^2*D)/3]

¹Equation #2b under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, assuming that gravel is made up of 40% voids.

²Equation #2b under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, assuming that soil compost amendment is made up of 20% voids.

³Equation #1 under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, but calculation also takes into account surface side slopes.

Inputs:		of Gravel Layer, D _{gravel} (ft) =		
	Depth of Soil	Amendment Area, D _{soil} (ft) =	1	Refer to Table 4.3 in VA DEQ Stormwater Design Specification No. 4; Note that compost amendment may not be necessary for HSG A/B soils
	Length of Waterbar So	oil Amendment Area, L (ft) =	50	Assume max. length of 50' for waterbar soil amendment areas (i.e., limited to permanent ROW)
	Width of Waterbar Soil	I Amendment Area, W (ft) =	3	
	Inside Embani	kment Side Slopes, S (H:V) =	2	Assume 2H:1V surface side slopes for waterbars
	Number of Perm. Water	erbars in Drainage Area, n =	5	
	Design I	Infiltration Rate, IR (in/hr) =	0.2	Min. rate of 0.30 in/hr for HSG A soils and 0.15-0.30 in/hr for HSG B soils (see Chap. 4, p. 4-30 in VA Stormwater Management Handbook Volume II (First Edition, 1999)
	Surf	face Ponding Depth, D (ft) =	0.5	Assume 0.5' CFS height at the end of waterbars
Calculations:	Total St	torage Depth per BMP (ft) =	1.5	
	Surface Stor	rage Volume per BMP (cf) =	102	
	Subsurface Stor	rage Volume per BMP (cf) =	30	
	Total Stor	rage Volume per BMP (cf) =	132	
	Total BMP Storage Vol	lume in Drainage Area (cf) =	661	
	Calculated Infiltr	ation Period per BMP (hr) =	53	
		Depth-Storage D	Pata	
Depth (ft)	Width (ft)	Length (ft)	Storage Volume per BMP (cf)	Storage Volume in Drainage Area (cf)
0	3	50	0	0
0.5	3	50	15	75
1	3	50	30	150
1.5	5	52	132	661
2	7	54	291	1457

ENERGY BALANCE METHOD

Inputs:

	1-	Yr Event
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)
Pre-Developed Condition	1.178	6327
Developed Condition	0.905	5666
Pre-Developed (Forest) Condition	0.771	5176

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.8

Calculations:	Check #1:	$Q_{developed} \le IF \times [(Q_{pre-developed} \times RV_{pre-developed}) / RV_{developed}]$ >	0.905	≤ OK	1.052
	Check #2:	Q _{developed} ≤ Q _{pre-developed} ->	0.905	≤ OK	1.178
	Check #3:	$Q_{developed} \frac{Shall\ not}{N}$ be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed}$ >	0.905	shall not be required to be ≤	0.704

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	•
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 – Manning's n Values for Open Channel Flow

nan	nel T	уре		Manning n	
			Min.	Normal	Max
1.	Exc	avated or Dredged Channels¹			
	a.	Earth, Straight, and Uniform:			
		Clean, recently completed	0.016	0.018	0.020
		Clean, after weathering	0.018	0.022	0.02
		Gravel, uniform section, clean	0.022	0.025	0.030
		With short grass, few weeds	0.022	0.027	0.03
	b.	Earth Winding and Sluggish:			
		No vegetation	0.023	0.025	0.030
		Grass, some weeds	0.025	0.030	0.03
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040
		Earth bottom and rubble sides	0.028	0.030	0.03
		Stony bottom and weedy banks	0.025	0.035	0.040
		Cobble bottom and clean sides	0.030	<mark>0.040</mark>	0.050
	c.	Dragline-Excavated or Dredged:		•	
		No vegetation	0.025	0.028	0.03
		Light brush on banks	0.035	0.050	0.06
	d.	Rock Cuts:			
		Smooth and uniform	0.025	0.035	0.040
		Jagged and irregular	0.035	0.040	0.050
	e.	Channels not Maintained, Weeds and Brush Uncut:			
		Dense weeds, high as flow depth	0.050	0.080	0.120
		Clean bottom, brush on sides	0.040	0.050	0.080
		Same as above, highest stage of flow	0.045	0.070	0.110
		Dense brush, high stage	0.080	0.100	0.140
2.	Mai	n Channels²			
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.03
	b.	Same as above, but more stones and weeds	0.030	0.035	0.04
	c.	Clean, winding, some pools and shoals	0.033	0.040	0.04
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.05
	f.	Same as (d) with more stones	0.045	0.050	0.060
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

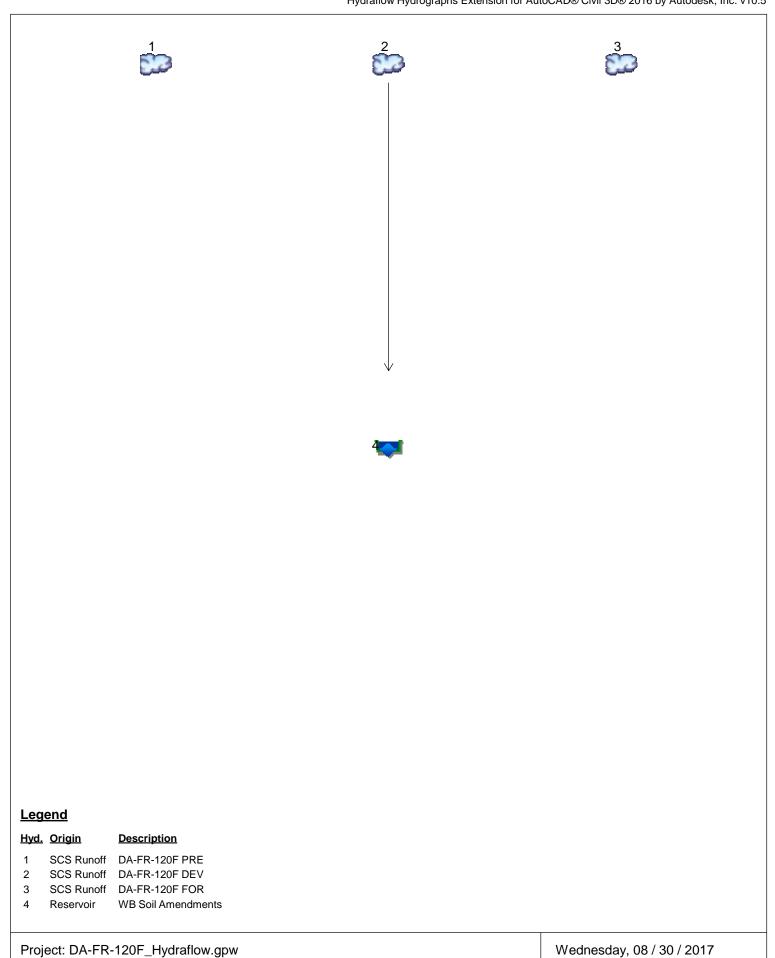
-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

Wednesday, 08 / 30 / 2017

Summary Reports	Watershed Model Schematic	. 1
Summary Reports	Hydrograph Return Period Recap	. 2
Hydrograph Reports. 4 Hydrograph No. 1, SCS Runoff, DA-FR-120F PRE. 4 TR-55 Tc Worksheet. 5 Hydrograph No. 2, SCS Runoff, DA-FR-120F DEV. 6 TR-55 Tc Worksheet. 7 Hydrograph No. 3, SCS Runoff, DA-FR-120F FOR. 8 TR-55 Tc Worksheet. 9 Hydrograph No. 4, Reservoir, WB Soil Amendments. 10 Pond Report - Waterbar Soil Amendments. 11 2 - Year Summary Reports. 14 Hydrograph No. 1, SCS Runoff, DA-FR-120F PRE. 14 Hydrograph No. 3, SCS Runoff, DA-FR-120F DEV. 15 Hydrograph No. 4, Reservoir, WB Soil Amendments. 17 10 - Year Summary Report. 18 Hydrograph Reports. 19 Hydrograph Reports. 19 Hydrograph No. 1, SCS Runoff, DA-FR-120F PRE. 19 Hydrograph No. 2, SCS Runoff, DA-FR-120F PRE. 19 Hydrograph No. 2, SCS Runoff, DA-FR-120F PGR. 21 Hydrograph No. 4, Reserv	1 - Year	
Hydrograph Reports. 4 Hydrograph No. 1, SCS Runoff, DA-FR-120F PRE. 4 TR-55 Tc Worksheet. 5 Hydrograph No. 2, SCS Runoff, DA-FR-120F DEV. 6 TR-55 Tc Worksheet. 7 Hydrograph No. 3, SCS Runoff, DA-FR-120F FOR. 8 TR-55 Tc Worksheet. 9 Hydrograph No. 4, Reservoir, WB Soil Amendments. 10 Pond Report - Waterbar Soil Amendments. 11 2 - Year Summary Reports. 14 Hydrograph No. 1, SCS Runoff, DA-FR-120F PRE. 14 Hydrograph No. 3, SCS Runoff, DA-FR-120F DEV. 15 Hydrograph No. 4, Reservoir, WB Soil Amendments. 17 10 - Year Summary Report. 18 Hydrograph Reports. 19 Hydrograph Reports. 19 Hydrograph No. 1, SCS Runoff, DA-FR-120F PRE. 19 Hydrograph No. 2, SCS Runoff, DA-FR-120F PRE. 19 Hydrograph No. 2, SCS Runoff, DA-FR-120F PGR. 21 Hydrograph No. 4, Reserv	Summary Report	3
TR-55 Tc Worksheet 5 Hydrograph No. 2, SCS Runoff, DA-FR-120F DEV 6 TR-55 Tc Worksheet 7 Hydrograph No. 3, SCS Runoff, DA-FR-120F FOR 8 TR-55 Tc Worksheet 9 Hydrograph No. 4, Reservoir, WB Soil Amendments 10 Pond Report - Waterbar Soil Amendments 11 2 - Year Summary Report 13 Hydrograph Reports 14 Hydrograph No. 1, SCS Runoff, DA-FR-120F PRE 14 Hydrograph No. 2, SCS Runoff, DA-FR-120F DEV 15 Hydrograph No. 3, SCS Runoff, DA-FR-120F FOR 16 Hydrograph No. 4, Reservoir, WB Soil Amendments 17 10 - Year Summary Report 18 Hydrograph No. 1, SCS Runoff, DA-FR-120F PRE 19 Hydrograph No. 2, SCS Runoff, DA-FR-120F DEV 20 Hydrograph No. 3, SCS Runoff, DA-FR-120F PRE 19 Hydrograph No. 3, SCS Runoff, DA-FR-120F FOR 21 Hydrograph No. 4, Reservoir, WB Soil Amendments 22 Hydrograph No. 4, Reservoir, WB Soil Amendments 22	Hydrograph Reports	4
Hydrograph No. 2, SCS Runoff, DA-FR-120F DEV	Hydrograph No. 1, SCS Runoff, DA-FR-120F PRE	4
TR-55 Tc Worksheet		
Hydrograph No. 3, SCS Runoff, DA-FR-120F FOR		
TR-55 Tc Worksheet	TR-55 Tc Worksheet	7
Hydrograph No. 4, Reservoir, WB Soil Amendments		
Pond Report - Waterbar Soil Amendments		
2 - Year Summary Report	Hydrograph No. 4, Reservoir, WB Soil Amendments	. 10
Summary Reports	Pond Report - Waterbar Soil Amendments	11
Summary Reports	O. V	
Hydrograph Reports	_ · · · · · ·	
Hydrograph No. 2, SCS Runoff, DA-FR-120F DEV		
Hydrograph No. 2, SCS Runoff, DA-FR-120F DEV	Hydrograph Reports	14
Hydrograph No. 3, SCS Runoff, DA-FR-120F FOR		
Hydrograph No. 4, Reservoir, WB Soil Amendments		
10 - Year Summary Report		
Summary Report	Hydrograph No. 4, Reservoir, WB Soil Amendments	. 17
Summary Report	10 - Voor	
Hydrograph Reports		10
Hydrograph No. 1, SCS Runoff, DA-FR-120F PRE		
Hydrograph No. 2, SCS Runoff, DA-FR-120F DEV	Hydrograph No. 1 SCS Pupoff DA-ED-120E DDE	10
Hydrograph No. 3, SCS Runoff, DA-FR-120F FOR		
Hydrograph No. 4, Reservoir, WB Soil Amendments		
IDF Report	Trydrograph No. 4, Neservoir, WD ooii Amendments	~~
	IDF Report	23



Hydrograph Return Period Recap Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hydrograph	Inflow				Hydrograph					
No. type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
SCS Runoff		1.178	2.095			8.865				DA-FR-120F PRE
SCS Runoff		1.178	2.095			8.865				DA-FR-120F DEV
SCS Runoff		0.771	1.531			7.742				DA-FR-120F FOR
Reservoir	2	0.905	1.950			8.803				WB Soil Amendments
	type (origin) SCS Runoff SCS Runoff SCS Runoff	type (origin) hyd(s) SCS Runoff SCS Runoff SCS Runoff	type (origin) hyd(s) 1-yr SCS Runoff 1.178 SCS Runoff 1.178 SCS Runoff 0.771	type (origin) hyd(s) 1-yr 2-yr SCS Runoff 1.178 2.095 SCS Runoff 1.178 2.095 SCS Runoff 0.771 1.531	type (origin) hyd(s) 1-yr 2-yr 3-yr SCS Runoff 1.178 2.095 SCS Runoff 1.178 2.095 SCS Runoff 0.771 1.531	type (origin) hyd(s) 1-yr 2-yr 3-yr 5-yr SCS Runoff 1.178 2.095 SCS Runoff 1.178 2.095 SCS Runoff 0.771 1.531	type (origin) hyd(s) 1-yr 2-yr 3-yr 5-yr 10-yr SCS Runoff 1.178 2.095 8.865 SCS Runoff 1.178 2.095 8.865 SCS Runoff 0.771 1.531 7.742	type (origin) hyd(s) 1-yr 2-yr 3-yr 5-yr 10-yr 25-yr SCS Runoff 1.178 2.095 8.865 SCS Runoff 1.178 2.095 8.865 SCS Runoff 0.771 1.531 7.742	type (origin) hyd(s) 1-yr 2-yr 3-yr 5-yr 10-yr 25-yr 50-yr SCS Runoff 1.178 2.095 8.865 SCS Runoff 0.771 1.531 7.742	type (origin) hyd(s) 1-yr 2-yr 3-yr 5-yr 10-yr 25-yr 50-yr 100-yr SCS Runoff 1.178 2.095 8.865 SCS Runoff 0.771 1.531 7.742

Proj. file: DA-FR-120F_Hydraflow.gpw

Wednesday, 08 / 30 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

yd. o.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	1.178	1	727	6,327				DA-FR-120F PRE
2	SCS Runoff	1.178	1	727	6,327				DA-FR-120F DEV
3	SCS Runoff	0.771	1	727	5,176				DA-FR-120F FOR
4	Reservoir	0.905	1	734	5,666	2	101.59	803	WB Soil Amendments
	-FR-120F_H	1	-1	1	Return I	1	1		y, 08 / 30 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

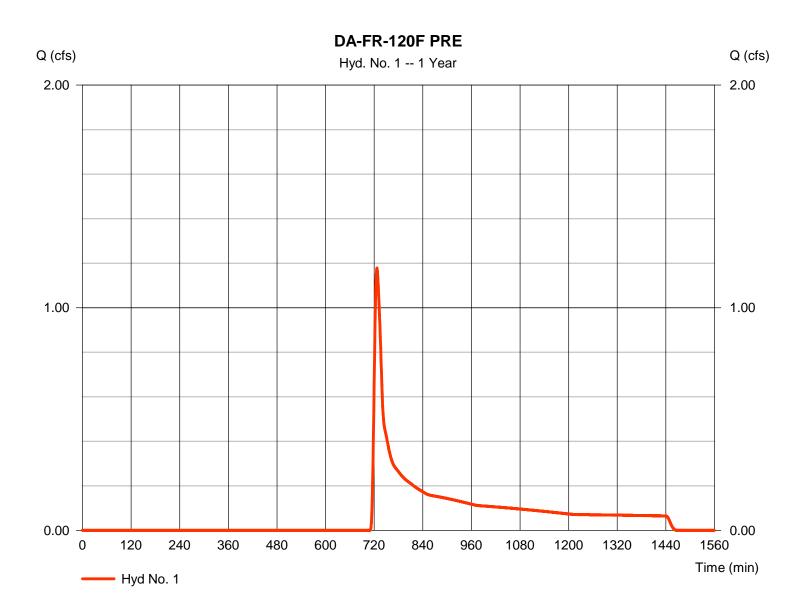
Wednesday, 08 / 30 / 2017

Hyd. No. 1

DA-FR-120F PRE

Hydrograph type = SCS Runoff Peak discharge = 1.178 cfsStorm frequency Time to peak = 727 min = 1 yrsTime interval = 1 minHyd. volume = 6.327 cuftCurve number Drainage area = 5.010 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.90 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.040 \times 65) + (0.001 \times 98) + (3.466 \times 58) + (0.014 \times 100) + (1.494 \times 55)] / 5.010$



Hyd. No. 1

DA-FR-120F PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 4.91		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		12.04
Travel Time (min)	= 13.94	+	0.00	+	0.00	=	13.94
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 611.45 = 10.13 = Unpaved =5.14	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.98	+	0.00	+	0.00	=	1.98
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

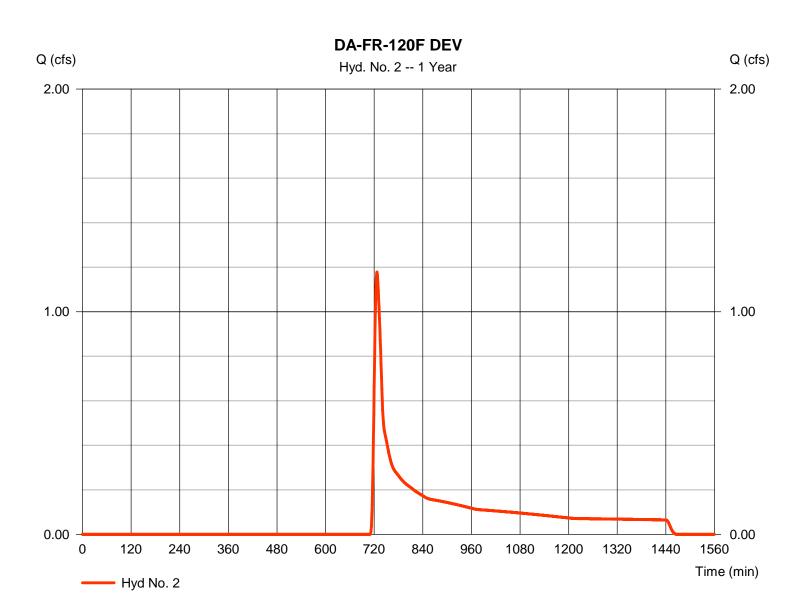
Wednesday, 08 / 30 / 2017

Hyd. No. 2

DA-FR-120F DEV

Hydrograph type = SCS Runoff Peak discharge = 1.178 cfsStorm frequency Time to peak = 727 min = 1 yrsTime interval = 1 minHyd. volume = 6.327 cuftCurve number Drainage area = 5.010 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.90 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.900 \times 48) + (0.040 \times 82) + (0.001 \times 98) + (4.045 \times 58) + (0.014 \times 100) + (0.014 \times 55)] / 5.010$



Hyd. No. 2

DA-FR-120F DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 4.91		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00			
Travel Time (min)	= 13.94	+	0.00	+	0.00	=	13.94	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 36.93 = 5.85 = Unpaved =3.90	i	549.32 10.60 Unpave 5.25	d	0.00 0.00 Paved 0.00			
Travel Time (min)	= 0.16	+	1.74	+	0.00	=	1.90	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015			
Flow length (ft)	({0})28.7		0.0		0.0			
Travel Time (min)	= 0.10	+	0.00	+	0.00	=	0.10	
Total Travel Time, Tc								

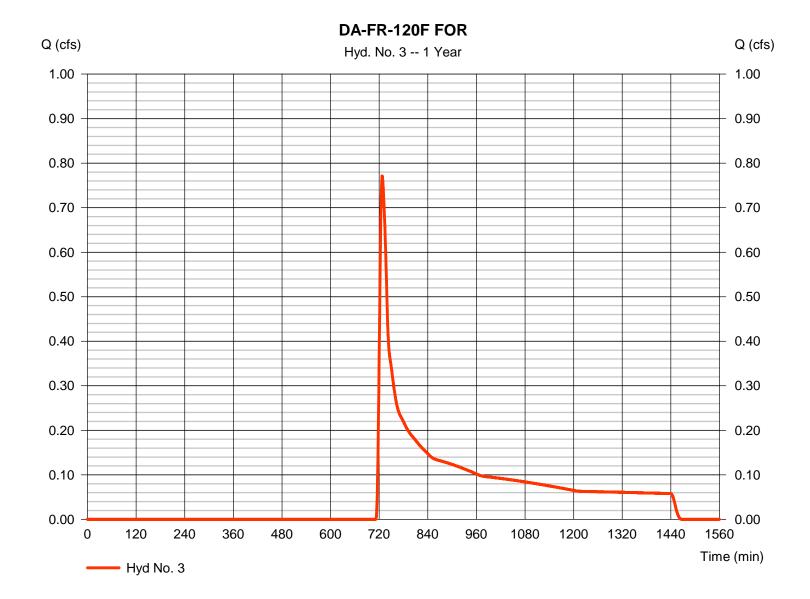
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 30 / 2017

Hyd. No. 3

DA-FR-120F FOR

Hydrograph type = SCS Runoff Peak discharge = 0.771 cfsStorm frequency Time to peak = 727 min = 1 yrsTime interval = 1 min Hyd. volume = 5,176 cuftDrainage area Curve number = 5.010 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 15.90 min = TR55 Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



Hyd. No. 3

DA-FR-120F FOR

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 4.91		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		12.04
Travel Time (min)	= 13.94	+	0.00	+	0.00	=	13.94
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 611.45 = 10.13 = Unpaved =5.14	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.98	+	0.00	+	0.00	=	1.98
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

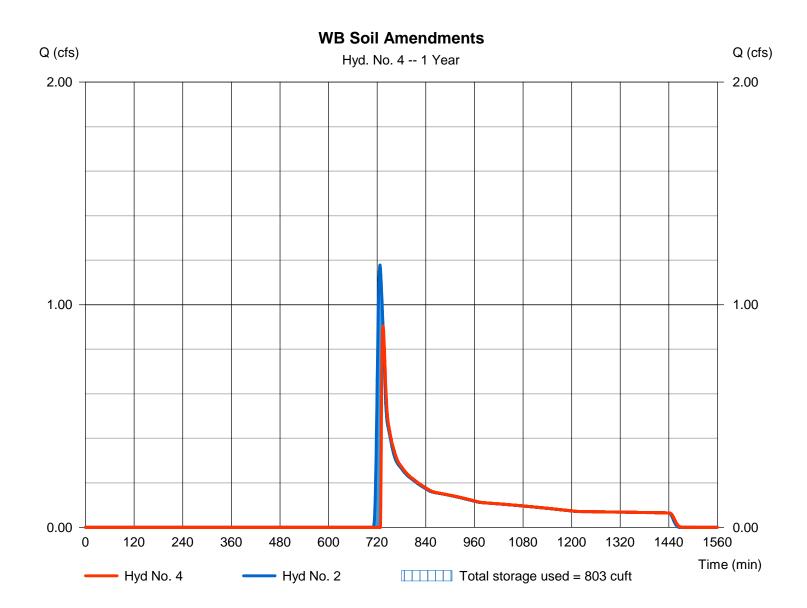
Wednesday, 08 / 30 / 2017

Hyd. No. 4

WB Soil Amendments

Hydrograph type Peak discharge = 0.905 cfs= Reservoir Storm frequency Time to peak = 734 min = 1 yrsTime interval = 1 minHyd. volume = 5,666 cuftInflow hyd. No. Max. Elevation = 2 - DA-FR-120F DEV = 101.59 ft= Waterbar Soil Amendments Reservoir name Max. Storage = 803 cuft

Storage Indication method used.



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 30 / 2017

Pond No. 1 - Waterbar Soil Amendments

Pond Data

Multi-Stage

Pond storage is based on user-defined values.

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	100.00	n/a	0	0
0.50	100.50	n/a	75	75
1.00	101.00	n/a	75	150
1.50	101.50	n/a	511	661
2.00	102.00	n/a	796	1,457

Culvert / Orifice Structures Weir Structures [D] [A] [B] [PrfRsr] [A] [B] [C] [C] Rise (in) = 0.000.00 0.00 0.00 Crest Len (ft) = 10.00 0.00 0.00 0.00 Span (in) = 0.000.00 0.00 0.00 Crest El. (ft) = 101.500.00 0.00 0.00 No. Barrels = 00 0 Weir Coeff. = 3.333.33 3.33 3.33 = 0.00 0.00 0.00 0.00 Weir Type = Rect Invert El. (ft) Length (ft) = 0.000.00 0.00 0.00 Multi-Stage = No No No No Slope (%) = 0.000.00 0.00 n/a N-Value = .013 .013 .013 n/a 0.60 0.60 0.60 = 0.000 (by Wet area) Orifice Coeff. = 0.60Exfil.(in/hr)

TW Elev. (ft)

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

= 0.00

Stage A	/ Storage /	Discharge	Table
---------	-------------	-----------	-------

= n/a

Stage ft	Storage cuft	Elevation ft	CIv A cfs	Clv B cfs	CIv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	100.00					0.00						0.000
0.05	8	100.05					0.00						0.000
0.10	15	100.10					0.00						0.000
0.15	23	100.15					0.00						0.000
0.20	30	100.20					0.00						0.000
0.25	38	100.25					0.00						0.000
0.30	45	100.30					0.00						0.000
0.35	53	100.35					0.00						0.000
0.40	60	100.40					0.00						0.000
0.45	68	100.45					0.00						0.000
0.50	75	100.50					0.00						0.000
0.55	83	100.55					0.00						0.000
0.60	90	100.60					0.00						0.000
0.65	98	100.65					0.00						0.000
0.70	105	100.70					0.00						0.000
0.75	113	100.75					0.00						0.000
0.80	120	100.80					0.00						0.000
0.85	128	100.85					0.00						0.000
0.90	135	100.90					0.00						0.000
0.95	143	100.95					0.00						0.000
1.00	150	101.00					0.00						0.000
1.05	201	101.05					0.00						0.000
1.10	252	101.10					0.00						0.000
1.15	303	101.15					0.00						0.000
1.20	354	101.20					0.00						0.000
1.25	406	101.25					0.00						0.000
1.30	457	101.30					0.00						0.000
1.35	508	101.35					0.00						0.000
1.40	559	101.40					0.00						0.000
1.45	610	101.45					0.00						0.000
1.50	661	101.50					0.00						0.000
1.55	741	101.55					0.37						0.372
1.60	820	101.60					1.05						1.053
1.65	900	101.65					1.93						1.935
1.70	979	101.70					2.98						2.979
1.75	1,059	101.75					4.16						4.163
1.80	1,139	101.80					5.47						5.472
1.85	1,218	101.85					6.90						6.896
1.90	1,298	101.90					8.43						8.425
											Continue	es on nex	t page

Continues on next page...

Waterbar Soil Amendments Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	CIv A cfs	Clv B cfs	CIv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
1.95	1,377	101.95					10.05						10.05
2.00	1,457	102.00					11.77						11.77

...End

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

lyd. lo.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	2.095	1	726	9,080				DA-FR-120F PRE	
2	SCS Runoff	2.095	1	726	9,080				DA-FR-120F DEV	
3	SCS Runoff	1.531	1	726	7,652				DA-FR-120F FOR	
3 4	SCS Runoff Reservoir	1.531		726 729	7,652 8,419	2	101.65	901	DA-FR-120F FOR WB Soil Amendments	
DA-FR-120F_Hydraflow.gpw					Return Period: 2 Year			Wednesday, 08 / 30 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 30 / 2017

= 484

Shape factor

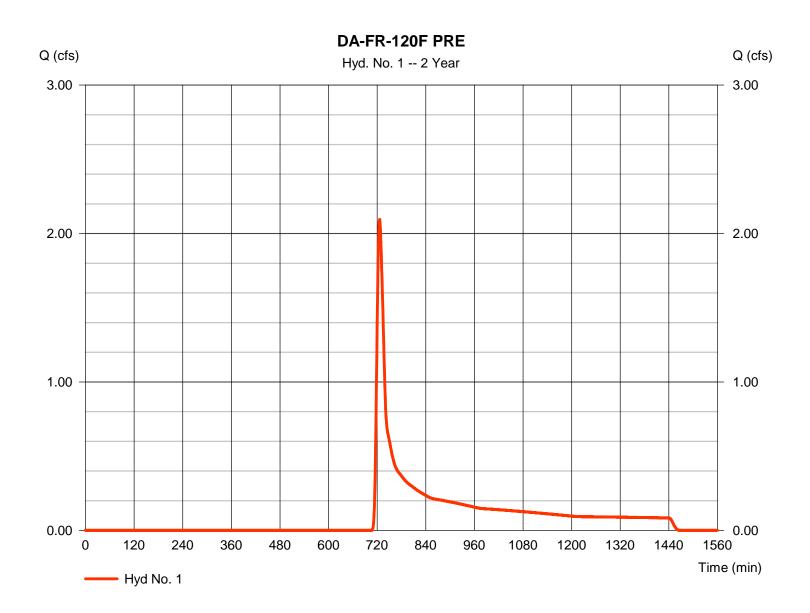
Hyd. No. 1

DA-FR-120F PRE

Storm duration

Hydrograph type = SCS Runoff Peak discharge = 2.095 cfsStorm frequency = 2 yrsTime to peak = 726 min Time interval = 1 minHyd. volume = 9.080 cuftDrainage area = 5.010 acCurve number = 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 15.90 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II

= 24 hrs



^{*} Composite (Area/CN) = $[(0.040 \times 65) + (0.001 \times 98) + (3.466 \times 58) + (0.014 \times 100) + (1.494 \times 55)] / 5.010$

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

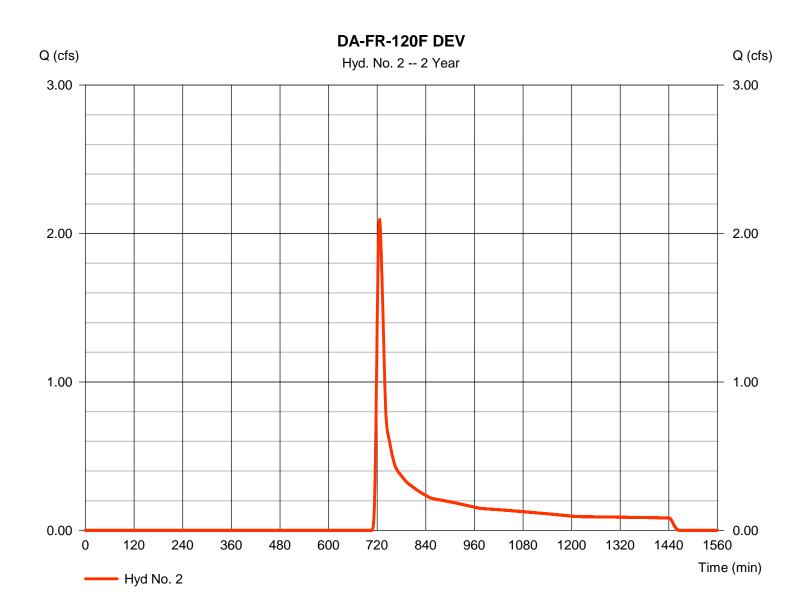
Wednesday, 08 / 30 / 2017

Hyd. No. 2

DA-FR-120F DEV

Hydrograph type = SCS Runoff Peak discharge = 2.095 cfsStorm frequency = 2 yrsTime to peak = 726 min Time interval = 1 minHyd. volume = 9.080 cuftDrainage area = 5.010 acCurve number = 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 15.90 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs = 484Shape factor

^{*} Composite (Area/CN) = $[(0.900 \times 48) + (0.040 \times 82) + (0.001 \times 98) + (4.045 \times 58) + (0.014 \times 100) + (0.014 \times 55)] / 5.010$



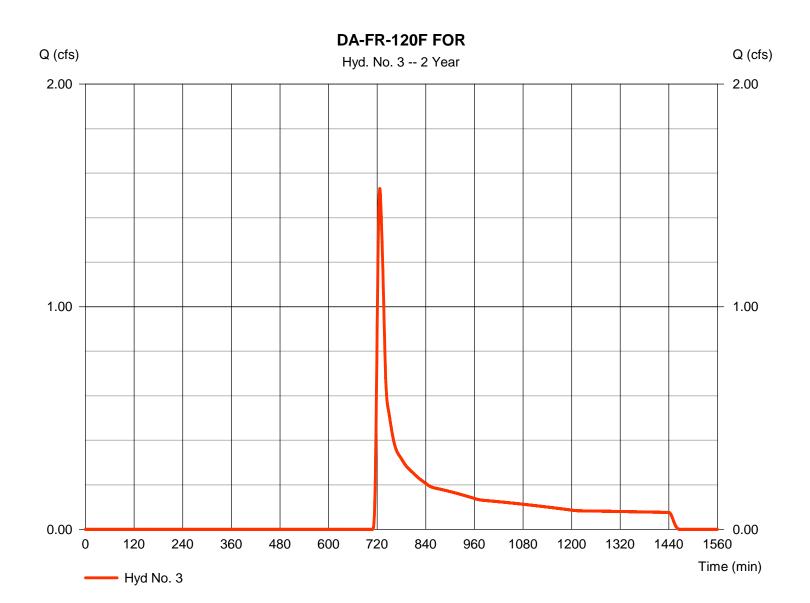
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 30 / 2017

Hyd. No. 3

DA-FR-120F FOR

Hydrograph type = SCS Runoff Peak discharge = 1.531 cfsStorm frequency = 2 yrsTime to peak = 726 min Time interval = 1 min Hyd. volume = 7.652 cuftDrainage area Curve number = 5.010 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 15.90 min = TR55 Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

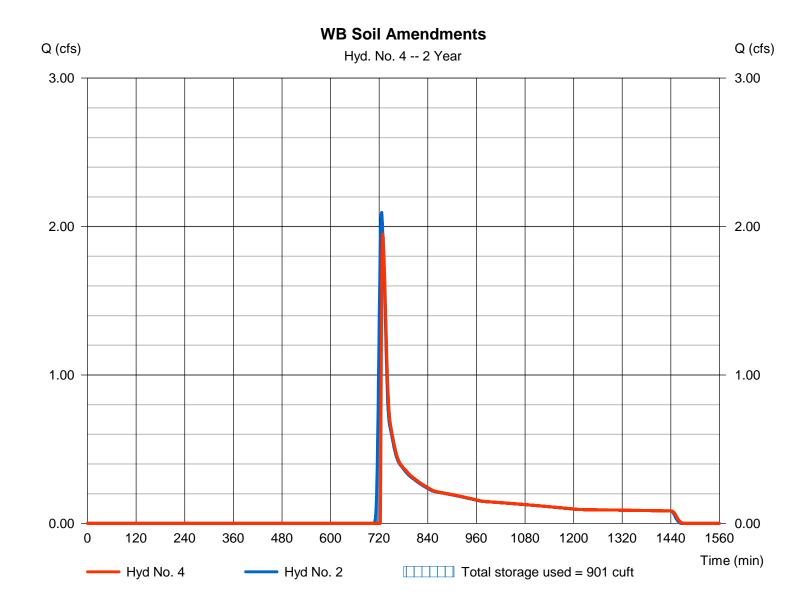
Wednesday, 08 / 30 / 2017

Hyd. No. 4

WB Soil Amendments

Hydrograph type = Reservoir Peak discharge = 1.950 cfsStorm frequency = 2 yrsTime to peak = 729 min Time interval = 1 minHyd. volume = 8,419 cuftInflow hyd. No. Max. Elevation = 2 - DA-FR-120F DEV $= 101.65 \, \text{ft}$ = Waterbar Soil Amendments Reservoir name Max. Storage = 901 cuft

Storage Indication method used.



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type	Peak flow	Time interval	Time to Peak	Hyd. volume	Inflow hyd(s)	Maximum elevation	Total strge used	Hydrograph Description	
	(origin)	(cfs)	(min)	(min)	(cuft)		(ft)	(cuft)	-	
1	SCS Runoff	8.865	1	724	27,564				DA-FR-120F PRE	
2	SCS Runoff	8.865	1	724	27,564				DA-FR-120F DEV	
3	SCS Runoff	7.742	1	724	24,831				DA-FR-120F FOR	
4	Reservoir	8.803	1	725	26,902	2	101.91	1,316	WB Soil Amendments	
DA	-FR-120F_H	/draflow.g	Jpw		Return F	turn Period: 10 Year Wednesday, 08 / 30 / 2017				

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

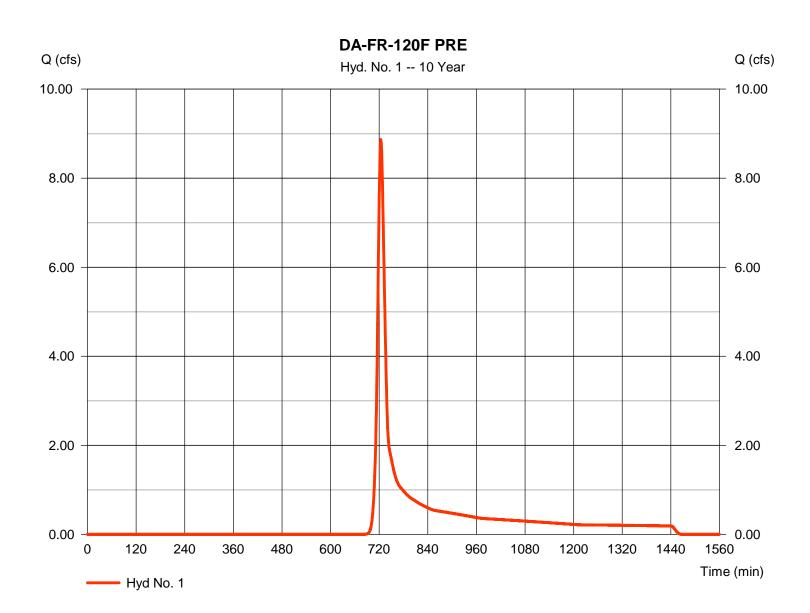
Wednesday, 08 / 30 / 2017

Hyd. No. 1

DA-FR-120F PRE

Hydrograph type = SCS Runoff Peak discharge = 8.865 cfsStorm frequency = 10 yrsTime to peak = 724 min Time interval = 1 minHyd. volume = 27,564 cuftCurve number Drainage area = 5.010 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.90 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.040 \times 65) + (0.001 \times 98) + (3.466 \times 58) + (0.014 \times 100) + (1.494 \times 55)] / 5.010$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

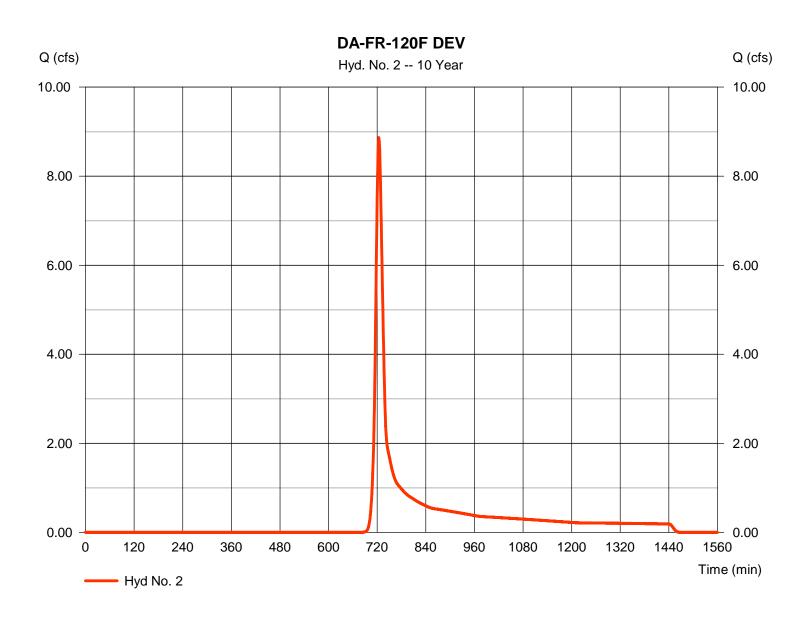
Wednesday, 08 / 30 / 2017

Hyd. No. 2

DA-FR-120F DEV

Hydrograph type = SCS Runoff Peak discharge = 8.865 cfsStorm frequency = 10 yrsTime to peak = 724 min Time interval = 1 minHyd. volume = 27,564 cuftCurve number Drainage area = 5.010 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.90 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inStorm duration = 24 hrs = 484Shape factor

^{*} Composite (Area/CN) = [(0.900 x 48) + (0.040 x 82) + (0.001 x 98) + (4.045 x 58) + (0.014 x 100) + (0.014 x 55)] / 5.010



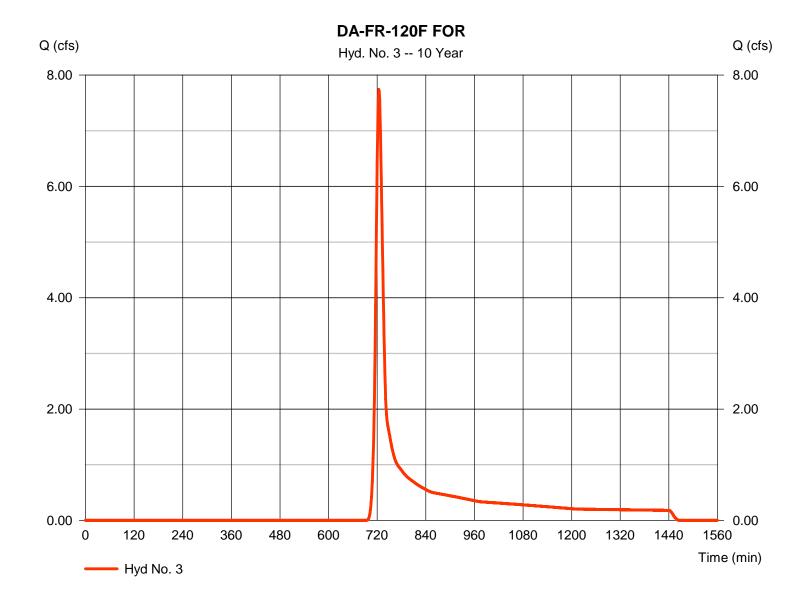
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 30 / 2017

Hyd. No. 3

DA-FR-120F FOR

Hydrograph type = SCS Runoff Peak discharge = 7.742 cfsStorm frequency = 10 yrsTime to peak = 724 min Time interval = 1 minHyd. volume = 24,831 cuftDrainage area Curve number = 5.010 ac= 55Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 15.90 min = TR55 Total precip. = 5.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

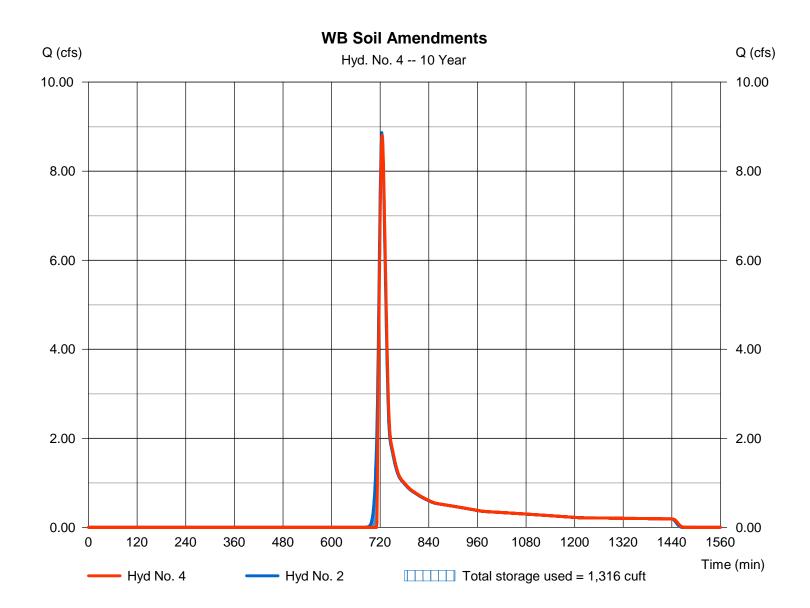
Wednesday, 08 / 30 / 2017

Hyd. No. 4

WB Soil Amendments

Hydrograph type Peak discharge = 8.803 cfs= Reservoir Storm frequency = 10 yrsTime to peak = 725 min Time interval = 1 minHyd. volume = 26,902 cuftInflow hyd. No. Max. Elevation = 2 - DA-FR-120F DEV $= 101.91 \, \text{ft}$ Reservoir name = Waterbar Soil Amendments Max. Storage = 1,316 cuft

Storage Indication method used.



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 30 / 2017

Return Period	Intensity-Du	Intensity-Duration-Frequency Equation Coefficients (FHA)									
(Yrs)	В	D	E	(N/A)							
1	0.0000	0.0000	0.0000								
2	69.8703	13.1000	0.8658								
3	0.0000	0.0000	0.0000								
5	79.2597	14.6000	0.8369								
10	88.2351	15.5000	0.8279								
25	102.6072	16.5000	0.8217								
50	114.8193	17.2000	0.8199								
100	127.1596	17.8000	0.8186								

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return					Intens	sity Values	(in/hr)					
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

ling\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.p

		Rainfall Precipitation Table (in)										
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr				
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00				
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				

DA-FR-121

DA-FR-121 is located in a meadow and forested areas with rolling slopes and contains agricultural land, existing dirt road and gravel road. No new impervious area is proposed within DA-FR-121. The total phosphorus load reduction required for DA-FR-121 is -0.28 lb/yr. Multiple points of analysis were evaluated within DA-FR-121 to evaluate the effects on each receiving stream/channel following construction. Specifically, DA-FR-121 was sub-divided into three sub-drainage areas (sub areas A through C).

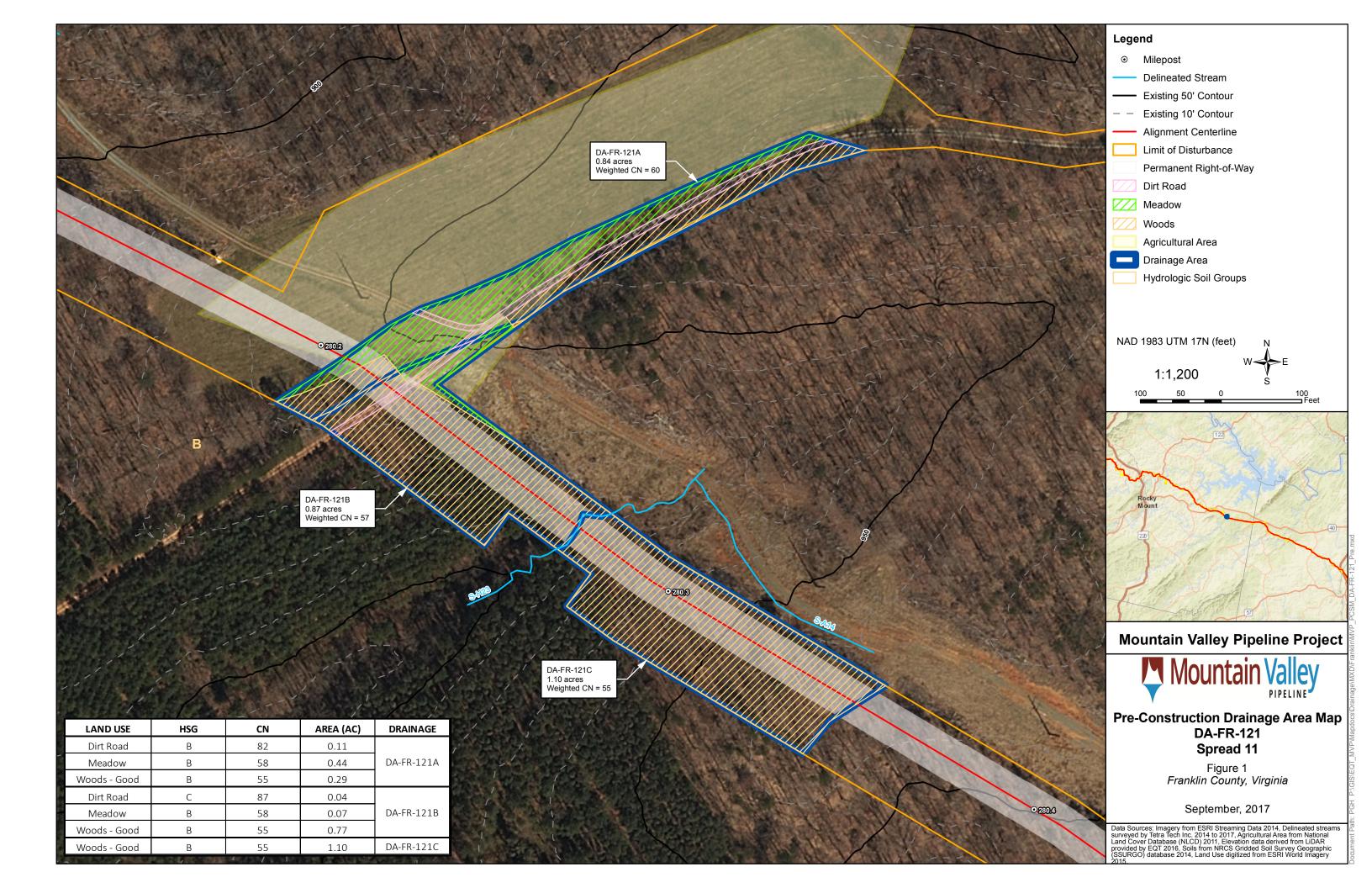
Sub-areas 121A and 121B contain both agricultural and non-agricultural areas within the limits of disturbance (LOD). Pre-construction agricultural areas will be returned to agricultural land use (i.e., returned to crop production, in identical condition) following construction. In non-agricultural areas, land use will be restored following construction as noted in the Stormwater Management (SWM) Narrative and the Annual Standards and Specifications. Agricultural areas within the LOD are included in the SWM quality analysis and the total permanent Right of Way (ROW) is analyzed via VRRM; in these calculations agricultural areas are considered "Forest/Open Space".

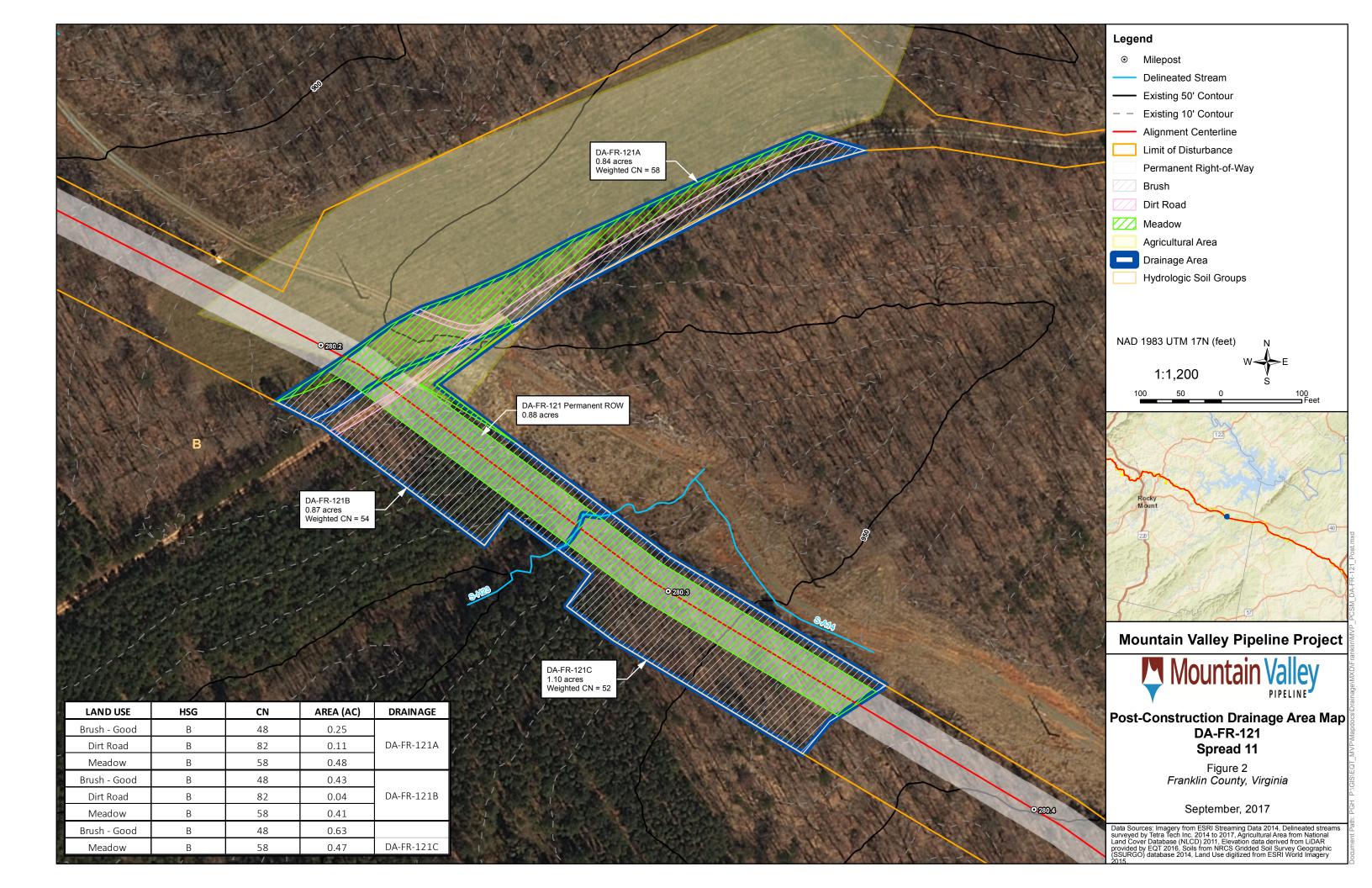
Stormwater quantity is met via the energy balance method for each of the three subareas DA-FR-121A, DA-FR-121B and DA-FR-121C. Agricultural areas within the study area are included in the SWM analysis, but an Improvement Factor (IF) of 1.0 is used when applying the Energy Balance Method. This improvement factor is used to account for the exemption of agricultural areas (§ 62.1-44.15:34 and 9VAC25-870-300) since such areas will be returned to agricultural land use (i.e., returned to crop production, in identical condition) following construction.

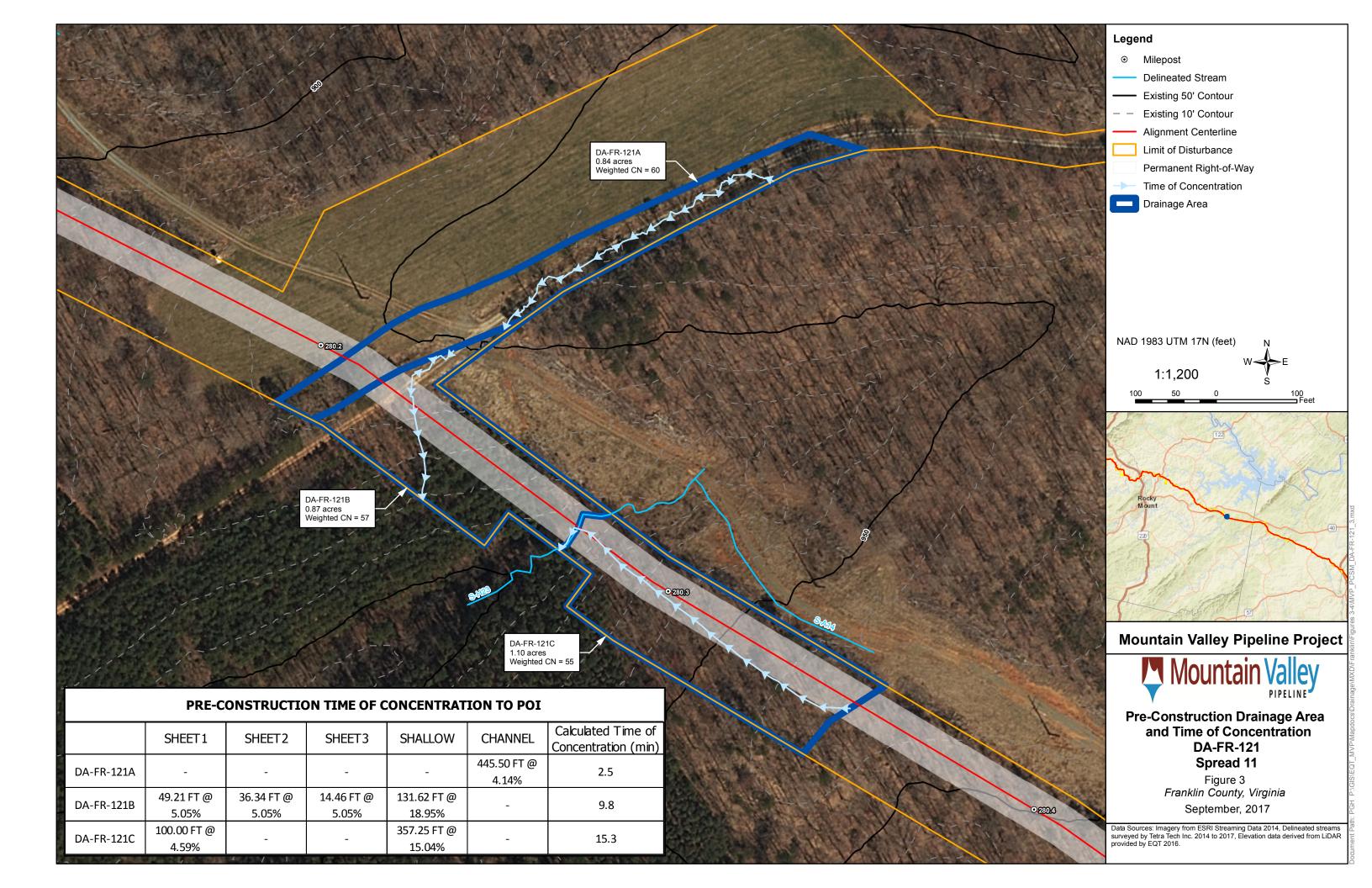
In addition, the Hydraflow Hydrograph's 10-year 24-hour peak discharge results indicate a reduction in flows ranging from 0.36 to 0.81 cfs for all drainage areas (as seen in table below).

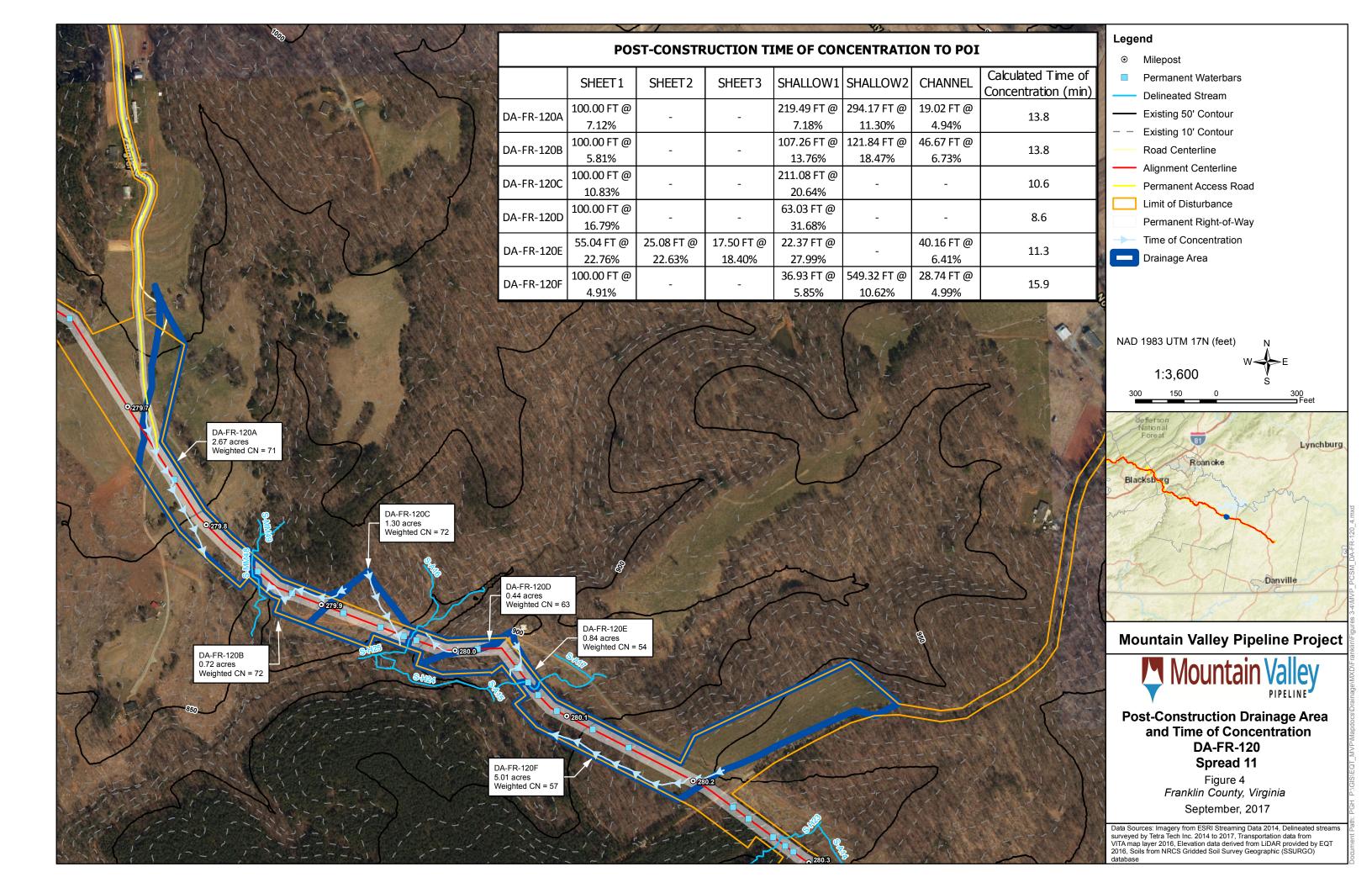
Sub Area	Pre Peak Flow,	Post Peak Flow, Q	Flow differential
	10-yr Q (cfs)	10-yr (cfs)	
DA-FR-121A	2.56	2.16	-0.40
DA-FR-121B	2.00	1.64	-0.36
DA-FR-121C	1.70	0.89	-0.81

Figures and calculations for each of the sub-areas for DA-FR-121 follow. See Appendix D of the Annual Standards and Specifications for further detail on stormwater methodology.









DEQ Virginia Runoff Reduction Method Re-Development Compliance Spreadsheet - Version 3.0

BMP Design Specifications List: 2013 Draft Stds & Specs

Site Summary - Linear Development Project***

Total Rainfall (in):	43
Total Disturbed Acreage:	0.88

Site Land Cover Summary

Pre-ReDevelopment Land Cover (acres)

$\circ \circ $	A soils	B Soils	C Soils	D Soils	Totals	% of Total
Forest/Open (acres)	0.00	0.87	0.00	0.00	0.87	99
Managed Turf (acres)	0.00	0.00	0.00	0.00	0.00	0
Impervious Cover (acres)	0.00	0.01	0.00	0.00	0.01	1
					0.88	100

Post-ReDevelopment Land Cover (acres)

0000000000000000000000000000000000000	A soils	B Soils	C Soils	D Soils	Totals	% of Total
Forest/Open (acres)	0.00	0.87	0.00	0.00	0.87	99
Managed Turf (acres)	0.00	0.00	0.00	0.00	0.00	0
Impervious Cover (acres)	0.00	0.01	0.00	0.00	0.01	1
Forest/Open Space areas must be protected in accordance with the Virginia Runoff Reduction Method					0.88	100

Site Tv and Land Cover Nutrient Loads

	Final Post-Development (Post-ReDevelopment & New Impervious)	Post- ReDevelopment	Post- Development (New Impervious)	Adjusted Pre- ReDevelopment
Site Rv	0.04	0.04		0.04
Treatment Volume (ft ³)	129	129		129
TP Load (lb/yr)	0.08	0.08		0.08
	Deseller TD Lead (III /).	0.2000*	*Paduction balow now	dayalanmant laad limita

ReDevelopment TP Load per acre (lb/acre/yr)	TP Load per acre (lb/acre/yr)	Load per acre (lb/acre/yr)
0.09	0.09	0.09

Baseline TP Load (lb/yr):

0.3608*

*Reduction below new development load limitation not required

Total TP Load Reduction Required (lb/yr)	-0.28	N/A***	N/A***
--	-------	--------	--------

***This is a linear development project

	Final Post-Development Load (Post-ReDevelopment & New Impervious)	Pre- ReDevelopment
TN Load (lb/yr)	0.58	0.58

Site Compliance Summary - ***Linear Development Project

Maximum % Reduction Required Below	20%	* Note: % Reduction will reduce post-development TP load to less than or equal to baseline load of 0.36 lb/yr (0.41 lb/ac/yr)
Pre-ReDevelopment Load	20%	[Post-Dev Reduction Requirement = Post-Dev TP load - baseline load of 0.36 lb/yr], baseline load = site area x 0.41 lb/ac/yr

Total Runoff Volume Reduction (ft ³)

_	
Total TP Load Reduction Achieved (lb/yr)	0.00
Total TN Load Reduction Achieved (lb/yr)	0.00
Remaining Post Development TP Load (lb/yr)	0.08
Remaining TP Load Reduction (lb/yr) Required	0.00

** TARGET TP REDUCTION EXCEEDED BY 0.28 LB/YEAR **

.....

^{*}Reduction below new development load limitation not required



ENERGY BALANCE METHOD

	n		

	1-Y	r Event
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)
Pre-Developed Condition	0.605	1281
Developed Condition	0.454	1078
Pre-Developed (Forest) Condition	0.263	804

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0

<u>Calculations:</u>	Check #1:	$Q_{\text{developed}} \leq \text{IF x } \left[\left\{ Q_{\text{pre-developed}} \times \text{RV}_{\text{pre-developed}} \right\} / \left. \text{RV}_{\text{developed}} \right] $	0.454	≤ OK	0.575
	Check #2:	Q _{developed} ≤ Q _{pre-developed} →	0.454	≤ OK	0.605
	Check #3:	$Q_{developed} \frac{shall\ not}{shall\ not}$ be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed} \longrightarrow$	0.454	<u>shall not</u> be required to be ≤	0.196

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 2 - Manning's n Values for Open Channel Flow

nanı	nnel Type		Manning n					
			Min.	Normal	Max.			
1.	Exc	cavated or Dredged Channels ¹						
	a.	Earth, Straight, and Uniform:						
		Clean, recently completed	0.016	0.018	0.020			
		Clean, after weathering	0.018	0.022	0.025			
		Gravel, uniform section, clean	0.022	0.025	0.030			
		With short grass, few weeds	0.022	0.027	0.033			
	b.	Earth Winding and Sluggish:						
		No vegetation	0.023	0.025	0.030			
		Grass, some weeds	0.025	0.030	0.033			
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040			
		Earth bottom and rubble sides	0.028	0.030	0.035			
		Stony bottom and weedy banks	0.025	0.035	0.040			
		Cobble bottom and clean sides	0.030	0.040	0.050			
	c.	Dragline-Excavated or Dredged:						
		No vegetation	0.025	0.028	0.033			
		Light brush on banks	0.035	0.050	0.060			
	d.	Rock Cuts:		•				
		Smooth and uniform	0.025	0.035	0.040			
		Jagged and irregular	0.035	0.040	0.050			
	e.	Channels not Maintained, Weeds and Brush Uncut:		•				
		Dense weeds, high as flow depth	0.050	0.080	0.120			
		Clean bottom, brush on sides	0.040	0.050	0.080			
		Same as above, highest stage of flow	0.045	0.070	0.110			
		Dense brush, high stage	0.080	0.100	0.140			
2.	Mai	n Channels²		•				
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	<mark>0.030</mark>	0.033			
	b.	Same as above, but more stones and weeds	0.030	0.035	0.040			
	c.	Clean, winding, some pools and shoals	0.033	0.040	0.045			
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050			
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.055			
	f.	Same as (d) with more stones	0.045	0.050	0.060			
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080			
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150			

Notes

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs.

Sources:

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 30 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	. 3
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-121A DEV	4
TR-55 Tc Worksheet	. 5
Hydrograph No. 2, SCS Runoff, DA-FR-121A FOR	. 6
TR-55 Tc Worksheet	
Hydrograph No. 9, SCS Runoff, DA-FR-121A PRE	
TR-55 Tc Worksheet	. 9
2 - Year	
Summary Report	10
Hydrograph Reports	11
Hydrograph No. 1, SCS Runoff, DA-FR-121A DEV	
Hydrograph No. 2, SCS Runoff, DA-FR-121A FOR	
Hydrograph No. 9, SCS Runoff, DA-FR-121A PRE	13
10 - Year	
Summary Report	14
Hydrograph Reports	15
Hydrograph No. 1, SCS Runoff, DA-FR-121A DEV	15
Hydrograph No. 2, SCS Runoff, DA-FR-121A FOR	
Hydrograph No. 9, SCS Runoff, DA-FR-121A PRE	17
IDF Report	18

Wednesday, 08 / 30 / 2017

Project: DA-FR-121A.gpw

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5 <u>Legend</u> Hyd. Origin **Description** SCS Runoff DA-FR-121A DEV 2 SCS Runoff DA-FR-121A FOR SCS Runoff DA-FR-121A PRE

Hydrograph Return Period Recap Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	Hydrograph	Inflow				Hydrograph					
lo.	type (origin)	hyd(s)	1-yr	2-yr	yr 3-yr 5-yr 10-yr 25-yr 50-yr 100		100-yr	Description			
1	SCS Runoff		0.454	0.743			2.559	 		DA-FR-121A DEV	
2	SCS Runoff		0.263	0.494			2.157	 		DA-FR-121A FOR	
9	SCS Runoff		0.605	0.916			2.830	 		DA-FR-121A PRE	

Proj. file: DA-FR-121A.gpw

Wednesday, 08 / 30 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

lyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.454	1	716	1,078				DA-FR-121A DEV
2	SCS Runoff	0.263	1	718	804				DA-FR-121A FOR
9	SCS Runoff SCS Runoff	0.263	1	718	1,281				DA-FR-121A PRE
DA	-FR-121A.gpv	v			Return F	Period: 1 Ye	ear	Wednesday	y, 08 / 30 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

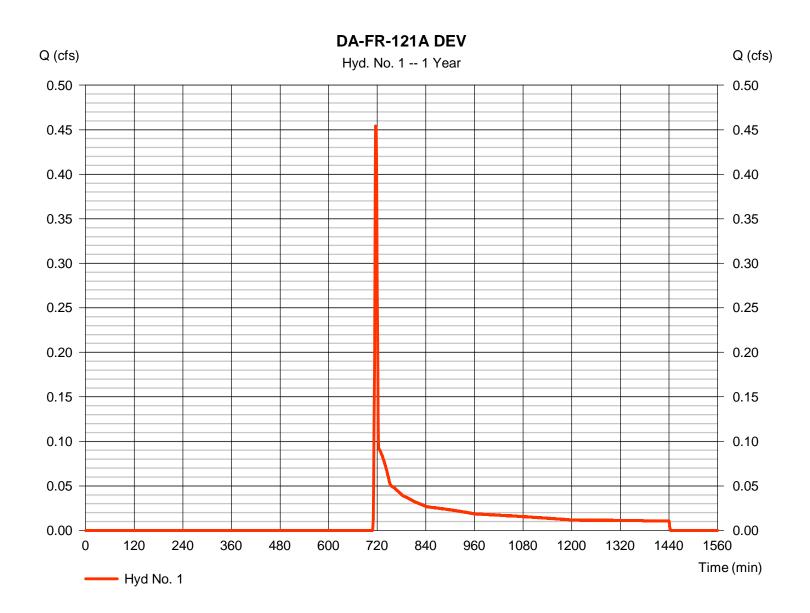
Wednesday, 08 / 30 / 2017

Hyd. No. 1

DA-FR-121A DEV

Hydrograph type = SCS Runoff Peak discharge = 0.454 cfsStorm frequency Time to peak = 716 min = 1 yrsTime interval = 1 min Hyd. volume = 1,078 cuftDrainage area Curve number = 0.840 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 2.50 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.250 \times 48) + (0.114 \times 82) + (0.476 \times 58)] / 0.840$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1

DA-FR-121A DEV

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.011 = 0.0 = 0.00 = 0.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 0.00 = 0.00 = Paved =0.00		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.52 = 15.63 = 4.14 = 0.030 =2.98		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})445.5		0.0		0.0		
Travel Time (min)	= 2.50	+	0.00	+	0.00	=	2.50
Total Travel Time, Tc							2.50 min

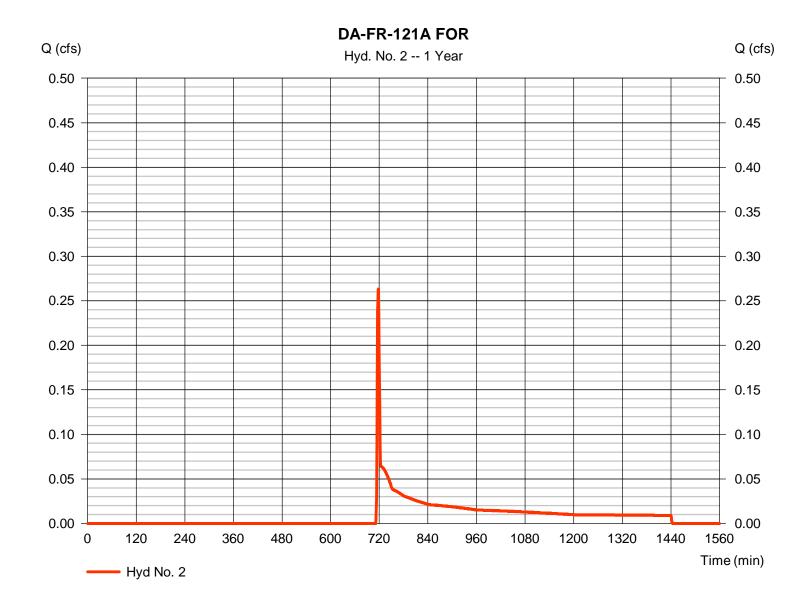
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 30 / 2017

Hyd. No. 2

DA-FR-121A FOR

Hydrograph type = SCS Runoff Peak discharge = 0.263 cfsStorm frequency Time to peak = 718 min = 1 yrsTime interval = 1 min Hyd. volume = 804 cuft Drainage area Curve number = 0.840 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 2.50 \, \text{min}$ Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2

DA-FR-121A FOR

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.011 = 0.0 = 0.00 = 0.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 0.00 = 0.00 = Paved =0.00		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.52 = 15.63 = 4.14 = 0.030 =2.98		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})445.5		0.0		0.0		
Travel Time (min)	= 2.50	+	0.00	+	0.00	=	2.50
Total Travel Time, Tc							2.50 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

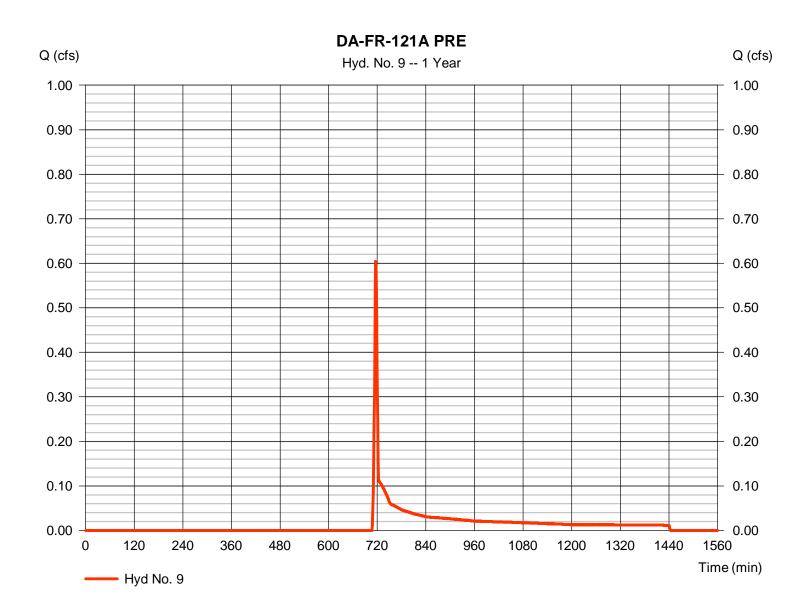
Wednesday, 08 / 30 / 2017

Hyd. No. 9

DA-FR-121A PRE

Hydrograph type = SCS Runoff Peak discharge = 0.605 cfsStorm frequency Time to peak = 716 min = 1 yrsTime interval = 1 min Hyd. volume = 1.281 cuftDrainage area Curve number = 0.840 ac= 60*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 2.50 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.114 \times 82) + (0.441 \times 58) + (0.286 \times 55)] / 0.840$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 9

DA-FR-121A PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.011 = 0.0 = 0.00 = 0.00		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 0.00 = 0.00 = Paved =0.00		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.52 = 15.63 = 4.14 = 0.030 =2.98		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})445.5		0.0		0.0		
Travel Time (min)	= 2.50	+	0.00	+	0.00	=	2.50
Total Travel Time, Tc							2.50 min

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

		- Inyulai				Trydianow Trydrographs Extension for Addocade Givil 306 2010 by Addocest, Inc. vi						
lyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description			
1	SCS Runoff	0.743	1	716	1,527				DA-FR-121A DEV			
2	SCS Runoff	0.494	1	716	1,188				DA-FR-121A FOR			
9	SCS Runoff	0.916	1	716	1,772				DA-FR-121A PRE			
DA-FR-121A.gpw					Return F	Period: 2 Ye	ear	Wednesda	y, 08 / 30 / 2017			

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

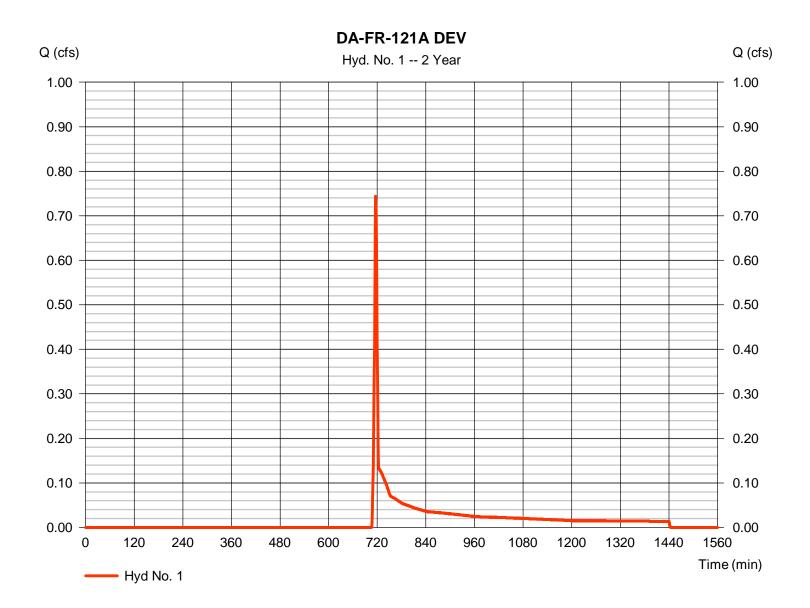
Wednesday, 08 / 30 / 2017

Hyd. No. 1

DA-FR-121A DEV

Hydrograph type = SCS Runoff Peak discharge = 0.743 cfsStorm frequency Time to peak = 716 min = 2 yrsTime interval = 1 min Hyd. volume = 1,527 cuftDrainage area Curve number = 0.840 ac= 58* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 2.50 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.250 \times 48) + (0.114 \times 82) + (0.476 \times 58)] / 0.840$



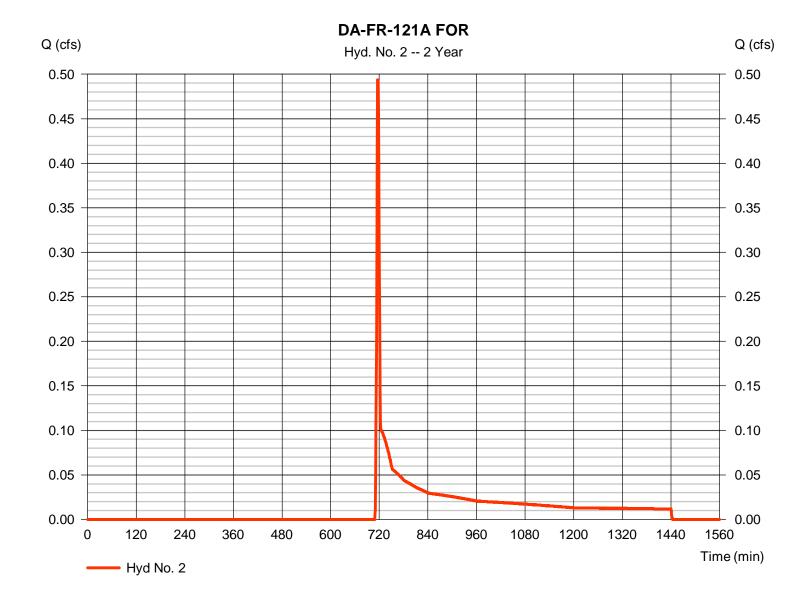
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 30 / 2017

Hyd. No. 2

DA-FR-121A FOR

= SCS Runoff Hydrograph type Peak discharge = 0.494 cfsStorm frequency Time to peak = 716 min = 2 yrsTime interval = 1 min Hyd. volume = 1,188 cuftDrainage area Curve number = 0.840 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 2.50 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

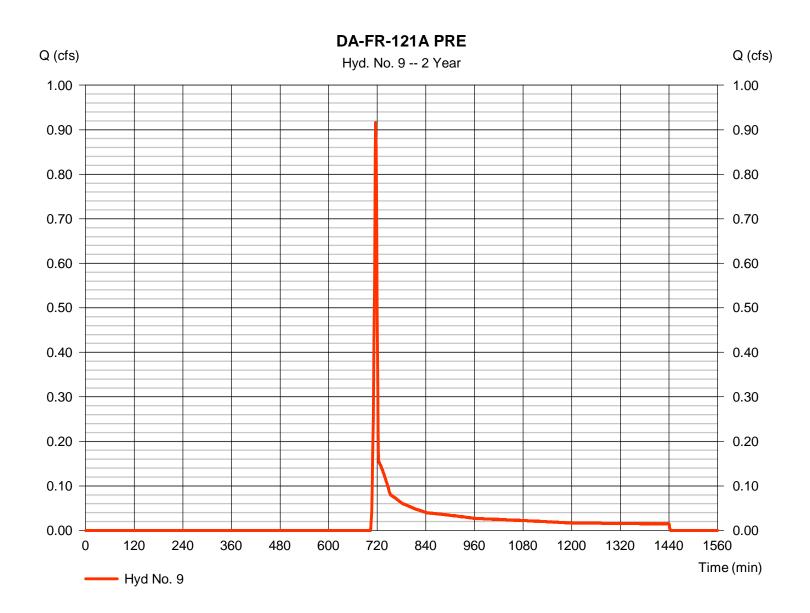
Wednesday, 08 / 30 / 2017

Hyd. No. 9

DA-FR-121A PRE

Hydrograph type = SCS Runoff Peak discharge = 0.916 cfsStorm frequency Time to peak = 716 min = 2 yrsTime interval = 1 min Hyd. volume = 1,772 cuftDrainage area Curve number = 0.840 ac= 60*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 2.50 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.114 \times 82) + (0.441 \times 58) + (0.286 \times 55)] / 0.840$



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

				- Tryaranow Tr	yarograpno Extor	KIETSIOTTO AUTOCADO CIVIL 3DO 2010 DY AUTOCESA, ITIC. V			
lyd. lo.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	2.559	1	716	4,496				DA-FR-121A DEV
2	SCS Runoff	2.157	1	716	3,855				DA-FR-121A FOR
9	SCS Runoff	2.830	1	716	4,940				DA-FR-121A PRE
DΑ	-FR-121A.gpv	W			Return F	Period: 10 \	⁄ear	Wednesda	y, 08 / 30 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

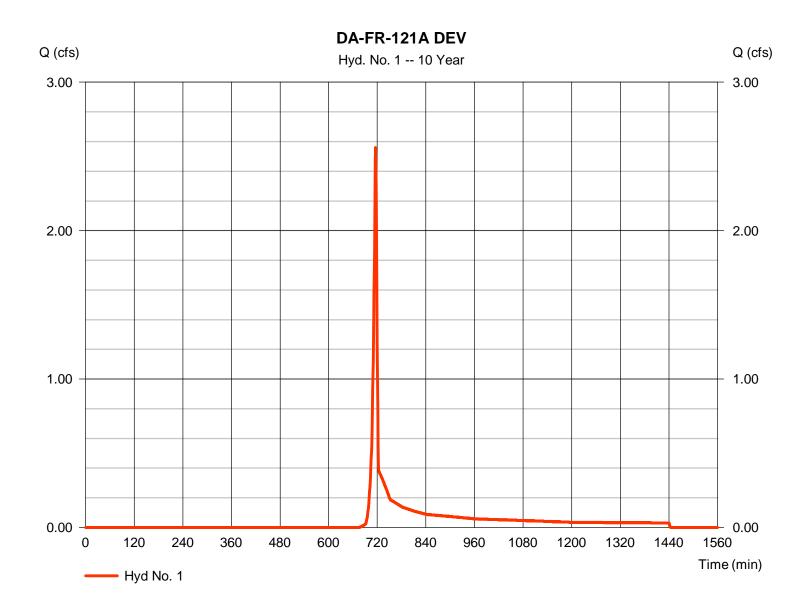
Wednesday, 08 / 30 / 2017

Hyd. No. 1

DA-FR-121A DEV

= SCS Runoff Hydrograph type Peak discharge = 2.559 cfsStorm frequency Time to peak = 716 min = 10 yrsTime interval = 1 min Hyd. volume = 4,496 cuftCurve number Drainage area = 0.840 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 2.50 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.250 \times 48) + (0.114 \times 82) + (0.476 \times 58)] / 0.840$



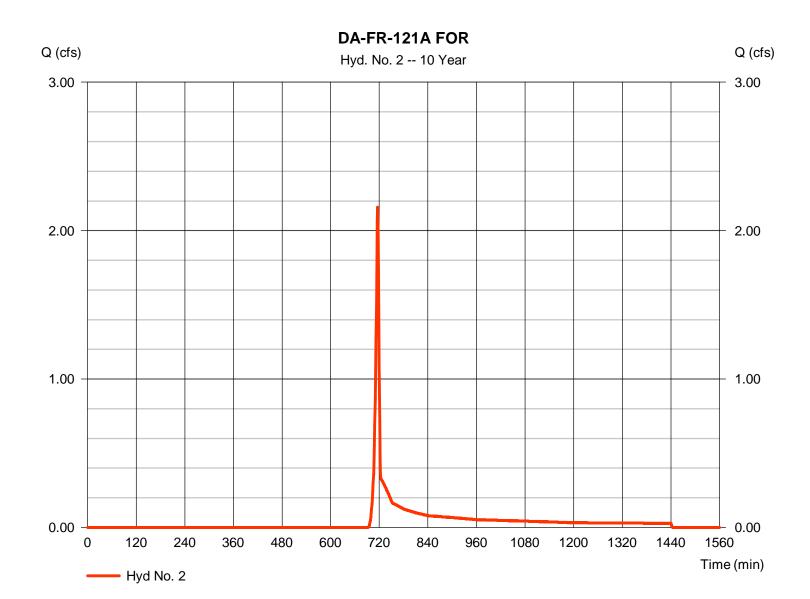
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 30 / 2017

Hyd. No. 2

DA-FR-121A FOR

= SCS Runoff Hydrograph type Peak discharge = 2.157 cfsStorm frequency Time to peak = 716 min = 10 yrsTime interval = 1 min Hyd. volume = 3.855 cuftDrainage area Curve number = 0.840 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 2.50 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

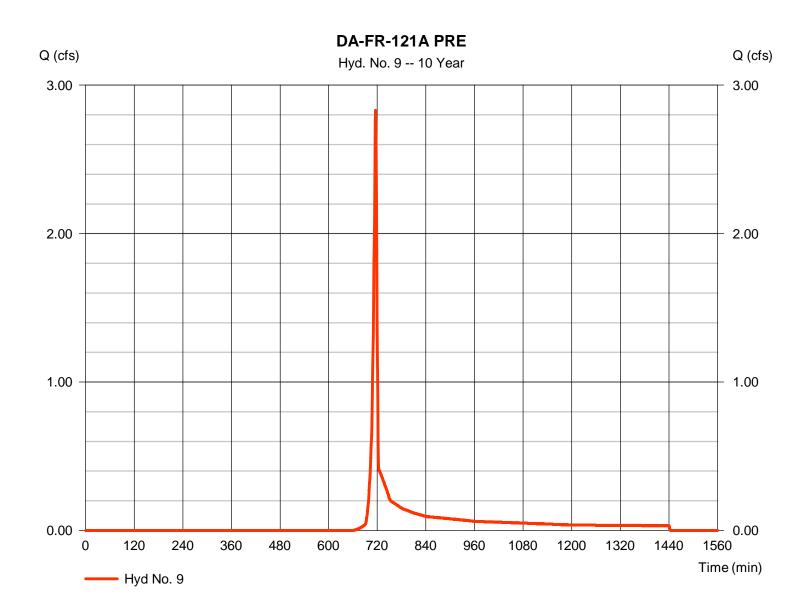
Wednesday, 08 / 30 / 2017

Hyd. No. 9

DA-FR-121A PRE

= SCS Runoff Hydrograph type Peak discharge = 2.830 cfsStorm frequency Time to peak = 716 min = 10 yrsTime interval = 1 min Hyd. volume = 4,940 cuftDrainage area Curve number = 0.840 ac= 60*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 2.50 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.114 \times 82) + (0.441 \times 58) + (0.286 \times 55)] / 0.840$



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Wednesday, 08 / 30 / 2017

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)									
(Yrs)	В	D	E	(N/A)						
1	0.0000	0.0000	0.0000							
2	69.8703	13.1000	0.8658							
3	0.0000	0.0000	0.0000							
5	79.2597	14.6000	0.8369							
10	88.2351	15.5000	0.8279							
25	102.6072	16.5000	0.8217							
50	114.8193	17.2000	0.8199							
100	127.1596	17.8000	0.8186							

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return Period (Yrs)		Intensity Values (in/hr)													
	5 min	10	15	20	25	30	35	40	45	50	55	60			
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70			
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15			
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46			
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91			
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25			
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60			

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc|

		Rainfall Precipitation Table (in)												
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr						
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00						
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00						
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00						
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00						
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00						
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00						
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00						
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00						



ENERGY BALANCE METHOD

	1-Yr Event					
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)				
Pre-Developed Condition	0.286	1085				
Developed Condition	0.144	796				
Pre-Developed (Forest) Condition	0.143	888				

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.8

<u>Calculations:</u>	Check #1:	$Q_{\text{developed}} \leq \text{IF x } \left[\left\{ Q_{\text{pre-developed}} \times \text{RV}_{\text{pre-developed}} \right\} / \text{RV}_{\text{developed}} \right] $	0.144	≤ OK	0.312
	Check #2:	Q _{developed} ≤ Q _{pre-developed} >	0.144	≤ OK	0.286
	Check #3:	$Q_{\text{developed } shall \ not}$ be required to be $\leq (Q_{\text{forest}} \times RV_{\text{forest}}) / RV_{\text{developed}} \longrightarrow$	0.144	shall not be required to be ≤	0.160

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

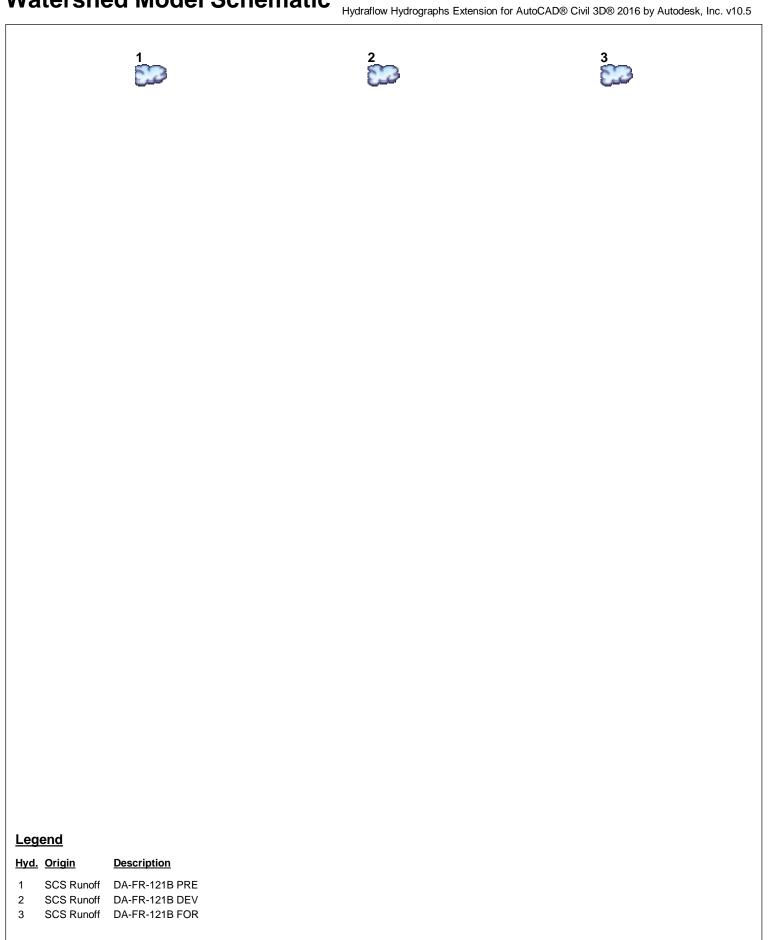
-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	4
Hydrograph No. 1, SCS Runoff, DA-FR-121B PRE	4
TR-55 Tc Worksheet	5
Hydrograph No. 2, SCS Runoff, DA-FR-121B DEV	6
TR-55 Tc Worksheet	
Hydrograph No. 3, SCS Runoff, DA-FR-121B FOR	
TR-55 Tc Worksheet	9
2 - Year	
Summary Report	10
Hydrograph Reports	11
Hydrograph No. 1, SCS Runoff, DA-FR-121B PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-121B DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-121B FOR	13
10 - Year	
Summary Report	14
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-121B PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-121B DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-121B FOR	17
IDE Papart	10



Project: P:\EQT Corporation\Mountain West Pipeline\Engineering\Stormwater Modeling\Framildind@quoby\@alc@ation Packages\Batcl

Hydrograph Return Period Recap

łyd.	Hydrograph	Inflow				Hydrograph					
lo.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		0.286	0.501			1.995				DA-FR-121B PRE
2	SCS Runoff		0.144	0.310			1.639				DA-FR-121B DEV
3	SCS Runoff		0.143	0.285			1.425				DA-FR-121B FOR

Proj. file: P:\EQT Corporation\Mountain West Pipeline\Engineering\Stormwater Modelidag\F0ank2in C201nTy\Calculation Packages\Batc

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

		_		<u>, </u>		Tryuranow r	Tydrographs Exte	TISIOTI TOI AUTOCA	D® Civil 3D® 2016 by Autodesk, Inc. v10.5
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.286	1	722	1,085				DA-FR-121B PRE
2	SCS Runoff	0.144	1	723	796				DA-FR-121B DEV
3	SCS Runoff	0.144	1	723	796 888				DA-FR-121B FOR

P:\EQT Corporation\Mountain West Pipeline Engineering Storm / Storm Meeter Modeling Filter Modeling Coent / Calculation Packages\Batch 9\Hydra

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

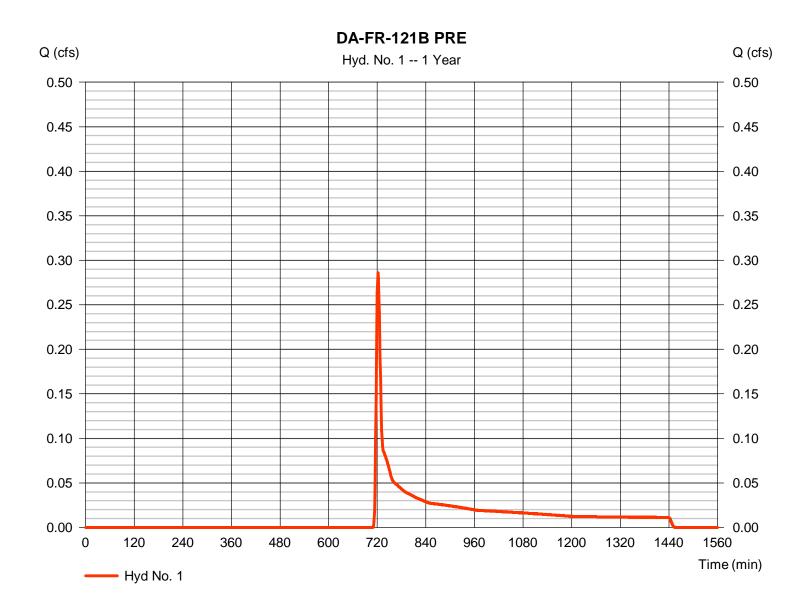
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-121B PRE

Hydrograph type = SCS Runoff Peak discharge = 0.286 cfsStorm frequency Time to peak = 722 min = 1 yrsTime interval = 1 min Hyd. volume = 1,085 cuftDrainage area Curve number = 0.870 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 9.80 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.038 \times 87) + (0.071 \times 58) + (0.766 \times 55)] / 0.870$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1

DA-FR-121B PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>			
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.011 = 49.2 = 3.70 = 5.05		0.400 36.3 3.70 5.05		0.400 14.5 3.70 5.05		0.54			
Travel Time (min)	= 0.44	+	6.13	+	2.94	=	9.51			
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 131.62 = 18.95 = Unpaved =7.02	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00					
Travel Time (min)	= 0.31	+	0.00	+	0.00	=	0.31			
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015					
Flow length (ft)	({0})0.0		0.0		0.0					
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00			
Total Travel Time, Tc										

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

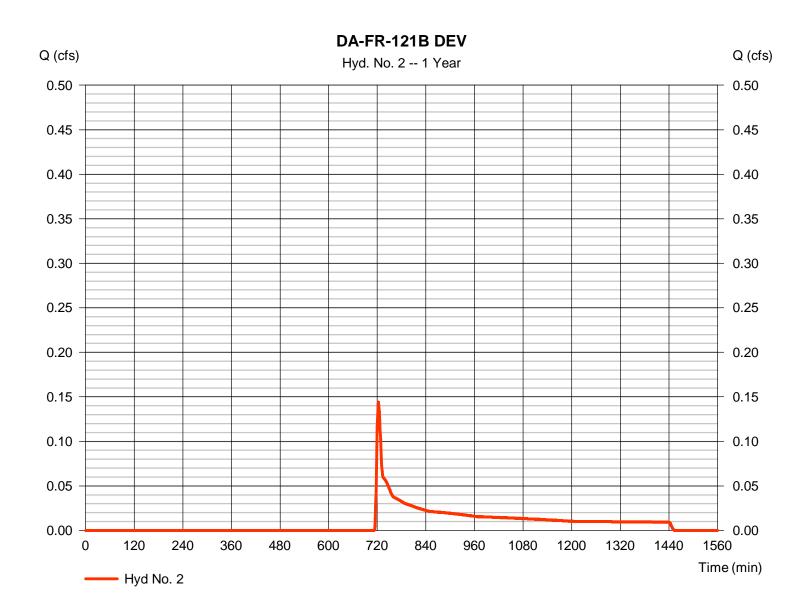
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-121B DEV

Hydrograph type = SCS Runoff Peak discharge = 0.144 cfsStorm frequency Time to peak = 723 min = 1 yrsTime interval = 1 min Hyd. volume = 796 cuft Drainage area Curve number = 54* = 0.870 acBasin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 9.80 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.428 \times 48) + (0.038 \times 82) + (0.408 \times 58)] / 0.870$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2

DA-FR-121B DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.011 = 49.2 = 3.70 = 5.05		0.400 36.3 3.70 5.05		0.400 14.5 3.70 5.05		
Travel Time (min)	= 0.44	+	6.13	+	2.94	=	9.51
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 131.62 = 18.95 = Unpave =7.02	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.31	+	0.00	+	0.00	=	0.31
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							9.80 min

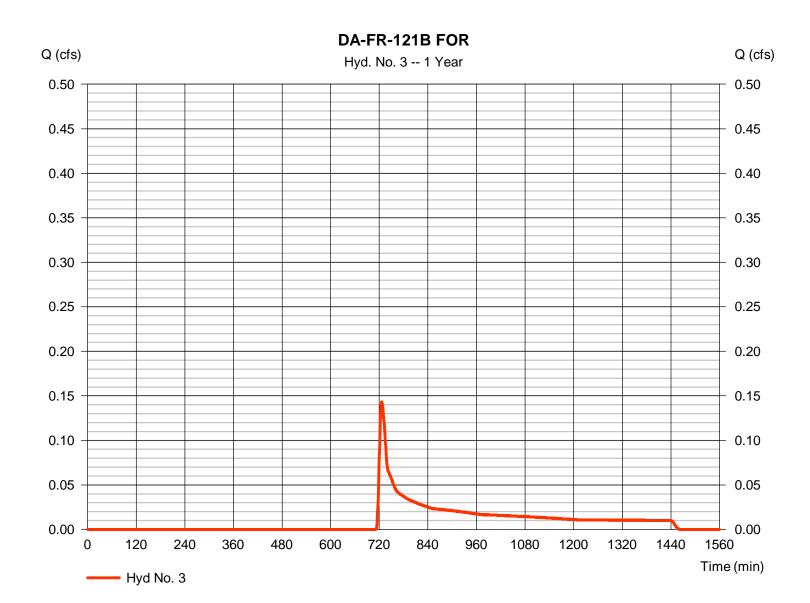
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-121B FOR

Hydrograph type = SCS Runoff Peak discharge = 0.143 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 min Hyd. volume = 888 cuft Drainage area Curve number = 0.870 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 14.10 min Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3

DA-FR-121B FOR

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 5.05 = 13.79	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	13.79
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 131.62 = 18.95 = Unpaved =7.02	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.31	+	0.00	+	0.00	=	0.31
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.501	1	721	1,557				DA-FR-121B PRE
2	SCS Runoff	0.310	1	722	1,197				DA-FR-121B DEV
2 3	SCS Runoff SCS Runoff	0.310	1 1	722	1,197				DA-FR-121B FOR

P:\EQT Corporation\Mountain West Pipeline Engineering 6to 2n1/exter Modeling Flictor Modeling Coent 2 Calculation Packages\Batch 9\Hydra

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

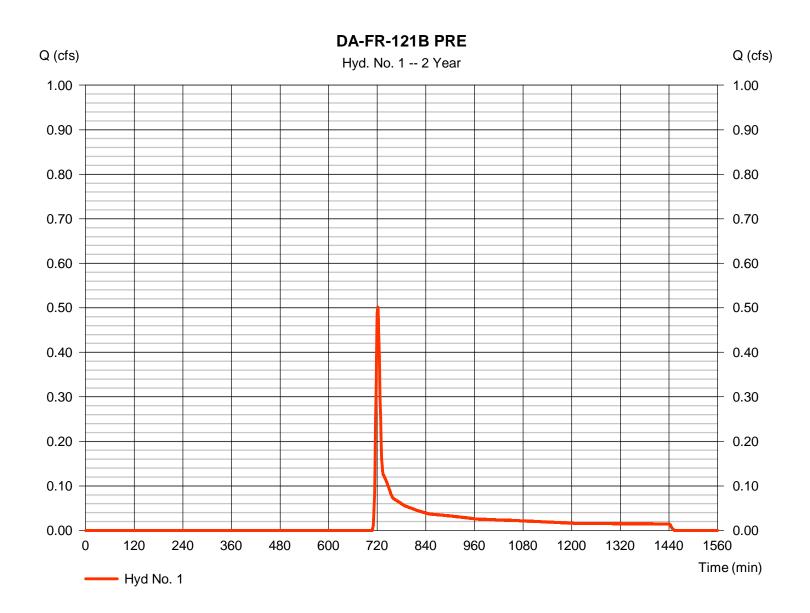
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-121B PRE

Hydrograph type = SCS Runoff Peak discharge = 0.501 cfsStorm frequency Time to peak = 721 min = 2 yrsTime interval = 1 min Hyd. volume = 1,557 cuftDrainage area Curve number = 0.870 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 9.80 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.038 \times 87) + (0.071 \times 58) + (0.766 \times 55)] / 0.870$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

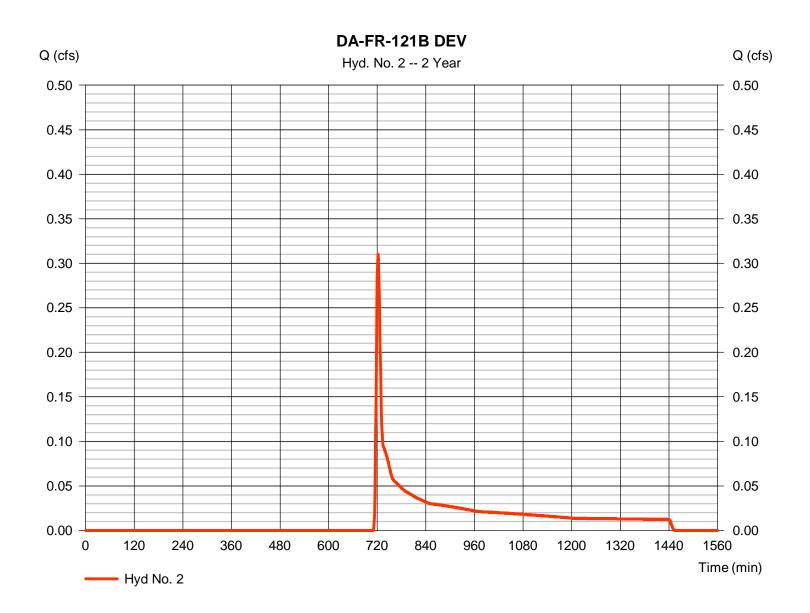
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-121B DEV

Hydrograph type = SCS Runoff Peak discharge = 0.310 cfsStorm frequency Time to peak = 722 min = 2 yrsTime interval = 1 min Hyd. volume = 1,197 cuftDrainage area Curve number = 0.870 ac= 54* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 9.80 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.428 \times 48) + (0.038 \times 82) + (0.408 \times 58)] / 0.870$



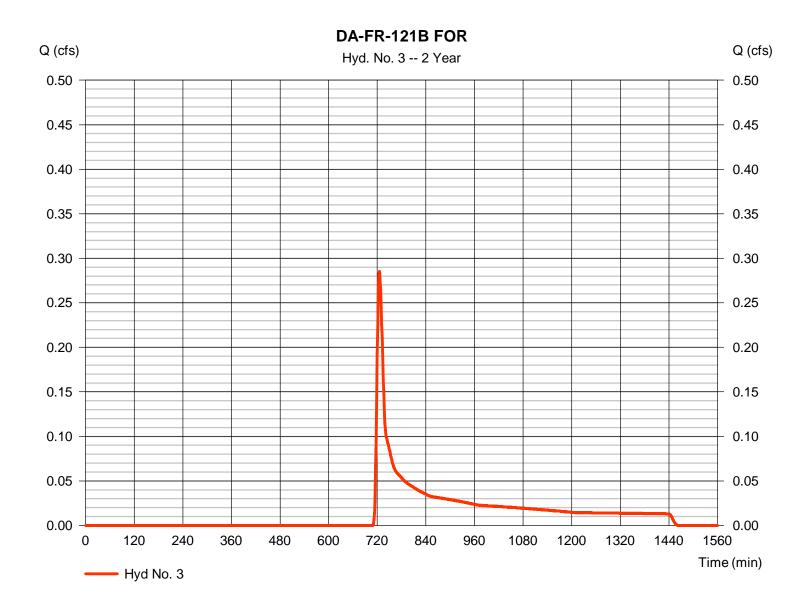
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-121B FOR

= SCS Runoff Hydrograph type Peak discharge = 0.285 cfsStorm frequency Time to peak = 725 min = 2 yrsTime interval = 1 min Hyd. volume = 1,312 cuftDrainage area Curve number = 0.870 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 14.10 min Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

łyd. ło.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	1.995	1	720	4,727				DA-FR-121B PRE
2	SCS Runoff	1.639	1	720	4,030				DA-FR-121B DEV
2 3	SCS Runoff SCS Runoff	1.639	1 1	720 723	4,030 4,259				DA-FR-121B DEV DA-FR-121B FOR

P:\EQT Corporation\Mountain West Pipeline Engineering Stort Owater Modeling Flator Modeling On the Carolina Packages Batch 9\Hydra

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

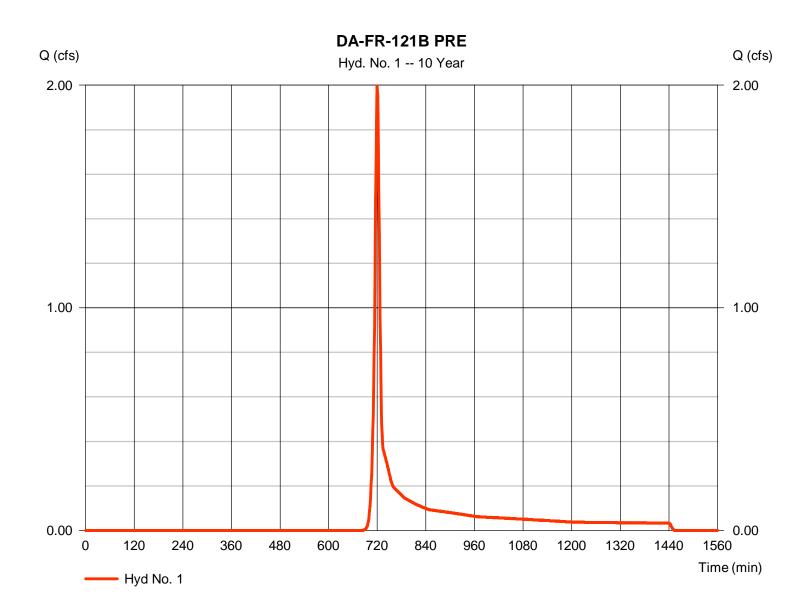
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-121B PRE

= SCS Runoff Hydrograph type Peak discharge = 1.995 cfsStorm frequency Time to peak = 720 min = 10 yrsTime interval = 1 min Hyd. volume = 4,727 cuftDrainage area Curve number = 0.870 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 9.80 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.038 \times 87) + (0.071 \times 58) + (0.766 \times 55)] / 0.870$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

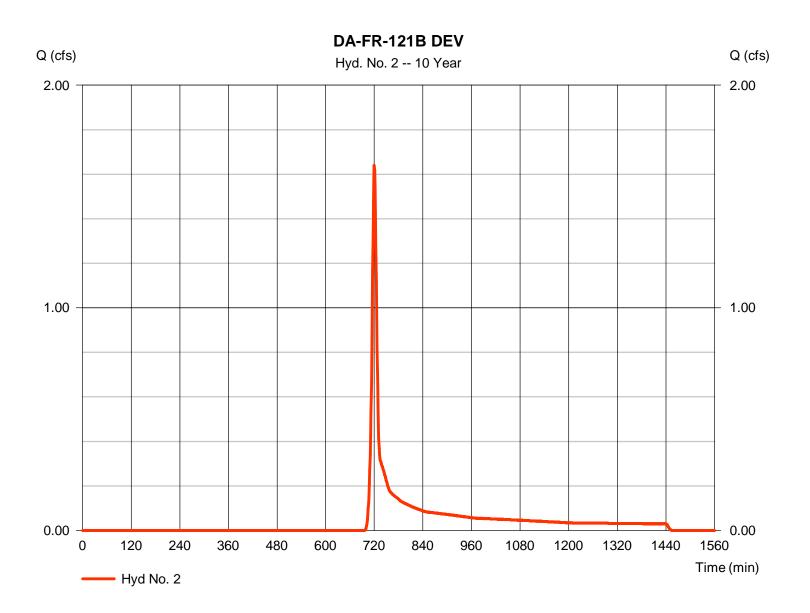
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-121B DEV

= SCS Runoff Hydrograph type Peak discharge = 1.639 cfsStorm frequency Time to peak = 720 min = 10 yrsTime interval = 1 min Hyd. volume = 4,030 cuftDrainage area Curve number = 0.870 ac= 54*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 9.80 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.428 \times 48) + (0.038 \times 82) + (0.408 \times 58)] / 0.870$



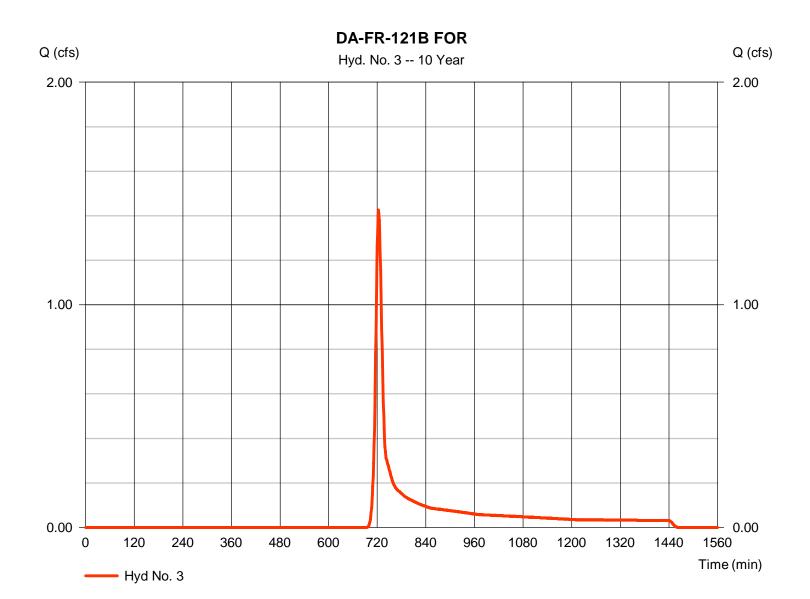
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-121B FOR

= SCS Runoff = 1.425 cfsHydrograph type Peak discharge Storm frequency Time to peak = 723 min = 10 yrsTime interval = 1 min Hyd. volume = 4,259 cuftDrainage area Curve number = 0.870 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 14.10 min Total precip. Distribution = 5.70 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Return Period	Intensity-Du	Intensity-Duration-Frequency Equation Coefficients (FHA)							
(Yrs)	В	D	E	(N/A)					
1	0.0000	0.0000	0.0000						
2	69.8703	13.1000	0.8658						
3	0.0000	0.0000	0.0000						
5	79.2597	14.6000	0.8369						
10	88.2351	15.5000	0.8279						
25	102.6072	16.5000	0.8217						
50	114.8193	17.2000	0.8199						
100	127.1596	17.8000	0.8186						

File name: SampleFHA.idf

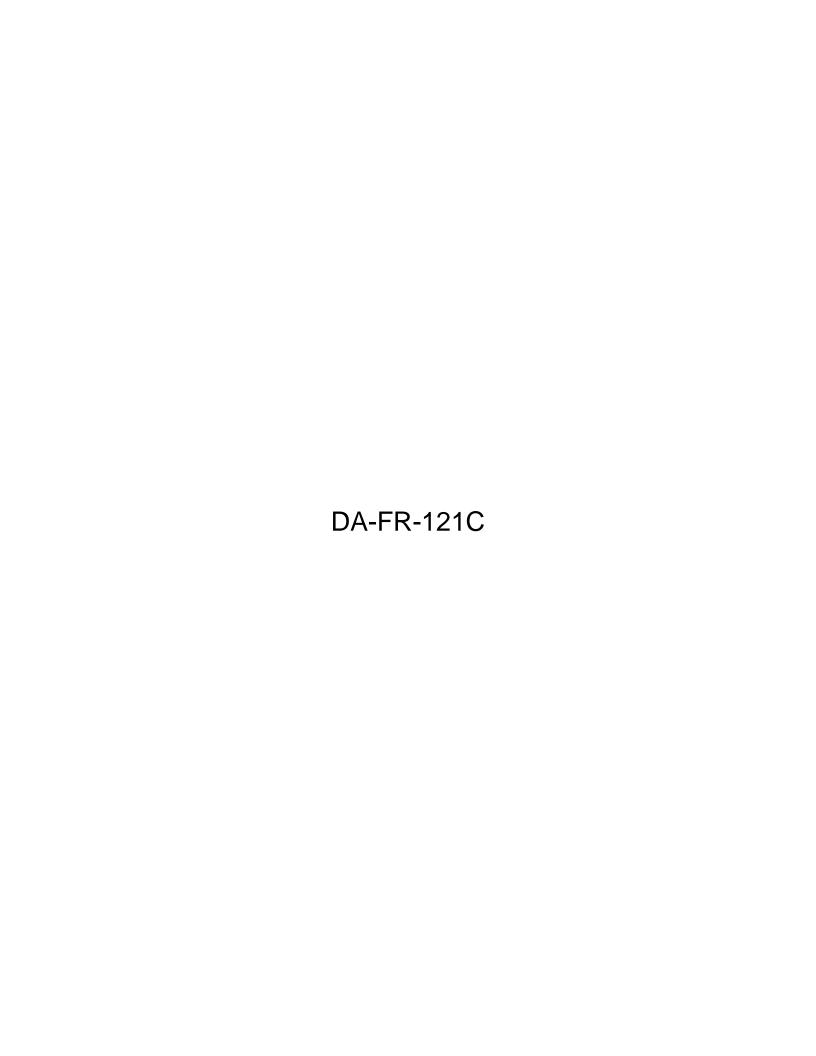
Intensity = $B/(Tc + D)^E$

Return		Intensity Values (in/hr)											
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60	
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70	
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15	
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46	
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91	
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25	
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60	

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc|

	Rainfall Precipitation Table (in)									
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr		
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00		
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		



ENERGY BALANCE METHOD

In		

	1-Y	r Event
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)
Pre-Developed Condition	0.169	1134
Developed Condition	0.056	790
Pre-Developed (Forest) Condition	0.169	1136

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF =

<u>Calculations:</u>	Check #1:	$Q_{developed} \le IF \times \{(Q_{pre-developed} \times RV_{pre-developed}) / RV_{developed}\}$	0.056	≤ N/A - See Check #3	0.194
	Check #2:	$Q_{developed} \le Q_{pre-developed}$ \Longrightarrow	0.056	≤ N/A - See Check #3	0.169
	Check #3:	$Q_{developed} \frac{shall\ not}{not}$ be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed}$ >	0.056	shall not be required to be ≤	0.243

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	·
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	<mark>0.40</mark>
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 – Manning's n Values for Open Channel Flow

nan	nel T	уре		Manning n	
			Min.	Normal	Max
1.	Exc	avated or Dredged Channels¹			
	a.	Earth, Straight, and Uniform:			
		Clean, recently completed	0.016	0.018	0.020
		Clean, after weathering	0.018	0.022	0.02
		Gravel, uniform section, clean	0.022	0.025	0.030
		With short grass, few weeds	0.022	0.027	0.03
	b.	Earth Winding and Sluggish:			
		No vegetation	0.023	0.025	0.030
		Grass, some weeds	0.025	0.030	0.03
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040
		Earth bottom and rubble sides	0.028	0.030	0.03
		Stony bottom and weedy banks	0.025	0.035	0.040
		Cobble bottom and clean sides	0.030	<mark>0.040</mark>	0.050
	c.	Dragline-Excavated or Dredged:		•	
		No vegetation	0.025	0.028	0.03
		Light brush on banks	0.035	0.050	0.06
	d.	Rock Cuts:			
		Smooth and uniform	0.025	0.035	0.040
		Jagged and irregular	0.035	0.040	0.050
	e.	Channels not Maintained, Weeds and Brush Uncut:			
		Dense weeds, high as flow depth	0.050	0.080	0.120
		Clean bottom, brush on sides	0.040	0.050	0.080
		Same as above, highest stage of flow	0.045	0.070	0.110
		Dense brush, high stage	0.080	0.100	0.140
2.	Mai	n Channels²			
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.03
	b.	Same as above, but more stones and weeds	0.030	0.035	0.04
	c.	Clean, winding, some pools and shoals	0.033	0.040	0.04
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.05
	f.	Same as (d) with more stones	0.045	0.050	0.060
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

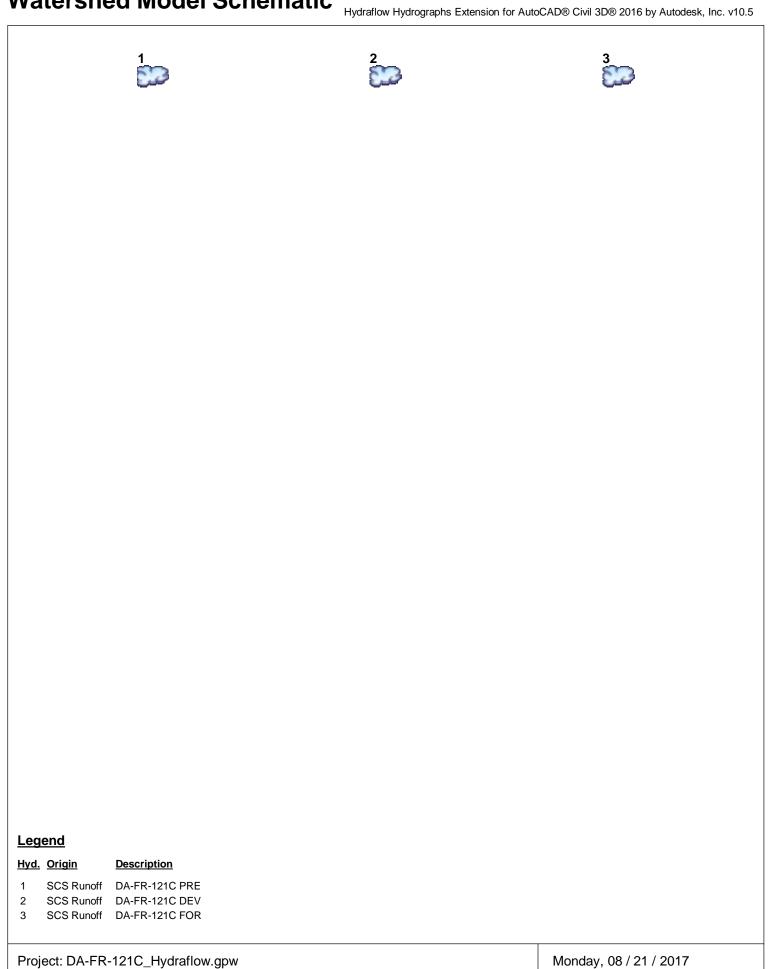
-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	4
Hydrograph No. 1, SCS Runoff, DA-FR-121C PRE	
TR-55 Tc Worksheet	
Hydrograph No. 2, SCS Runoff, DA-FR-121C DEV	
TR-55 Tc Worksheet	7
Hydrograph No. 3, SCS Runoff, DA-FR-121C FOR	
TR-55 Tc Worksheet	9
2 - Year	
Summary Report	10
Hydrograph Reports	11
Hydrograph No. 1, SCS Runoff, DA-FR-121C PRE	11
Hydrograph No. 2, SCS Runoff, DA-FR-121C DEV	12
Hydrograph No. 3, SCS Runoff, DA-FR-121C FOR	
10 - Year	
	4.4
Summary Report	
Hydrograph Reports Hydrograph No. 1, SCS Runoff, DA-FR-121C PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-121C DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-121C FOR	10
Tiyurugrapii No. 3, 303 Kurioli, DA-FR-1210 FOR	17
IDE Papart	1Ω



Hydrograph Return Period Recap Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.		Inflow hyd(s)				Hydrograph					
0.			1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		0.169	0.335			1.697				DA-FR-121C PRE
2	SCS Runoff		0.056	0.126			0.891				DA-FR-121C DEV
3	SCS Runoff		0.169	0.336			1.700				DA-FR-121C FOR

Proj. file: DA-FR-121C_Hydraflow.gpw

Monday, 08 / 21 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	0.169	1	727	1,134				DA-FR-121C PRE	
2	SCS Runoff	0.056	1	749	790				DA-FR-121C DEV	
2										
DA-FR-121C_Hydraflow.gpw					Return F	Period: 1 Ye	ear	Monday, 08 / 21 / 2017		

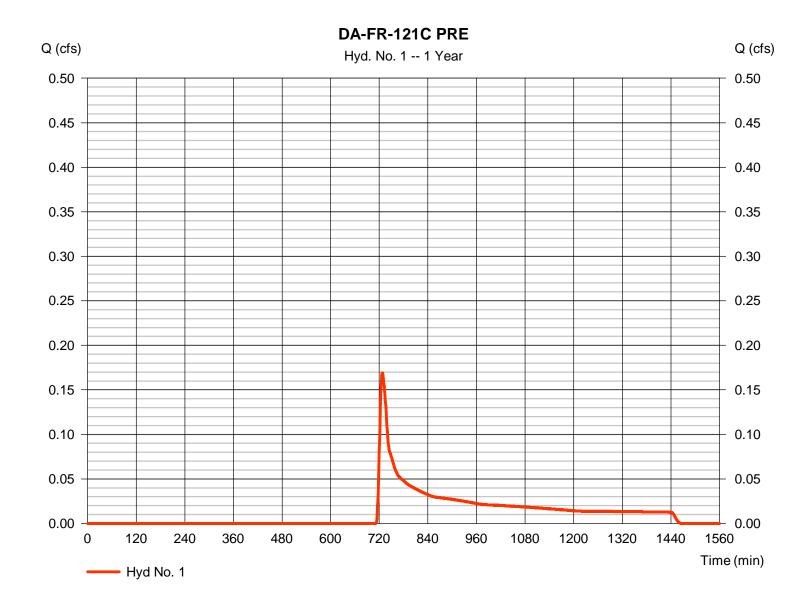
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-121C PRE

Hydrograph type = SCS Runoff Peak discharge = 0.169 cfsStorm frequency Time to peak = 727 min = 1 yrsTime interval = 1 min Hyd. volume = 1,134 cuftDrainage area Curve number = 1.098 ac= 55 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 15.30 min Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5 $\,$

Hyd. No. 1DA-FR-121C PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 4.59 = 14.32	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	14.32
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 357.25 = 15.04 = Unpaved =6.26	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.95	+	0.00	+	0.00	=	0.95
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							15.30 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

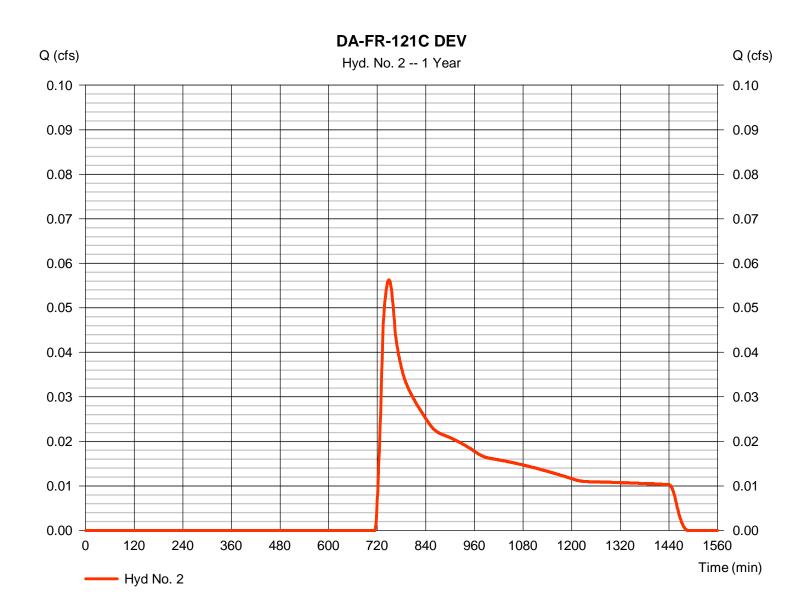
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-121C DEV

Hydrograph type = SCS Runoff Peak discharge = 0.056 cfsStorm frequency Time to peak = 749 min = 1 yrsTime interval = 1 min Hyd. volume = 790 cuft Drainage area Curve number = 1.100 ac= 52*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 29.20 min Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.628 x 48) + (0.470 x 58)] / 1.100



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2

DA-FR-121C DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 33.2 = 3.70 = 0.70 = 12.59	+	0.800 66.8 3.70 6.52 15.69	+	0.011 0.0 0.00 0.00 0.00	=	28.28
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 107.61 = 11.46 = Unpave =5.46	d	186.36 20.75 Unpave 7.35	d	35.18 6.12 Unpave 3.99	ed	
Travel Time (min)	= 0.33	+	0.42	+	0.15	=	0.90
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		2.00 4.47 5.00 0.040 4.86		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})8.5		4.1		0.0		
Travel Time (min)	= 0.03	+	0.01	+	0.00	=	0.04
Total Travel Time, Tc							29.20 min

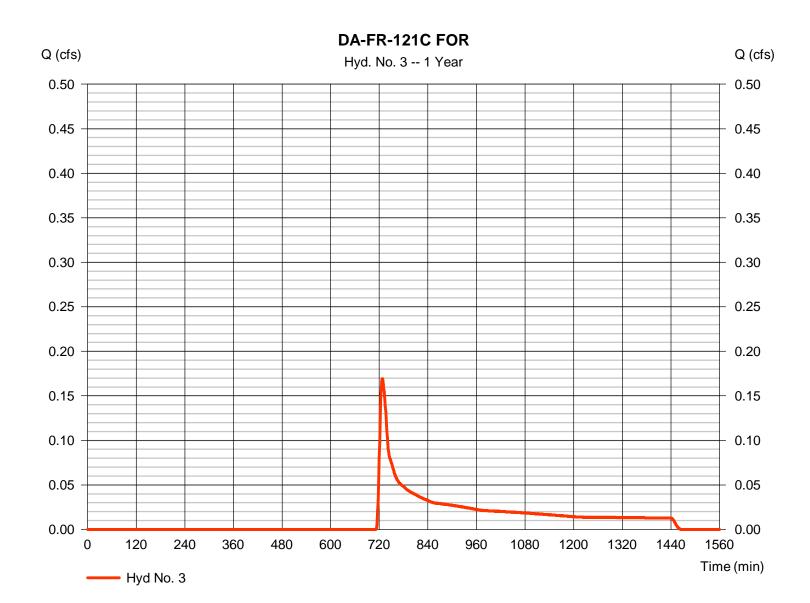
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-121C FOR

Hydrograph type = SCS Runoff Peak discharge = 0.169 cfsStorm frequency Time to peak = 727 min = 1 yrsTime interval = 1 min Hyd. volume = 1,136 cuftDrainage area Curve number = 1.100 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 15.30 min Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5 $\,$

Hyd. No. 3

DA-FR-121C FOR

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 4.59		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 14.32	+	0.00	+	0.00	=	14.32
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 357.25 = 15.04 = Unpave =6.26	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.95	+	0.00	+	0.00	=	0.95
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							15.30 min

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.335	1	726	1,677				DA-FR-121C PRE
2	SCS Runoff	0.126	1	740	1,238				DA-FR-121C DEV
2									
DA-FR-121C_Hydraflow.gpw				Return F	eriod: 2 Ye	ear	Monday, 08 / 21 / 2017		

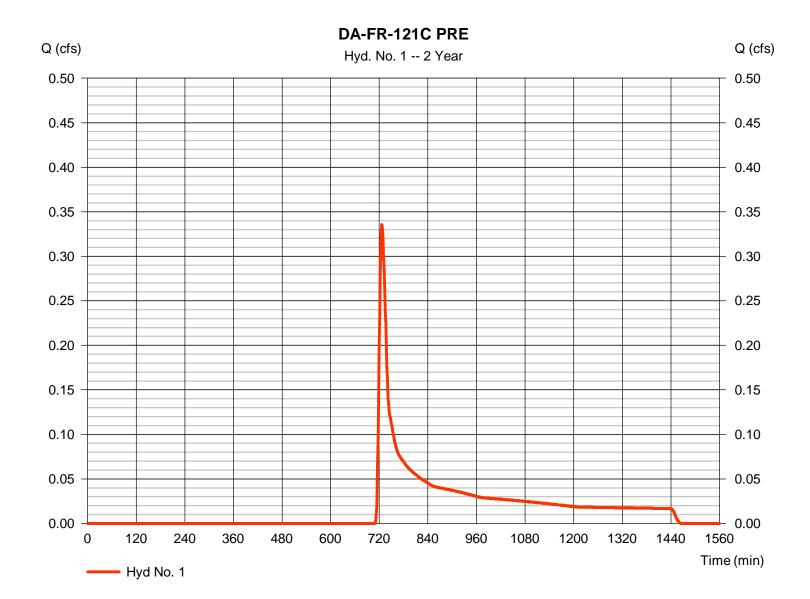
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-121C PRE

Hydrograph type = SCS Runoff Peak discharge = 0.335 cfsStorm frequency Time to peak = 726 min = 2 yrsTime interval = 1 min Hyd. volume = 1,677 cuftDrainage area Curve number = 1.098 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 15.30 min Total precip. Distribution = 3.70 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

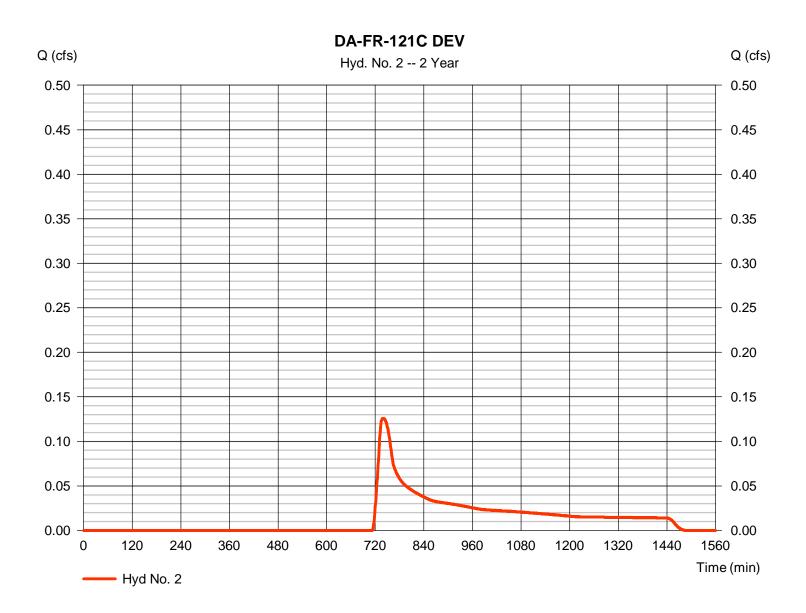
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-121C DEV

Hydrograph type = SCS Runoff Peak discharge = 0.126 cfsStorm frequency Time to peak = 740 min = 2 yrsTime interval = 1 min Hyd. volume = 1.238 cuft Drainage area Curve number = 1.100 ac= 52*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 29.20 min Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.628 x 48) + (0.470 x 58)] / 1.100



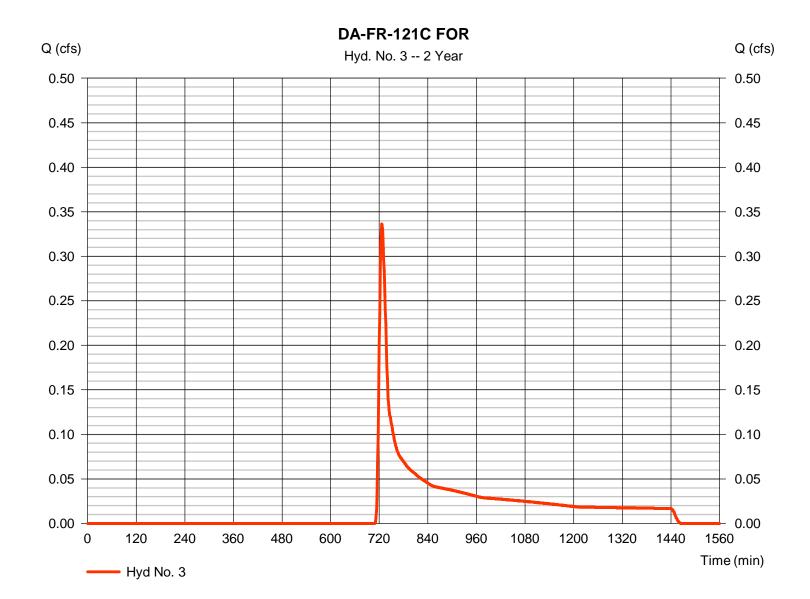
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-121C FOR

Hydrograph type = SCS Runoff Peak discharge = 0.336 cfsStorm frequency Time to peak = 726 min = 2 yrsTime interval = 1 min Hyd. volume = 1,680 cuftDrainage area Curve number = 1.100 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 15.30 min Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	1.697	1	724	5,441				DA-FR-121C PRE	
2	SCS Runoff	0.891	1	734	4,532				DA-FR-121C DEV	
DA-FR-121C_Hydraflow.gpw					Return F	Period: 10 Y	′ear	Monday, 08 / 21 / 2017		

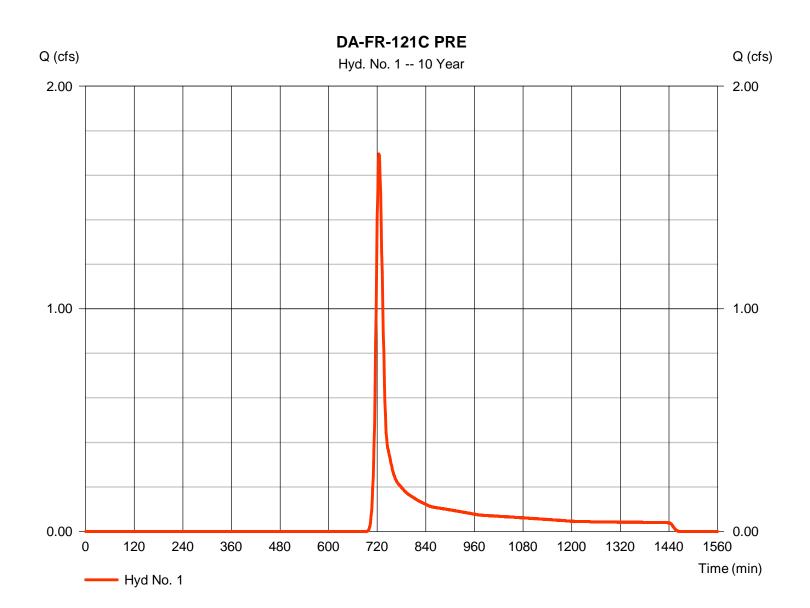
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-121C PRE

Hydrograph type = SCS Runoff Peak discharge = 1.697 cfsStorm frequency Time to peak = 724 min = 10 yrsTime interval = 1 min Hyd. volume = 5,441 cuftDrainage area Curve number = 1.098 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.30 min Total precip. Distribution = 5.70 in= Type II Storm duration = 484 = 24 hrs Shape factor



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

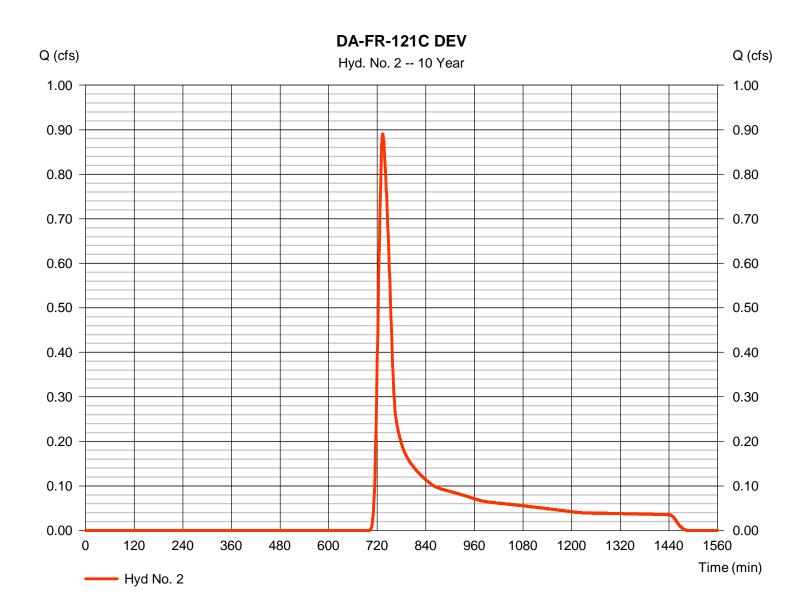
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-121C DEV

= SCS Runoff Hydrograph type Peak discharge = 0.891 cfsStorm frequency Time to peak = 734 min = 10 yrsTime interval = 1 min Hyd. volume = 4,532 cuftDrainage area Curve number = 1.100 ac= 52*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 29.20 min Total precip. = 5.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = [(0.628 x 48) + (0.470 x 58)] / 1.100



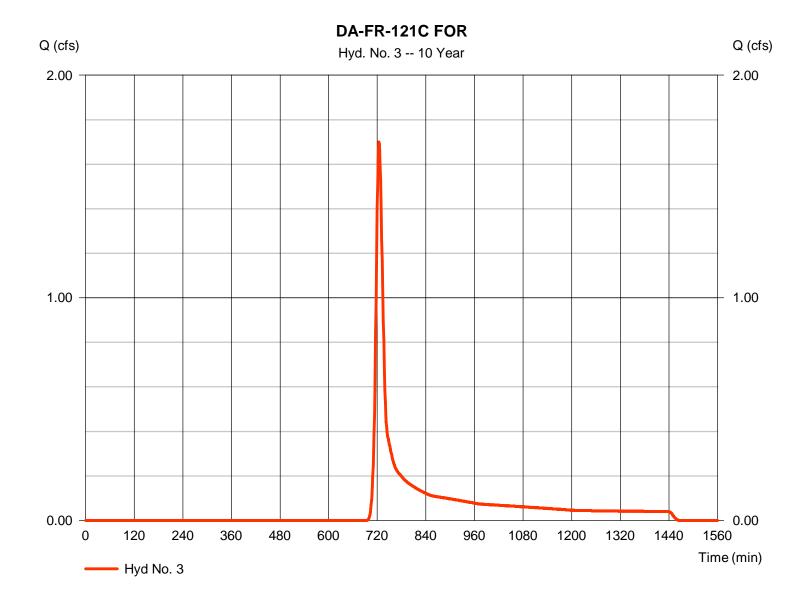
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-121C FOR

Hydrograph type = SCS Runoff = 1.700 cfsPeak discharge Storm frequency Time to peak = 724 min = 10 yrsTime interval = 1 min Hyd. volume = 5,452 cuftDrainage area Curve number = 1.100 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 15.30 min Total precip. Distribution = 5.70 in= Type II Storm duration = 484 = 24 hrs Shape factor



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Return Period	Intensity-Du	Intensity-Duration-Frequency Equation Coefficients (FHA)									
(Yrs)	В	D	E	(N/A)							
1	0.0000	0.0000	0.0000								
2	69.8703	13.1000	0.8658								
3	0.0000	0.0000	0.0000								
5	79.2597	14.6000	0.8369								
10	88.2351	15.5000	0.8279								
25	102.6072	16.5000	0.8217								
50	114.8193	17.2000	0.8199								
100	127.1596	17.8000	0.8186								

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return					(in/hr)							
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc

	Rainfall Precipitation Table (in)									
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr		
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00		
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		

DA-FR-122

DA-FR-122 is located in a meadow and forested areas with rolling slopes and contains existing dirt road and houses. The total phosphorus load reduction required for DA-FR-122 is -1.02 lb/yr. Total phosphorus load reduction is not required for DA-FR-122. Multiple points of analysis were evaluated within DA-FR-122 to evaluate the effects on each receiving stream/channel following construction. Specifically, DA-FR-122 was subdivided into six sub-drainage areas (sub-areas A through F).

Sub-areas 122E and 122F contain both agricultural and non-agricultural areas within the limits of disturbance (LOD). Pre-construction agricultural areas will be returned to agricultural land use (i.e., returned to crop production, in identical condition) following construction. In non-agricultural areas, land use will be restored following construction as noted in the Stormwater Management (SWM) Narrative and the Annual Standards and Specifications. Agricultural areas within the LOD are included in the SWM quality analysis and the total permanent Right of Way (ROW) is analyzed via VRRM; in these calculations agricultural areas are considered "Forest/Open Space".

Stormwater quantity is met via the energy balance method for each of the six sub-areas DA-FR-122A through DA-FR-122F. Agricultural areas within the study area are included in the SWM analysis, but an Improvement Factor (IF) of 1.0 is used when applying the Energy Balance Method. This improvement factor is used to account for the exemption of agricultural areas (§ 62.1-44.15:34 and 9VAC25-870-300) since such areas will be returned to agricultural land use (i.e., returned to crop production, in identical condition) following construction.

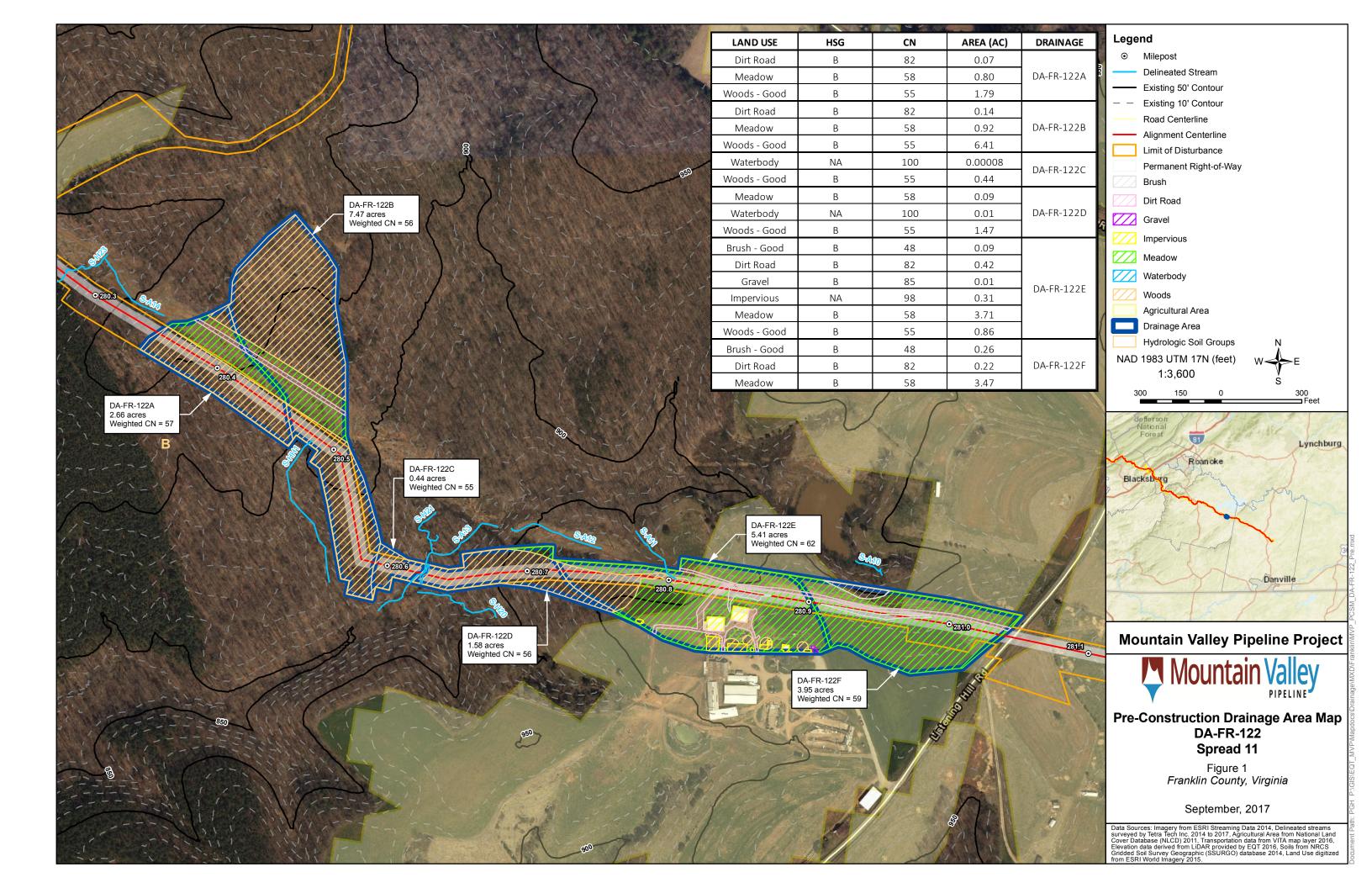
In addition, the Hydraflow Hydrograph's 10-year 24-hour peak discharge results indicate a reduction in flows ranging from 0 to 0.84 cfs for all drainage areas (as seen in table below).

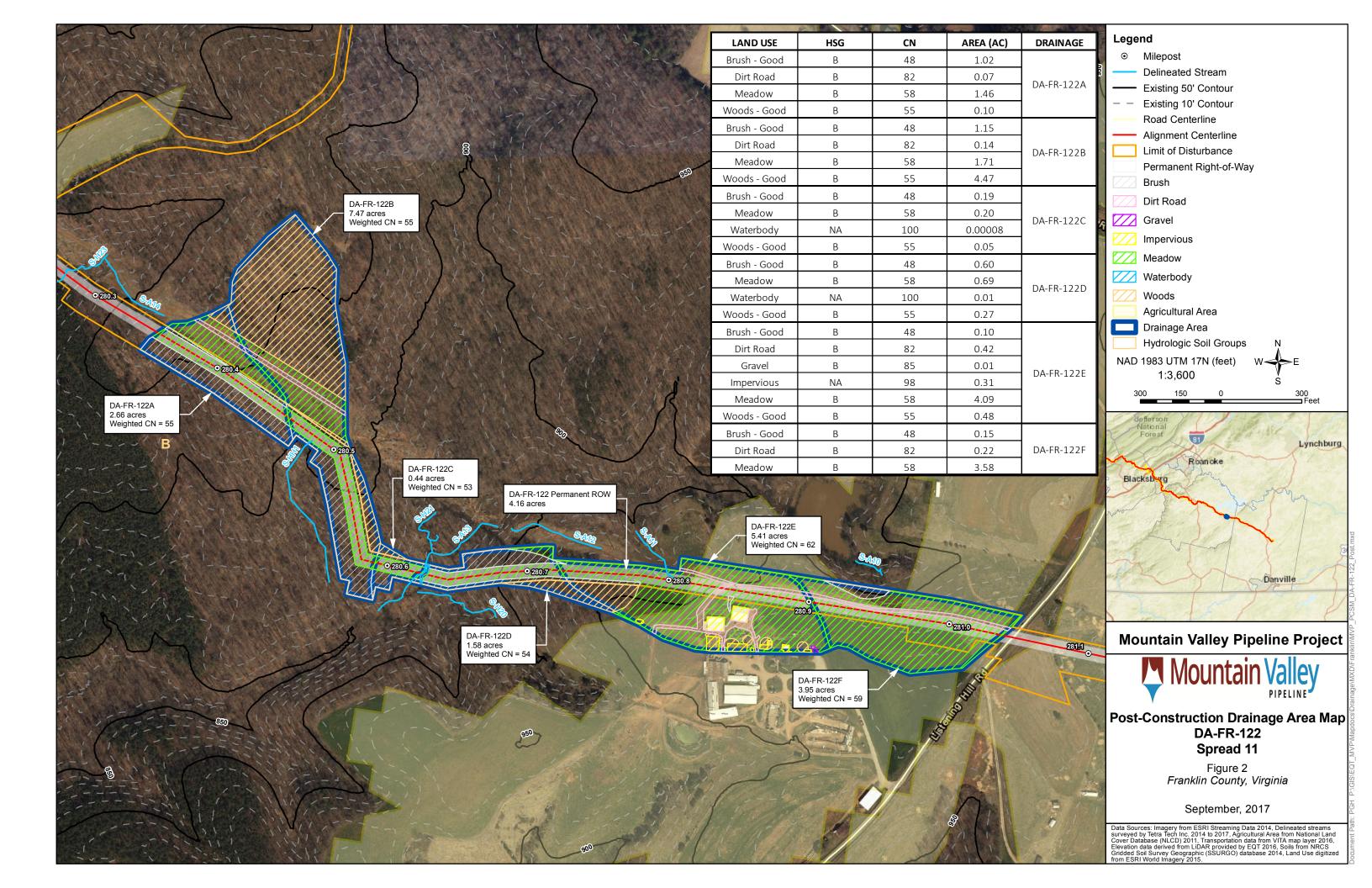
Sub Area	Pre Peak Flow,	Post Peak Flow, Q	Flow differential
	10-yr Q (cfs)	10-yr (cfs)	
DA-FR-122A	4.98	4.36	-0.62
DA-FR-122B	12.38	11.54	-0.84
DA-FR-122C	0.89	0.77	-0.12
DA-FR-122D	3.20	2.56	-0.64
DA-FR-122E	13.34	13.34	0
DA-FR-122F	7.89	7.89	0

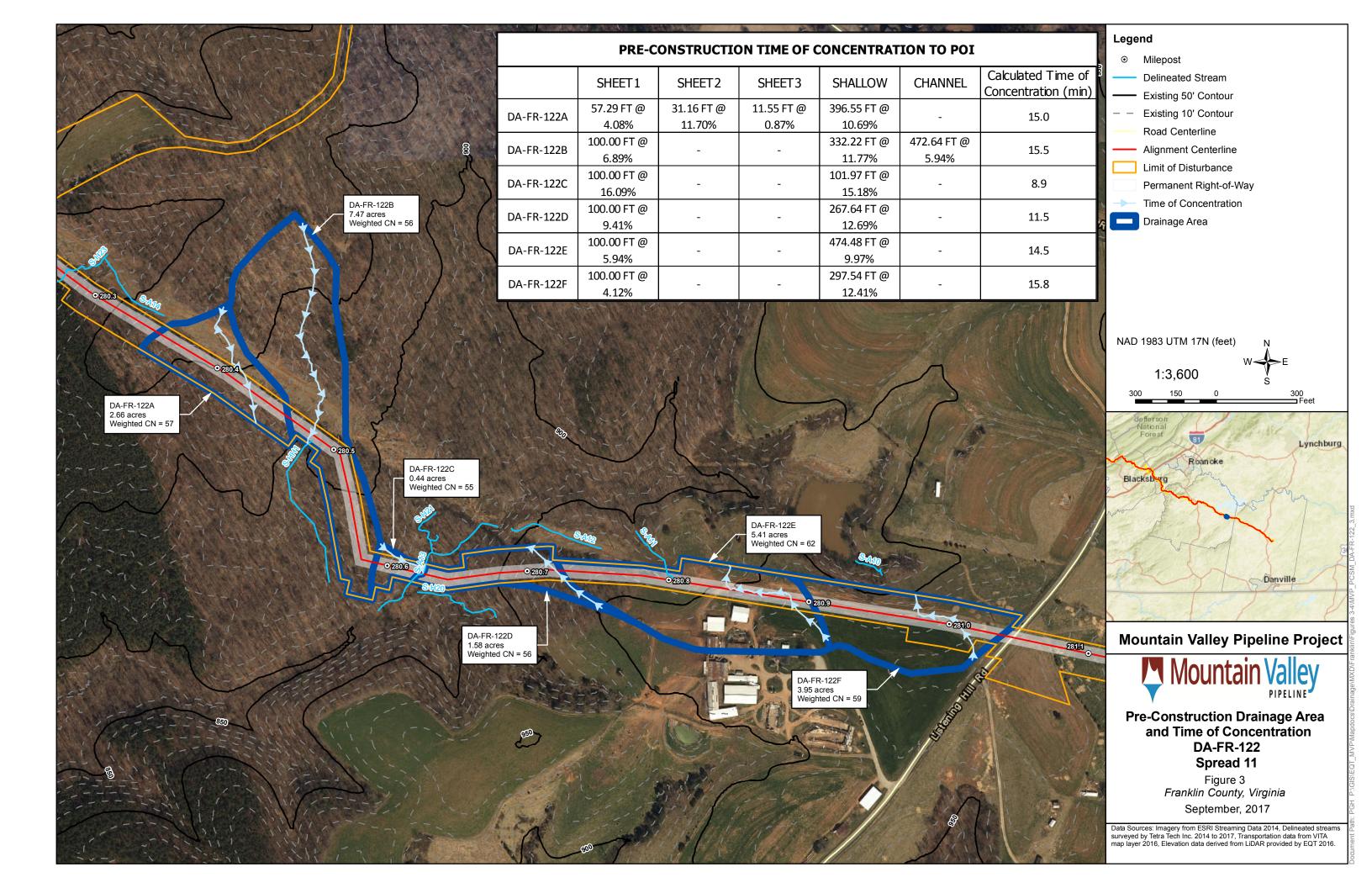
Figures and calculations for each of the sub-areas for DA-FR-122 follow. See Appendix D of the Annual Standards and Specifications for further detail on stormwater methodology.

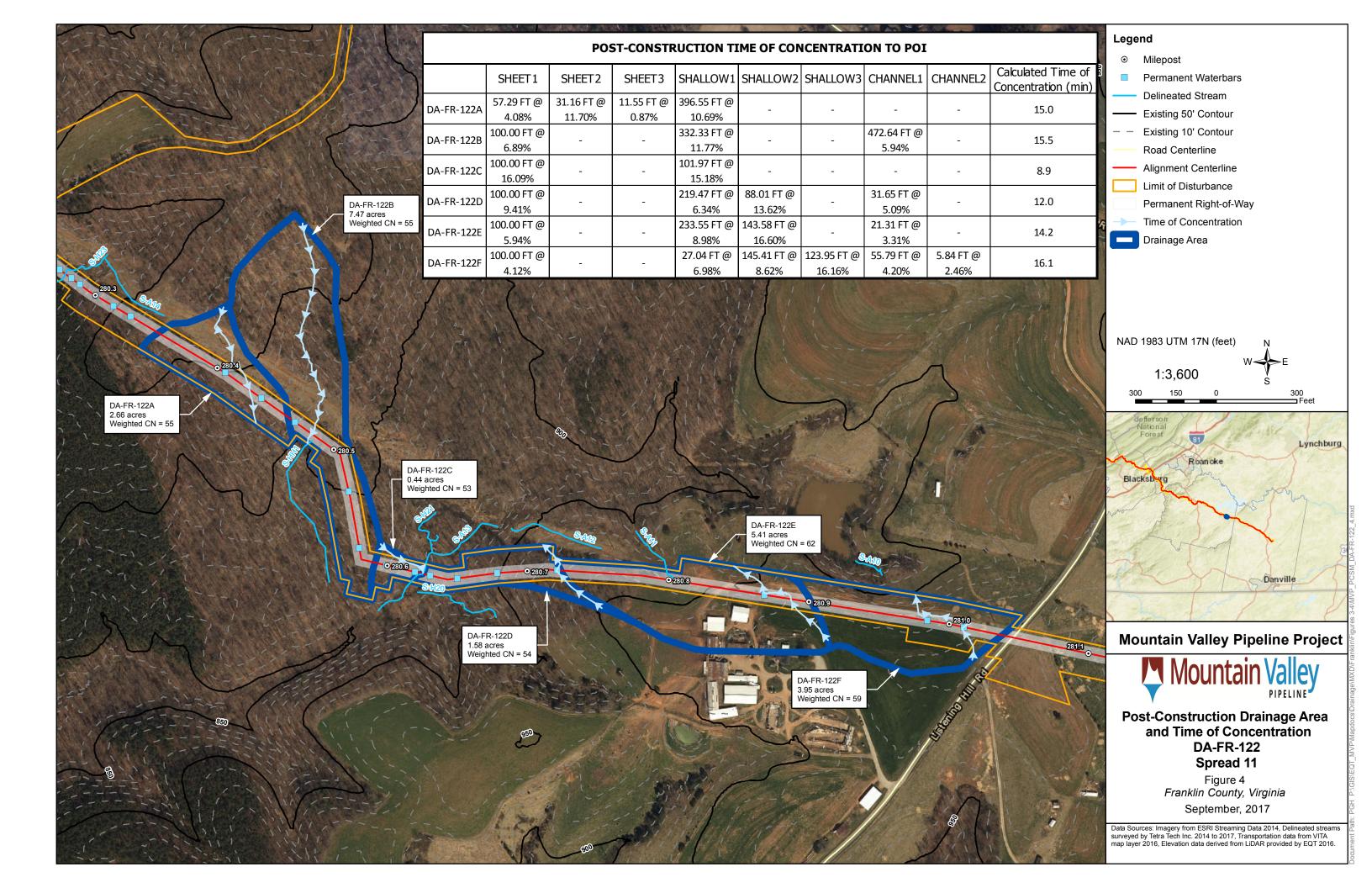
Note that the sub-areas 122E and 122F had time of concentration flow paths for the Post-Construction condition that accounted for a permanent water bar that is not cited within the drainage area. This permanent water bar was removed because it was cited

in an agricultural area, and resulting changes to the stormwater calculations were flagged during the QA/QC review process. This change was considered to be inconsequential because no stormwater BMPs are sited in DA-FR-122E or 122F, so the resulting changes were not made to the stormwater calculations prior to submittal.









DEQ Virginia Runoff Reduction Method Re-Development Compliance Spreadsheet - Version 3.0

BMP Design Specifications List: 2013 Draft Stds & Specs

Site Summary - Linear Development Project***

Total Rainfall (in):	43
Total Disturbed Acreage:	4.16

Site Land Cover Summary

Pre-ReDevelopment Land Cover (acres)

(4.51						
·	A soils	B Soils	C Soils	D Soils	Totals	% of Total
Forest/Open (acres)	0.00	3.97	0.00	0.00	3.97	95
Managed Turf (acres)	0.00	0.00	0.00	0.00	0.00	0
Impervious Cover (acres)	0.00	0.19	0.00	0.00	0.19	5
					4.16	100

Post-ReDevelopment Land Cover (acres)

~	A soils	B Soils	C Soils	D Soils	Totals	% of Total
Forest/Open (acres)	0.00	3.97	0.00	0.00	3.97	95
Managed Turf (acres)	0.00	0.00	0.00	0.00	0.00	0
Impervious Cover (acres)	0.00	0.19	0.00	0.00	0.19	5
* Forest/Open Space areas must be protected in	4.16	100				

Site Tv and Land Cover Nutrient Loads

	Final Post-Development (Post-ReDevelopment & New Impervious)	Post- ReDevelopment	Post- Development (New Impervious)	Adjusted Pre- ReDevelopment
Site Rv	0.07	0.07		0.07
Treatment Volume (ft ³)	1,088	1,088		1,088
TP Load (lb/yr)	0.68	0.68		0.68
	Deseller TD Lead (III /).	1 7056*	*Badustian balau nau	dayalanmant laad limita

Pre- ReDevelopment TP Load per acre (lb/acre/yr)	Final Post-Development TP Load per acre (lb/acre/yr)	Post-ReDevelopment TP Load per acre (lb/acre/yr)
0.16	0.16	0.16

*Reduction below new development load limitation not required

Total TP Load Reduction Required (lb/yr)	-1.02	N/A***	N/A***
--	-------	--------	--------

***This is a linear development project

	Final Post-Development Load (Post-ReDevelopment & New Impervious)	Pre- ReDevelopment
TN Load (lb/yr)	4.89	4.89

Site Compliance Summary - ***Linear Development Project

Maximum % Reduction Required Below	20%	* Note: % Reduction will reduce post-development TP load to less than or equal to baseline load of 1.71 lb/yr (0.41 lb/ac/yr)
Pre-ReDevelopment Load	20%	[Post-Dev Reduction Requirement = Post-Dev TP load - baseline load of 1.71 lb/yr], baseline load = site area x 0.41 lb/ac/yr

Total Runoff Volume Reduction (ft ³)	
--	--

Total TP Load Reduction Achieved (lb/yr)	0.00
Total TN Load Reduction Achieved (lb/yr)	0.00
Remaining Post Development TP Load (lb/yr)	0.68
Remaining TP Load Reduction (lb/yr) Required	0.00

** TARGET TP REDUCTION EXCEEDED BY 1.02 LB/YEAR **

^{*}Reduction below new development load limitation not required



ENERGY BALANCE METHOD

		ts

	1-Yr Event		
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)	
Pre-Developed Condition	0.670	3318	
Developed Condition	0.439	2714	
Pre-Developed (Forest) Condition	0.439	2714	

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.8

<u>Calculations:</u>	Check #1:	$Q_{developed} \le IF \times [(Q_{pre-developed} \times RV_{pre-developed}) / RV_{developed}] \qquad>$	0.439	≤ OK	0.655
	Check #2:	Q _{developed} ≤ Q _{pre-developed} >	0.439	≤ OK	0.670
	Check #3:	$Q_{developed} \underline{shall \ not}$ be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed} \longrightarrow$	0.439	<u>shall not</u> be required to be ≤	0.439

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 - 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

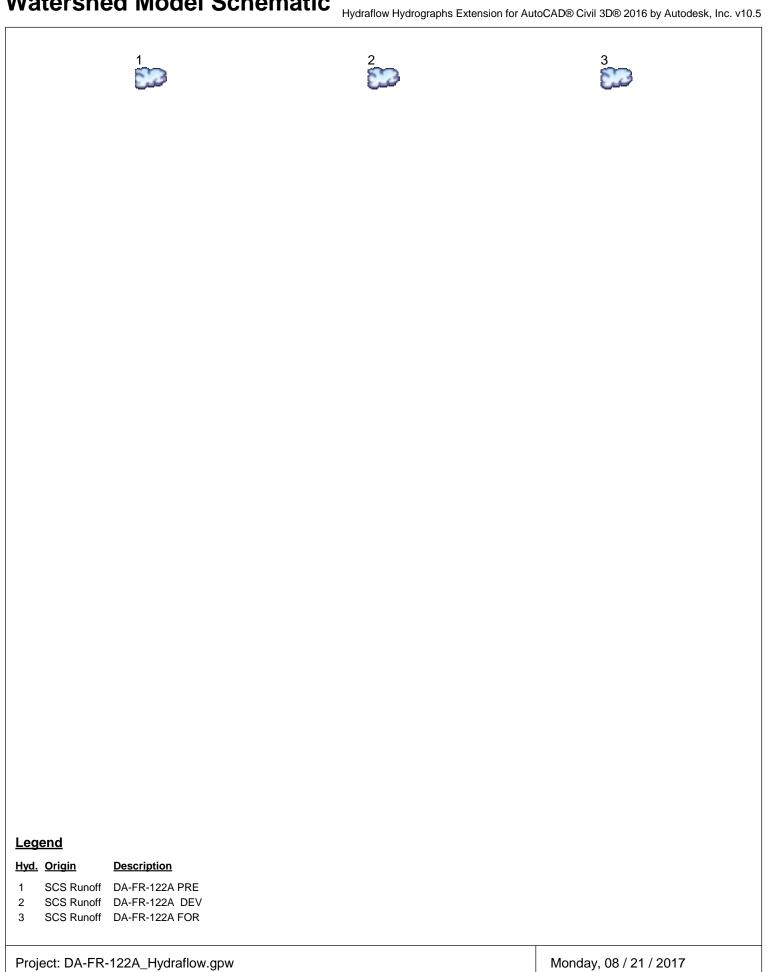
-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-122A PRE	
TR-55 Tc Worksheet	
Hydrograph No. 2, SCS Runoff, DA-FR-122A DEV	6
TR-55 Tc Worksheet	7
Hydrograph No. 3, SCS Runoff, DA-FR-122A FOR	8
TR-55 Tc Worksheet	9
2 - Year	
Summary Report	10
Hydrograph Reports	11
Hydrograph No. 1, SCS Runoff, DA-FR-122A PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-122A DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-122A FOR	13
10 - Year	
	4.4
Summary Report	
Hydrograph No. 4, CCC Burnett DA ED 400A DDE	
Hydrograph No. 1, SCS Runoff, DA-FR-122A PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-122A DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-122A FOR	17
IDE Papart	10



Hydrograph Return Period Recap Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	Hydrograph	Inflow				Hydrograph					
No.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		0.670	1.190			4.977				DA-FR-122A PRE
2	SCS Runoff		0.439	0.872			4.357				DA-FR-122A DEV
3	SCS Runoff		0.439	0.872			4.357				DA-FR-122A FOR

Proj. file: DA-FR-122A_Hydraflow.gpw

Monday, 08 / 21 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

			_		_						
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description		
1	SCS Runoff	0.670	1	725	3,318				DA-FR-122A PRE		
2	SCS Runoff	0.439	1	726	2,714				DA-FR-122A DEV		
3	SCS Runoff	0.439	1	726	2,714				DA-FR-122A FOR		
DA	DA-FR-122A_Hydraflow.gpw					eriod: 1 Ye	 ear	Monday, 08 / 21 / 2017			

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

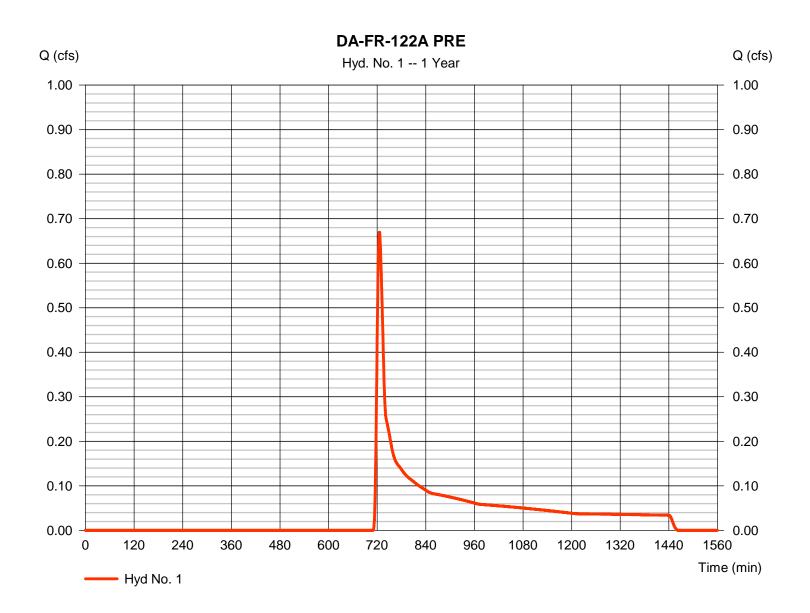
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-122A PRE

Hydrograph type = SCS Runoff Peak discharge = 0.670 cfsStorm frequency Time to peak = 725 min = 1 yrsTime interval = 1 minHyd. volume = 3.318 cuftDrainage area = 57* Curve number = 2.660 acBasin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.00 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.072 x 82) + (0.799 x 58) + (1.788 x 55)] / 2.660



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1 DA-FR-122A PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 57.3 = 3.70 = 4.08 = 9.62	+	0.400 31.2 3.70 11.70 3.88	+	0.011 11.6 3.70 0.87 0.28	=	13.77	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 396.53 = 10.69 = Unpaved =5.28	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00			
Travel Time (min)	= 1.25	+	0.00	+	0.00	=	1.25	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015			
Flow length (ft)	({0})0.0		0.0		0.0			
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00	
Total Travel Time, Tc							15.00 min	

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

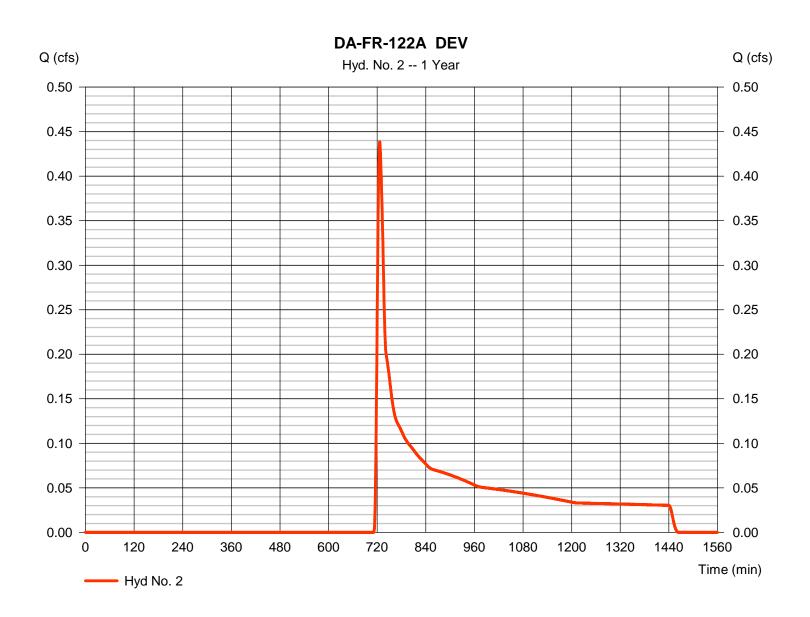
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-122A DEV

Hydrograph type = SCS Runoff Peak discharge = 0.439 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 minHyd. volume = 2.714 cuftDrainage area Curve number = 2.660 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.00 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(1.022 \times 48) + (0.072 \times 82) + (1.461 \times 58) + (0.104 \times 55)] / 2.660$



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2DA-FR-122A DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 57.3 = 3.70 = 4.08		0.400 31.2 3.70 11.70		0.011 11.6 3.70 0.87		12 77
Travel Time (min)	= 9.62	+	3.88	+	0.28	=	13.77
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 396.55 = 10.69 = Unpaved =5.28	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.25	+	0.00	+	0.00	=	1.25
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							15.00 min

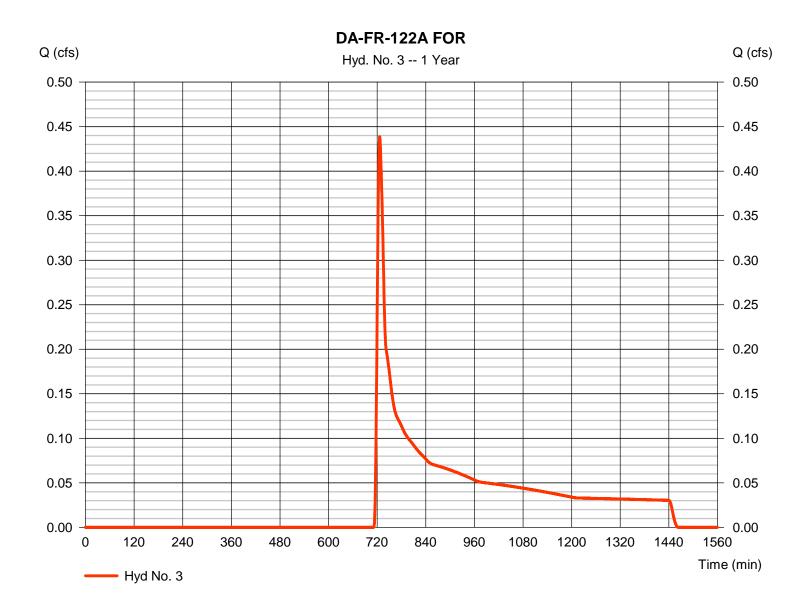
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-122A FOR

Hydrograph type = SCS Runoff Peak discharge = 0.439 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 minHyd. volume = 2.714 cuftDrainage area Curve number = 2.660 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.00 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Storm duration Shape factor = 24 hrs = 484



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3 DA-FR-122A FOR

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>		
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 57.3 = 3.70 = 4.08	+	0.400 31.2 3.70 11.70	+	0.011 11.6 3.70 0.87	=	13.77		
, ,									
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 396.53 = 10.69 = Unpaved =5.28	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00				
Travel Time (min)	= 1.25	+	0.00	+	0.00	=	1.25		
Channel Flow									
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015		0.00 0.00 0.00 0.015				
					0.00				
Flow length (ft)	({0})0.0		0.0		0.0				
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00		
Total Travel Time, Tc									

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	1.190	1	725	4,762				DA-FR-122A PRE
2	SCS Runoff	0.872	1	725	4,013				DA-FR-122A DEV
3	SCS Runoff	0.872	1	725	4,013				DA-FR-122A FOR
DA	-FR-122A_H	ydraflow.g	gpw		Return P	Period: 2 Ye	ear	Monday, 08	3 / 21 / 2017
					1			i .	

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

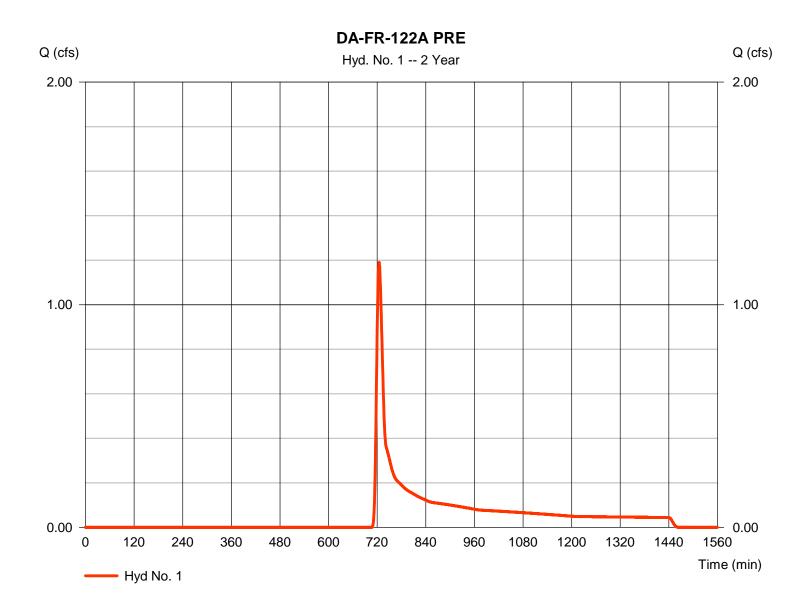
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-122A PRE

Hydrograph type = SCS Runoff Peak discharge = 1.190 cfsStorm frequency = 2 yrsTime to peak = 725 min Time interval = 1 minHyd. volume = 4,762 cuftDrainage area Curve number = 2.660 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 15.00 min = TR55 Total precip. = 3.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.072 x 82) + (0.799 x 58) + (1.788 x 55)] / 2.660



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

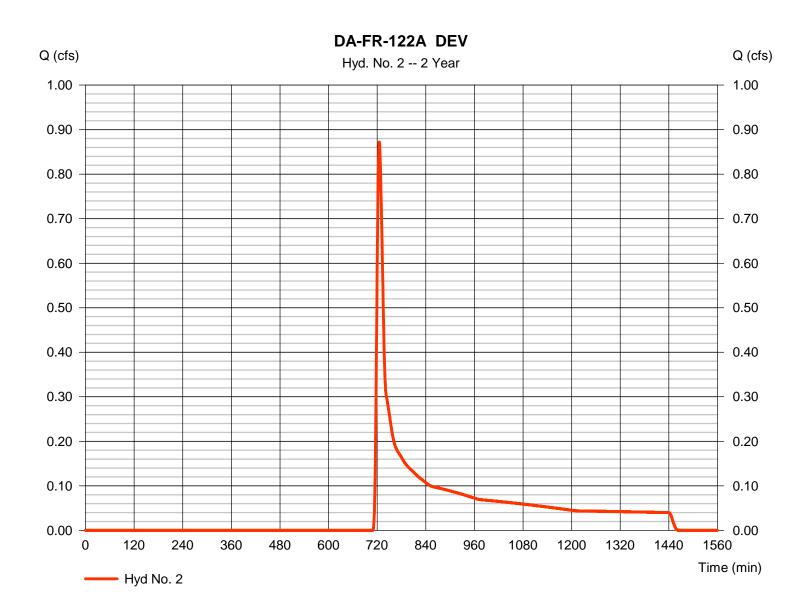
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-122A DEV

Hydrograph type = SCS Runoff Peak discharge = 0.872 cfsStorm frequency = 2 yrsTime to peak = 725 min Time interval = 1 minHyd. volume = 4.013 cuftCurve number Drainage area = 2.660 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.00 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484= 24 hrs

^{*} Composite (Area/CN) = [(1.022 x 48) + (0.072 x 82) + (1.461 x 58) + (0.104 x 55)] / 2.660



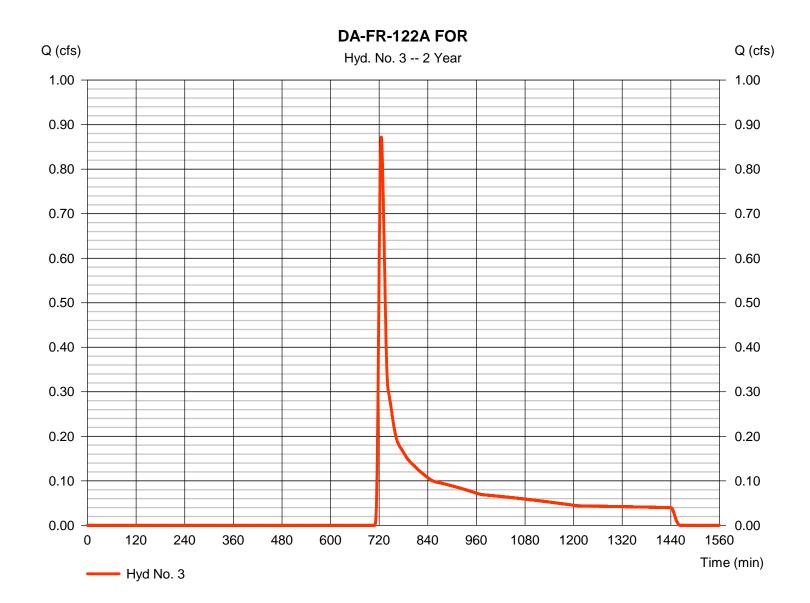
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-122A FOR

Hydrograph type = SCS Runoff Peak discharge = 0.872 cfsStorm frequency = 2 yrsTime to peak = 725 min Time interval = 1 min Hyd. volume = 4.013 cuftDrainage area Curve number = 2.660 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 15.00 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

lyd. lo.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	4.977	1	723	14,454				DA-FR-122A PRE
2	SCS Runoff	4.357	1	723	13,021				DA-FR-122A DEV
1 2 3									
DA	⊥ -FR-122A_H <u>y</u>	_ √draflow.g	Jpw		Return F	Period: 10 Y	⊥ ∕ear	Monday, 08	

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

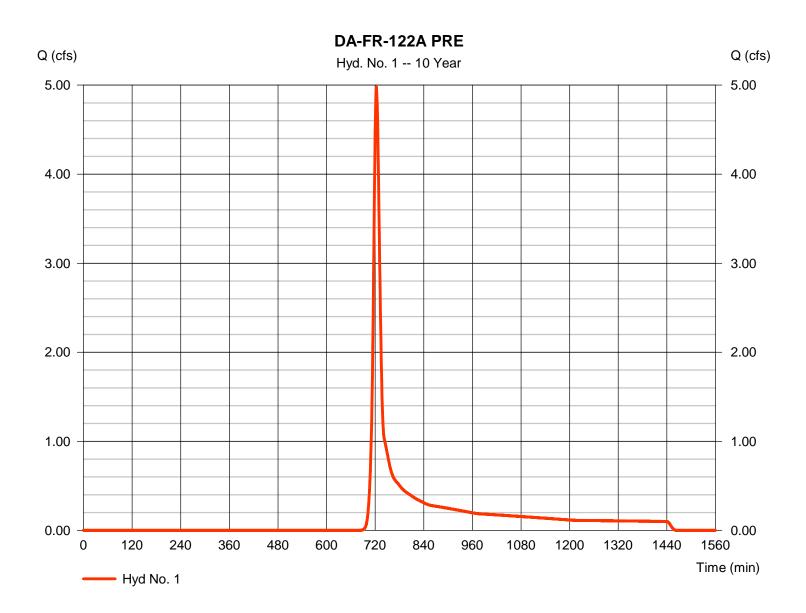
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-122A PRE

Hydrograph type = SCS Runoff Peak discharge = 4.977 cfsStorm frequency = 10 yrsTime to peak = 723 min Time interval = 1 minHyd. volume = 14,454 cuftCurve number Drainage area = 2.660 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 15.00 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inStorm duration = 24 hrs = 484Shape factor

^{*} Composite (Area/CN) = [(0.072 x 82) + (0.799 x 58) + (1.788 x 55)] / 2.660



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

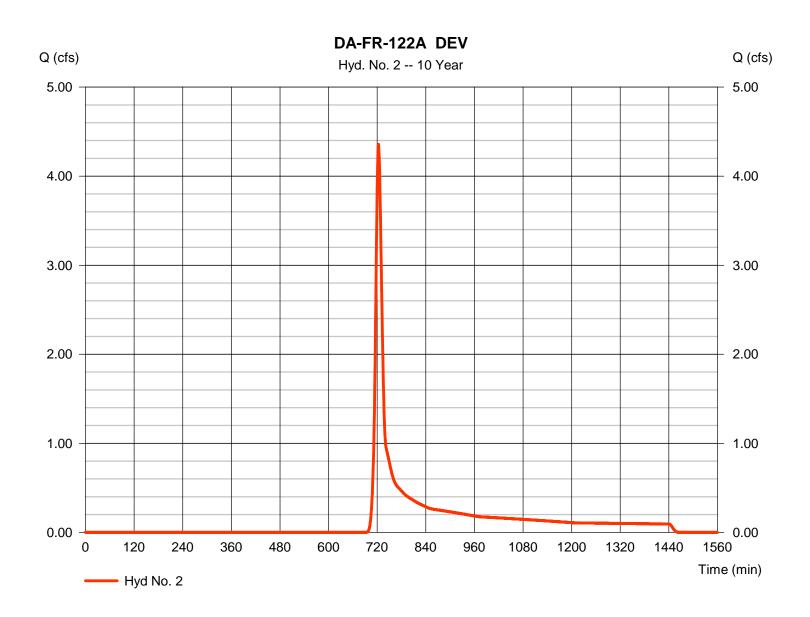
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-122A DEV

Hydrograph type = SCS Runoff Peak discharge = 4.357 cfsStorm frequency = 10 yrsTime to peak = 723 min Time interval = 1 minHyd. volume = 13.021 cuftDrainage area = 2.660 acCurve number = 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 15.00 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inStorm duration = 24 hrs = 484Shape factor

^{*} Composite (Area/CN) = $[(1.022 \times 48) + (0.072 \times 82) + (1.461 \times 58) + (0.104 \times 55)] / 2.660$



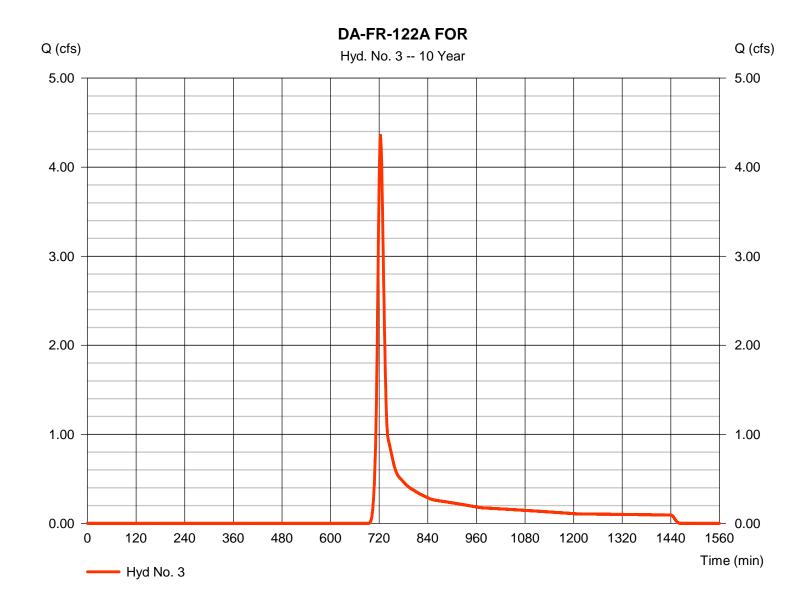
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-122A FOR

Hydrograph type = SCS Runoff Peak discharge = 4.357 cfsStorm frequency = 10 yrsTime to peak = 723 min Time interval = 1 minHyd. volume = 13,021 cuftDrainage area Curve number = 2.660 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 15.00 min = TR55 Total precip. = 5.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)								
(Yrs)	В	D	E	(N/A)					
1	0.0000	0.0000	0.0000						
2	69.8703	13.1000	0.8658						
3	0.0000	0.0000	0.0000						
5	79.2597	14.6000	0.8369						
10	88.2351	15.5000	0.8279						
25	102.6072	16.5000	0.8217						
50	114.8193	17.2000	0.8199						
100	127.1596	17.8000	0.8186						

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return	Intensity Values (in/hr)											
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

ling\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.p

	Rainfall Precipitation Table (in)									
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr		
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00		
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		



ENERGY BALANCE METHOD

		ts

	1-Y	r Event
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)
Pre-Developed Condition	1.441	8556
Developed Condition	1.150	7717
Pre-Developed (Forest) Condition	0.633	7622

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.8

<u>Calculations:</u>	Check #1:	$Q_{developed} \le IF \times \{(Q_{pre-developed} \times RV_{pre-developed}) / RV_{developed}\}$ >	1.150	≤ OK	1.278
	Check #2:	Q _{developed} ≤ Q _{pre-developed}	1.150	≤ OK	1.441
	Check #3:	$Q_{\text{developed } shall \ not}$ be required to be $\leq (Q_{\text{forest}} \times RV_{\text{forest}}) / RV_{\text{developed}} \longrightarrow$	1.150	shall not be required to be ≤	0.625

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	·
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 - Manning's n Values for Open Channel Flow

nanı	annel Type		Manning n			
			Min.	Normal	Max	
1.	Exc	avated or Dredged Channels¹				
	a.	Earth, Straight, and Uniform:				
		Clean, recently completed	0.016	0.018	0.020	
		Clean, after weathering	0.018	0.022	0.02	
		Gravel, uniform section, clean	0.022	0.025	0.03	
		With short grass, few weeds	0.022	0.027	0.03	
	b.	Earth Winding and Sluggish:				
		No vegetation	0.023	0.025	0.030	
		Grass, some weeds	0.025	0.030	0.03	
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040	
		Earth bottom and rubble sides	0.028	0.030	0.03	
		Stony bottom and weedy banks	0.025	0.035	0.04	
		Cobble bottom and clean sides	0.030	0.040	0.050	
	c.	Dragline-Excavated or Dredged:		•		
		No vegetation	0.025	0.028	0.03	
		Light brush on banks	0.035	0.050	0.06	
	d.	Rock Cuts:				
		Smooth and uniform	0.025	0.035	0.040	
		Jagged and irregular	0.035	0.040	0.05	
	e.	Channels not Maintained, Weeds and Brush Uncut:				
		Dense weeds, high as flow depth	0.050	0.080	0.120	
		Clean bottom, brush on sides	0.040	0.050	0.080	
		Same as above, highest stage of flow	0.045	0.070	0.110	
		Dense brush, high stage	0.080	0.100	0.140	
2.	Mai	n Channels²				
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	<mark>0.030</mark>	0.03	
	b.	Same as above, but more stones and weeds	0.030	0.035	0.04	
	C.	Clean, winding, some pools and shoals	0.033	0.040	0.04	
	d.	Same as above, but some weeds and stones	0.035	0.045	0.05	
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.05	
	f.	Same as (d) with more stones	0.045	0.050	0.06	
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080	
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150	

Notes

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs.

Sources:

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

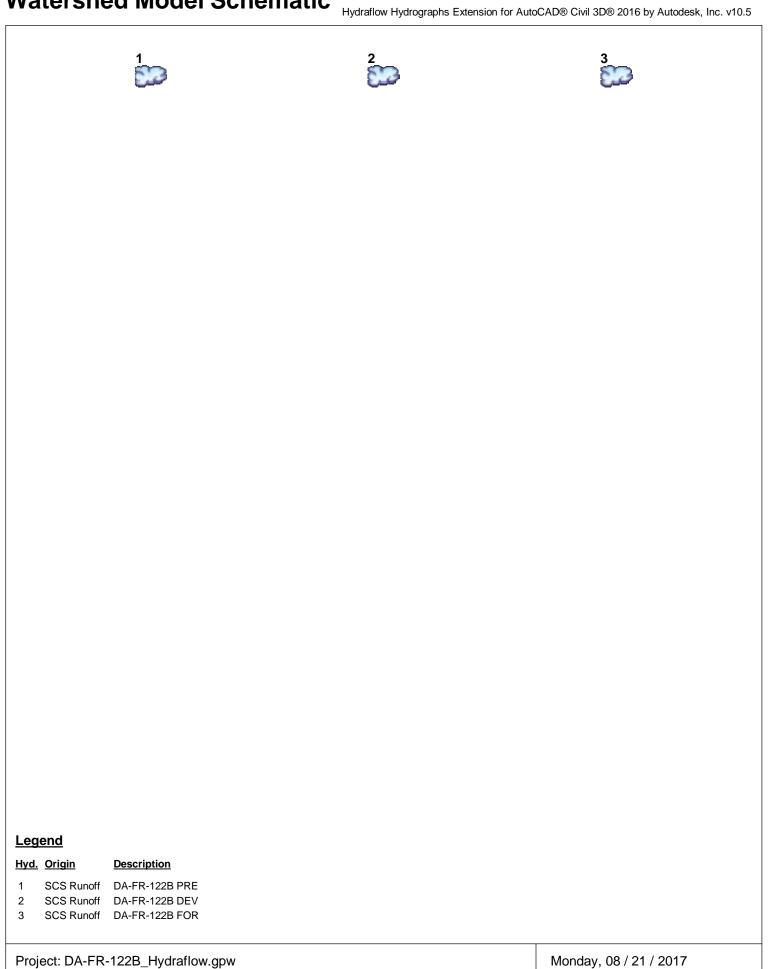
-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-122B PRE	
TR-55 Tc Worksheet	
Hydrograph No. 2, SCS Runoff, DA-FR-122B DEV	
TR-55 Tc Worksheet	7
Hydrograph No. 3, SCS Runoff, DA-FR-122B FOR	
TR-55 Tc Worksheet	9
2 - Year	
Summary Report	. 10
Hydrograph Reports	. 11
Hydrograph No. 1, SCS Runoff, DA-FR-122B PRE	. 11
Hydrograph No. 2, SCS Runoff, DA-FR-122B DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-122B FOR	
10 - Year	
Summary Report	
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-122B PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-122B DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-122B FOR	. 17
IDE Penort	1Ω



Hydrograph Return Period Recap Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

TVDE	Inflow	Peak Outflow (cfs)								Hydrograph Description	
type (origin)		hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
SCS Runoff		1.441	2.692			12.38				DA-FR-122B PRE	
SCS Runoff		1.150	2.283			11.54				DA-FR-122B DEV	
SCS Runoff		0.633	1.159			5.784				DA-FR-122B FOR	
•	SCS Runoff	SCS Runoff SCS Runoff	SCS Runoff 1.441 SCS Runoff 1.150	SCS Runoff 1.441 2.692 SCS Runoff 1.150 2.283	SCS Runoff 1.441 2.692 SCS Runoff 1.150 2.283	SCS Runoff 1.441 2.692 SCS Runoff 1.150 2.283	SCS Runoff 1.441 2.692 12.38 SCS Runoff 1.150 2.283 11.54	SCS Runoff 1.441 2.692 12.38 SCS Runoff 1.150 2.283 11.54	SCS Runoff 1.441 2.692 12.38	SCS Runoff 1.441 2.692 12.38 SCS Runoff 1.150 2.283 11.54	

Proj. file: DA-FR-122B_Hydraflow.gpw

Monday, 08 / 21 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	1.441	1	727	8,556				DA-FR-122B PRE
2	SCS Runoff	1.150	1	727	7,717				DA-FR-122B DEV
DA	-FR-122B_Hy	draflow.g	lpw		Return F	Return Period: 1 Year			3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

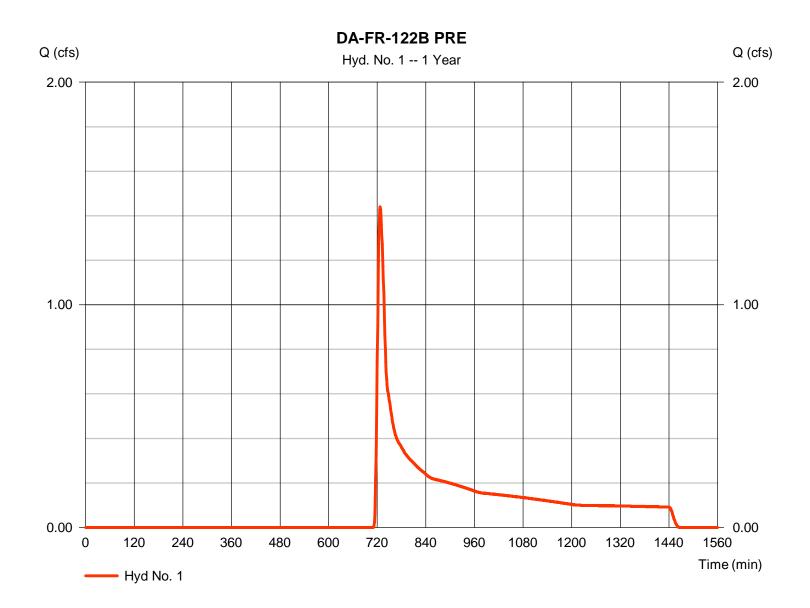
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-122B PRE

= SCS Runoff Hydrograph type Peak discharge = 1.441 cfsStorm frequency Time to peak = 727 min = 1 yrsTime interval = 1 minHyd. volume = 8,556 cuftDrainage area Curve number = 7.470 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.50 min Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.140 \times 82) + (0.917 \times 58) + (6.412 \times 55)] / 7.470$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1

DA-FR-122B PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 6.89 = 12.18	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	=	12.18
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 332.22 = 11.77 = Unpaved =5.54	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.00	+	0.00	+	0.00	=	1.00
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.11 = 14.58 = 5.94 = 0.030 =3.32		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})472.6		0.0		0.0		
Travel Time (min)	= 2.37	+	0.00	+	0.00	=	2.37
Total Travel Time, Tc							15.50 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

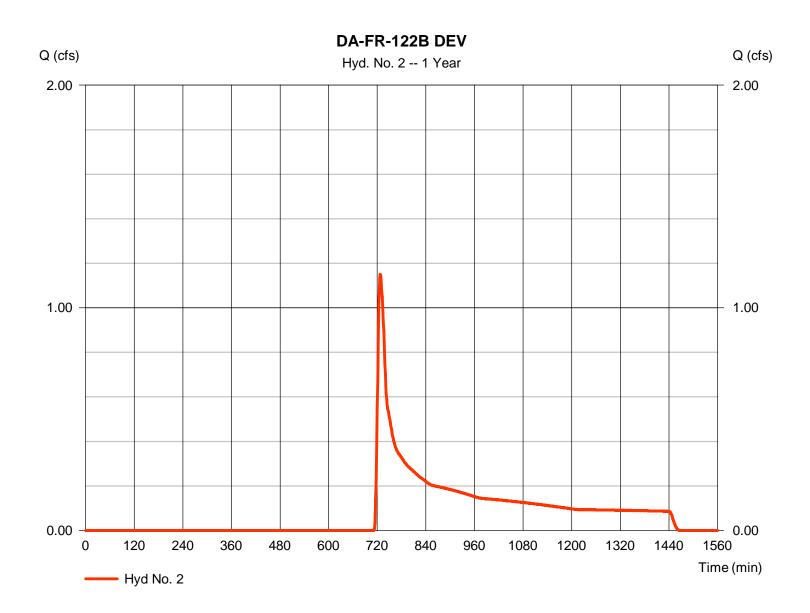
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-122B DEV

= SCS Runoff Hydrograph type Peak discharge = 1.150 cfsStorm frequency Time to peak = 727 min = 1 yrsTime interval = 1 minHyd. volume = 7,717 cuftDrainage area Curve number = 7.470 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.50 min Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(1.147 \times 48) + (0.140 \times 82) + (1.707 \times 58) + (4.475 \times 55)] / 7.470$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2

DA-FR-122B DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 6.89 = 12.18	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	=	12.18
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 332.22 = 11.77 = Unpaved =5.54	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.00	+	0.00	+	0.00	=	1.00
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.11 = 14.58 = 5.94 = 0.030 =3.32		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})472.6		0.0		0.0		
Travel Time (min)	= 2.37	+	0.00	+	0.00	=	2.37
Total Travel Time, Tc							15.50 min

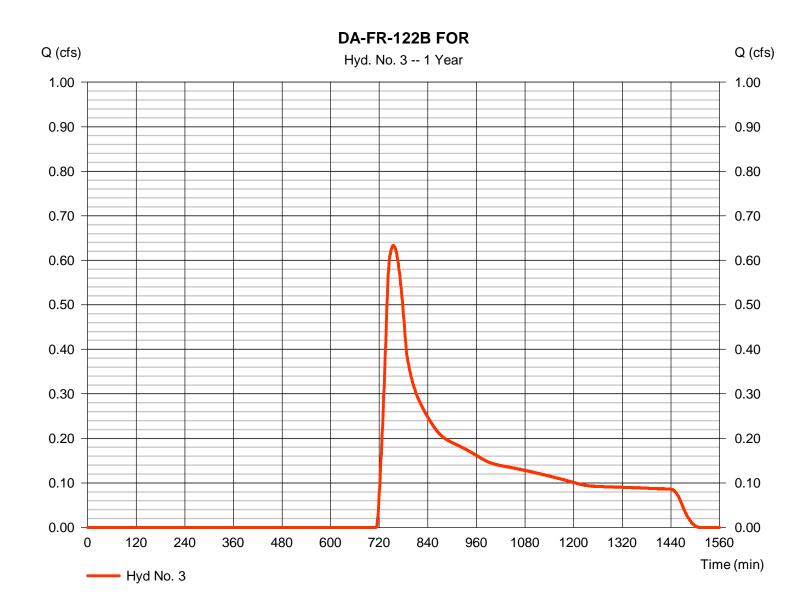
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-122B FOR

Hydrograph type = SCS Runoff Peak discharge = 0.633 cfsStorm frequency Time to peak = 755 min = 1 yrsTime interval = 1 min Hyd. volume = 7,622 cuftDrainage area Curve number = 7.470 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 44.80 \, \text{min}$ Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3

DA-FR-122B FOR

<u>Description</u>	<u>A</u>	<u>B</u>	<u>C</u>	<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 6.89 = 12.18 +	0.011 0.0 0.00 0.00 0.00	0.011 0.0 0.00 0.00 0.00 =	12.18
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 332.22 = 11.77 = Unpaved =5.54	0.00 0.00 Paved 0.00	0.00 0.00 Paved 0.00	
Travel Time (min)	= 1.00 +	0.00 +	0.00 =	1.00
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%)	= 2.11 = 14.58 = 5.94	0.00 0.00	0.00 0.00	
Manning's n-value Velocity (ft/s)	= 0.400 =0.25	0.00 0.015 0.00	0.00 0.015 0.00	
•	= 0.400	0.015	0.015	
Velocity (ft/s)	= 0.400 =0.25	0.015 0.00 0.0	0.015	31.65

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	2.692	1	726	12,455				DA-FR-122B PRE
2	SCS Runoff	2.283	1	726	11,410				DA-FR-122B DEV
DA	-FR-122B_Hy	draflow.g	lpw		Return F	Return Period: 2 Year			3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

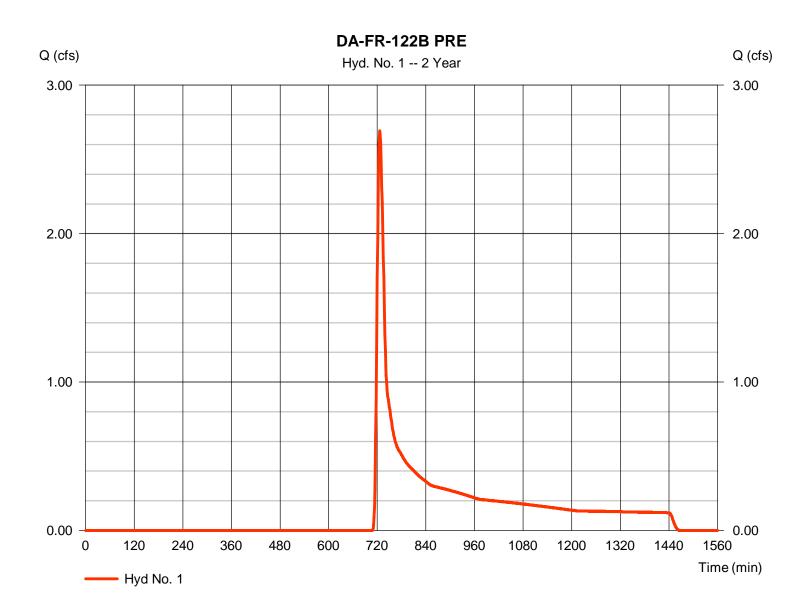
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-122B PRE

= SCS Runoff Hydrograph type Peak discharge = 2.692 cfsStorm frequency Time to peak = 726 min = 2 yrsTime interval = 1 min Hyd. volume = 12.455 cuft Curve number Drainage area = 7.470 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.50 min Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.140 \times 82) + (0.917 \times 58) + (6.412 \times 55)] / 7.470$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

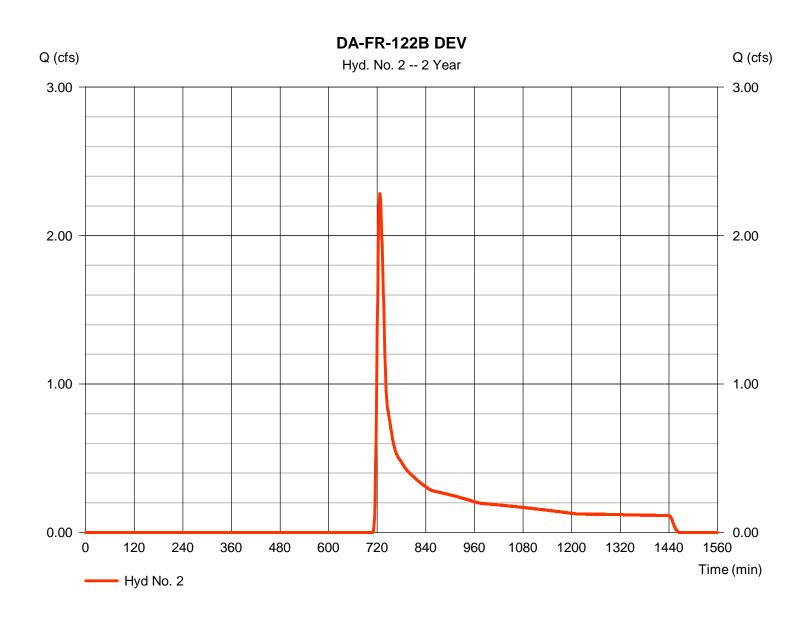
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-122B DEV

= SCS Runoff Hydrograph type Peak discharge = 2.283 cfs= 726 min Storm frequency Time to peak = 2 yrsTime interval = 1 minHyd. volume = 11.410 cuftCurve number Drainage area = 7.470 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.50 min Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(1.147 \times 48) + (0.140 \times 82) + (1.707 \times 58) + (4.475 \times 55)] / 7.470$



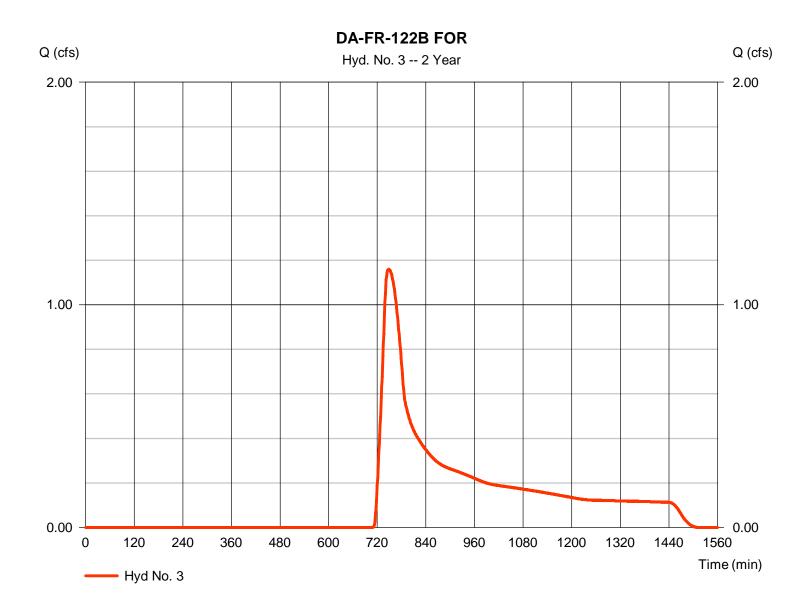
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-122B FOR

= SCS Runoff = 1.159 cfsHydrograph type Peak discharge Storm frequency Time to peak = 749 min = 2 yrsTime interval = 1 min Hyd. volume = 11,269 cuftDrainage area = 7.470 acCurve number = 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 44.80 \, \text{min}$ Total precip. = Type II Distribution = 3.70 inStorm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	12.38	1	724	39,044				DA-FR-122B PRE
2	SCS Runoff	11.54	1	724	37,023				DA-FR-122B DEV
DA	-FR-122B_H	/draflow.g	jpw	1	Return F	Return Period: 10 Year			3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

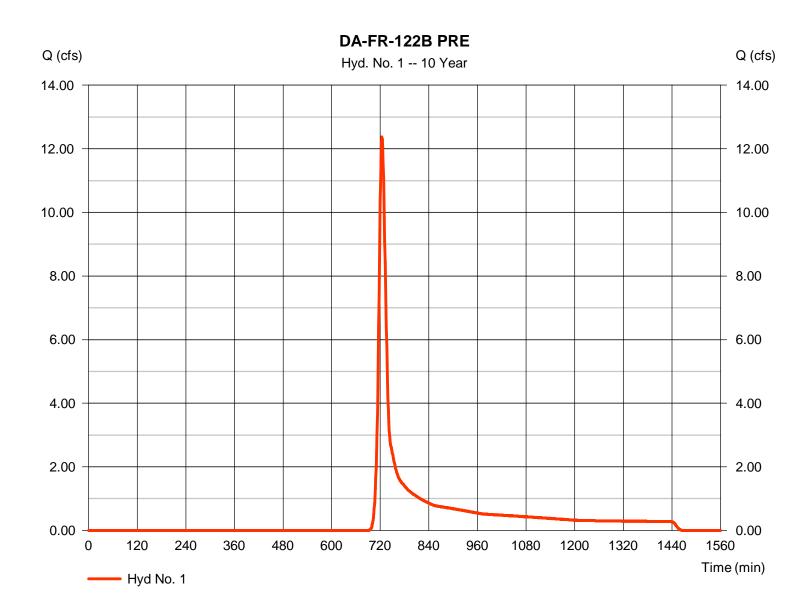
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-122B PRE

= SCS Runoff Hydrograph type Peak discharge = 12.38 cfsStorm frequency Time to peak = 724 min = 10 yrsTime interval = 1 min Hyd. volume = 39.044 cuftDrainage area Curve number = 7.470 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.50 min Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.140 \times 82) + (0.917 \times 58) + (6.412 \times 55)] / 7.470$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

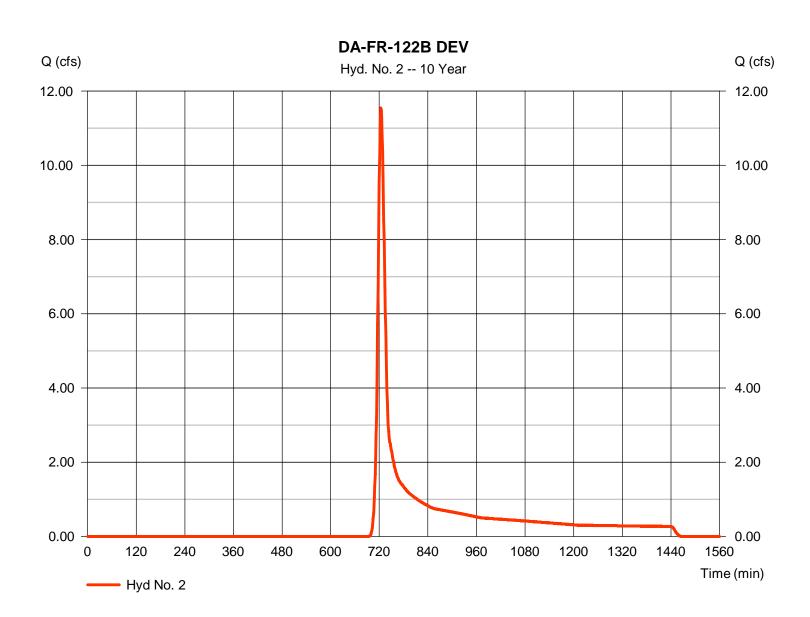
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-122B DEV

= SCS Runoff Hydrograph type Peak discharge = 11.54 cfsStorm frequency Time to peak = 724 min = 10 yrsTime interval = 1 min Hyd. volume = 37.023 cuftDrainage area Curve number = 7.470 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.50 min Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(1.147 \times 48) + (0.140 \times 82) + (1.707 \times 58) + (4.475 \times 55)] / 7.470$



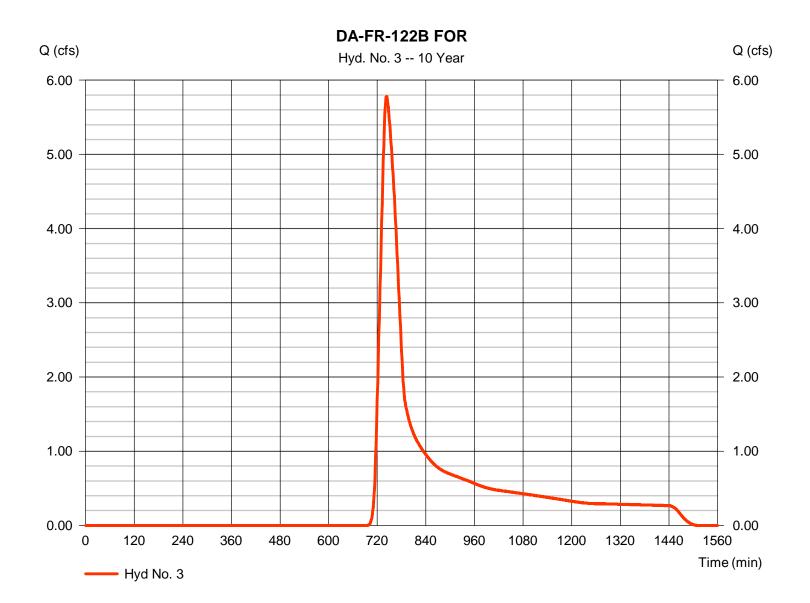
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-122B FOR

= SCS Runoff Hydrograph type Peak discharge = 5.784 cfsStorm frequency Time to peak = 743 min = 10 yrsTime interval = 1 min Hyd. volume = 36,566 cuftDrainage area Curve number = 7.470 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 44.80 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Return Period	Intensity-Du	Intensity-Duration-Frequency Equation Coefficients (FHA)									
(Yrs)	В	D	E	(N/A)							
1	0.0000	0.0000	0.0000								
2	69.8703	13.1000	0.8658								
3	0.0000	0.0000	0.0000								
5	79.2597	14.6000	0.8369								
10	88.2351	15.5000	0.8279								
25	102.6072	16.5000	0.8217								
50	114.8193	17.2000	0.8199								
100	127.1596	17.8000	0.8186								

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return Period (Yrs)		Intensity Values (in/hr)													
	5 min	10	15	20	25	30	35	40	45	50	55	60			
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70			
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15			
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46			
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91			
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25			
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60			

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc|

		Rainfall Precipitation Table (in)										
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr				
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00				
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				



ENERGY BALANCE METHOD

In		

	1-Y	r Event
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)
Pre-Developed Condition	0.094	449
Developed Condition	0.054	358
Pre-Developed (Forest) Condition	0.094	449

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.

Calculations:	Check #1:	$Q_{developed} \leq IF \times \left[\left(Q_{pre-developed} \times RV_{pre-developed} \right) / RV_{developed} \right]$	0.054	≤ N/A - See Check #3	0.106
	Check #2:	$Q_{developed} \le Q_{pre-developed}$	0.054	≤ N/A - See Check #3	0.094
	Check #3:	$Q_{developed} \frac{shall\ not}{shall\ not}$ be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed} \longrightarrow$	0.054	<u>shall not</u> be required to be ≤	0.118

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	·
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

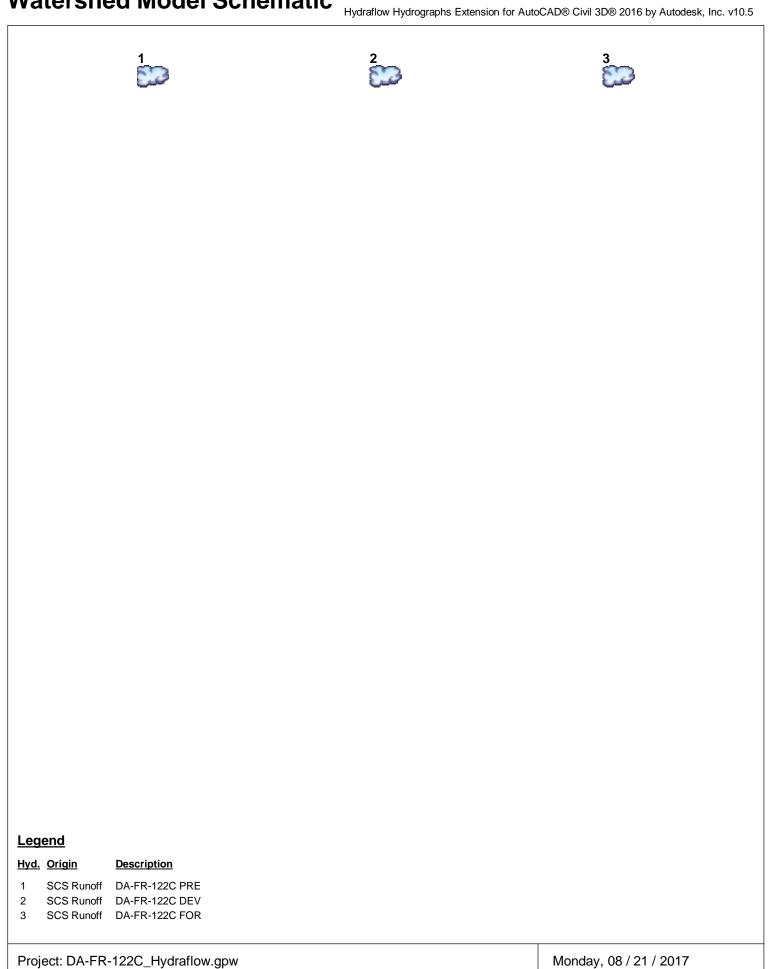
-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	4
Hydrograph ReportsHydrograph No. 1, SCS Runoff, DA-FR-122C PRE	4
TR-55 Tc Worksheet	5
Hydrograph No. 2, SCS Runoff, DA-FR-122C DEV	6
TR-55 Tc Worksheet	7
Hydrograph No. 3, SCS Runoff, DA-FR-122C FOR	8
TR-55 Tc Worksheet	9
2 - Year	
Summary Report	10
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-122C PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-122C DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-122C FOR	
40 Voor	
10 - Year	
Summary Report	
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-122C PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-122C DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-122C FOR	. 17
IDF Report	18



Hydrograph Return Period Recap Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

		Inflow				Hydrograph Description					
0.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
I	SCS Runoff		0.094	0.187			0.889				DA-FR-122C PRE
2	SCS Runoff		0.054	0.128			0.770				DA-FR-122C DEV
3	SCS Runoff		0.094	0.187			0.889				DA-FR-122C FOR

Proj. file: DA-FR-122C_Hydraflow.gpw

Monday, 08 / 21 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.094	1	723	449				DA-FR-122C PRE
2	SCS Runoff	0.054	1	723	358				DA-FR-122C DEV
2									
DA	-FR-122C_H _y	/draflow.g	jpw		Return F	Period: 1 Ye	ear	Monday, 08	3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

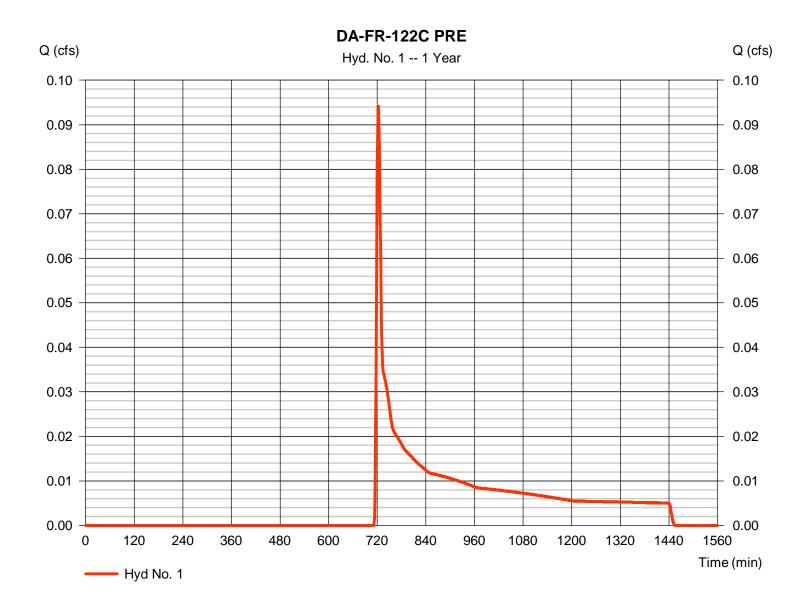
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-122C PRE

Hydrograph type = SCS Runoff Peak discharge = 0.094 cfsStorm frequency Time to peak = 723 min = 1 yrsTime interval = 1 min Hyd. volume = 449 cuft Drainage area Curve number = 0.440 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.90 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.437 \times 55) + (0.000 \times 100)] / 0.440$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1

DA-FR-122C PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 16.09		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 8.67	+	0.00	+	0.00	=	8.67
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 101.97 = 15.18 = Unpaved =6.29	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.27	+	0.00	+	0.00	=	0.27
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							8.90 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

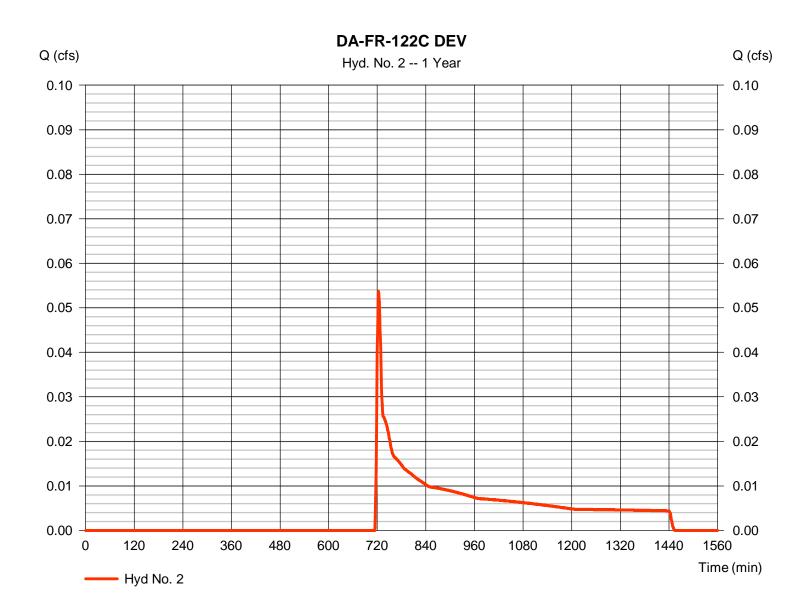
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-122C DEV

Hydrograph type = SCS Runoff Peak discharge = 0.054 cfsStorm frequency Time to peak = 723 min = 1 yrsTime interval = 1 min Hyd. volume = 358 cuft Drainage area Curve number = 0.440 ac= 53*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.90 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.189 \times 48) + (0.200 \times 58) + (0.000 \times 100) + (0.047 \times 55)] / 0.440$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2

DA-FR-122C DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 16.09		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 8.67	+	0.00	+	0.00	=	8.67
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 101.97 = 15.18 = Unpaved =6.29	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.27	+	0.00	+	0.00	=	0.27
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							8.90 min

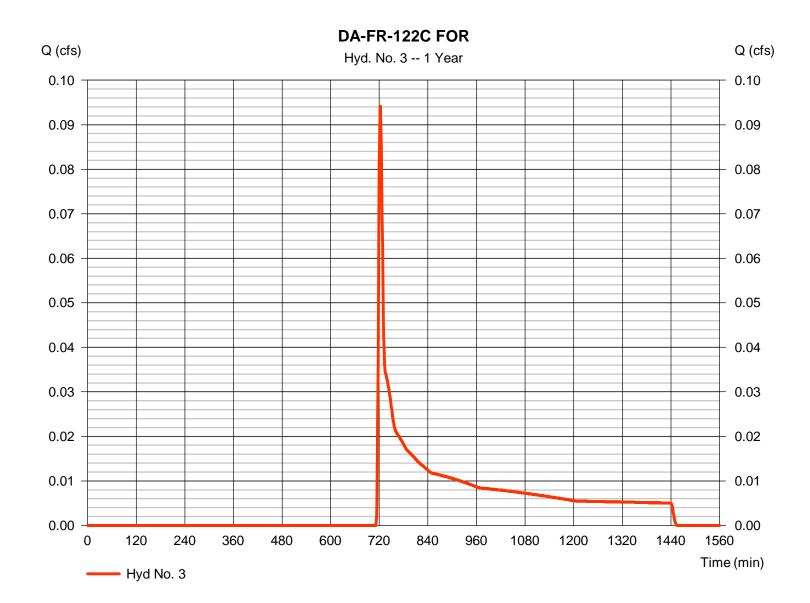
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-122C FOR

Hydrograph type = SCS Runoff Peak discharge = 0.094 cfsStorm frequency Time to peak = 723 min = 1 yrsTime interval = 1 min Hyd. volume = 449 cuft Drainage area Curve number = 0.440 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 8.90 \, \text{min}$ Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3

DA-FR-122C FOR

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 16.09		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 8.67	+	0.00	+	0.00	=	8.67
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 101.97 = 15.18 = Unpaved =6.29	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.27	+	0.00	+	0.00	=	0.27
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							8.90 min

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.187	1	722	664				DA-FR-122C PRE
2	SCS Runoff	0.128	1	722	549				DA-FR-122C DEV
DA	-FR-122C_H ₃	/draflow.g	jpw		Return F	Period: 2 Ye	ear	Monday, 08	3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

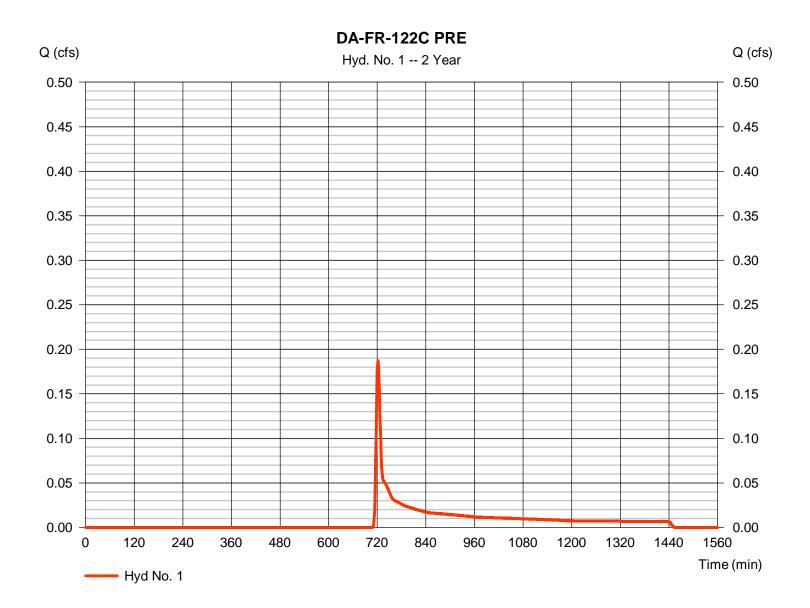
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-122C PRE

Hydrograph type = SCS Runoff Peak discharge = 0.187 cfsStorm frequency Time to peak = 722 min = 2 yrsTime interval = 1 min Hyd. volume = 664 cuft Drainage area Curve number = 0.440 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.90 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.437 \times 55) + (0.000 \times 100)] / 0.440$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

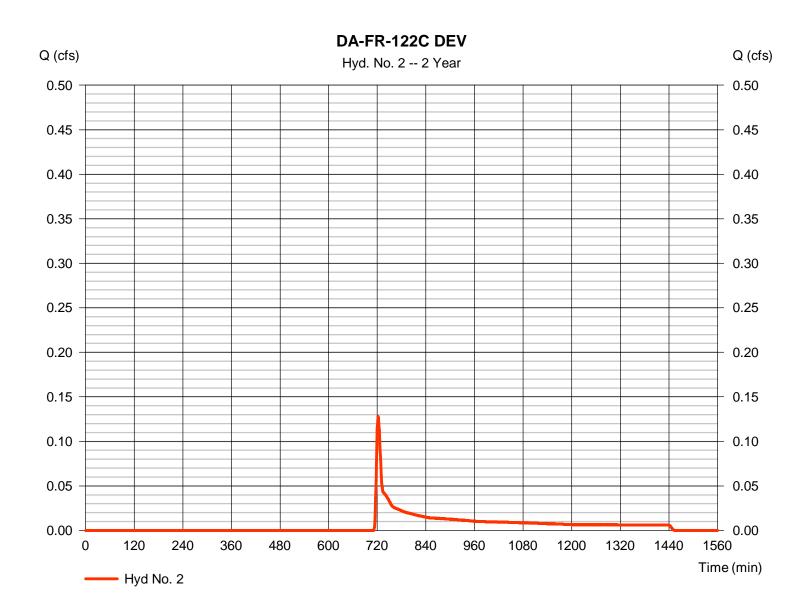
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-122C DEV

Hydrograph type = SCS Runoff Peak discharge = 0.128 cfsStorm frequency Time to peak = 722 min = 2 yrsTime interval = 1 min Hyd. volume = 549 cuft Drainage area Curve number = 0.440 ac= 53*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.90 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.189 \times 48) + (0.200 \times 58) + (0.000 \times 100) + (0.047 \times 55)] / 0.440$



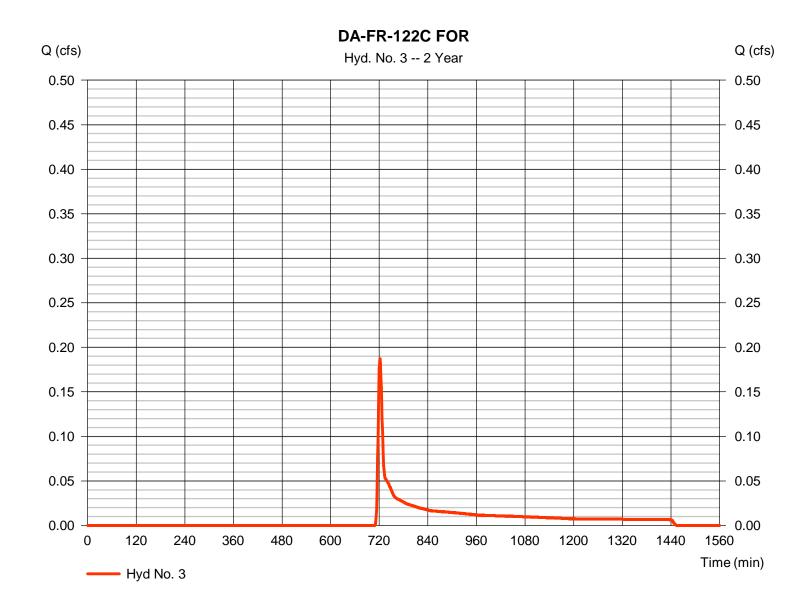
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-122C FOR

Hydrograph type = SCS Runoff Peak discharge = 0.187 cfsStorm frequency Time to peak = 722 min = 2 yrsTime interval = 1 min Hyd. volume = 664 cuft Drainage area Curve number = 0.440 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 8.90 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.889	1	720	2,154				DA-FR-122C PRE
2	SCS Runoff	0.770	1	720	1,925				DA-FR-122C DEV
	-FR-122C_H					Period: 10 \			3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

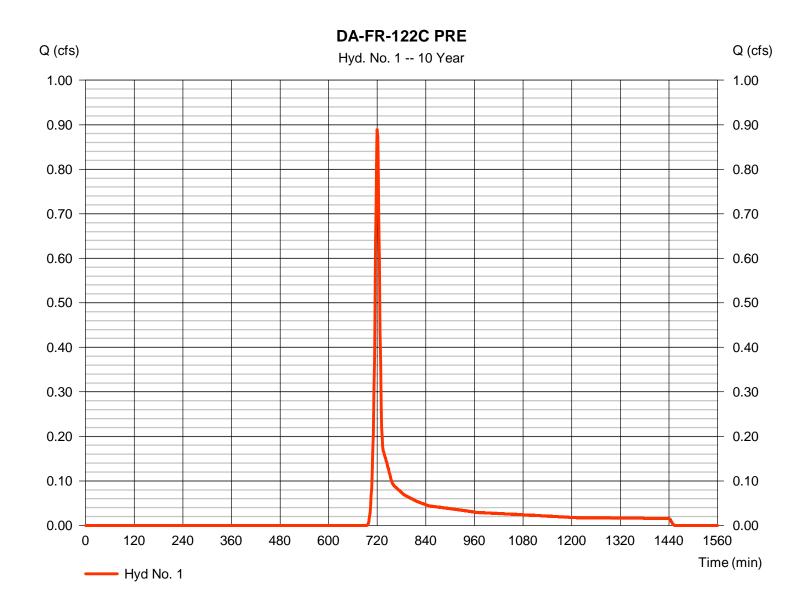
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-122C PRE

= SCS Runoff Hydrograph type Peak discharge = 0.889 cfsStorm frequency Time to peak = 720 min = 10 yrsTime interval = 1 min Hyd. volume = 2,154 cuftDrainage area Curve number = 0.440 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.90 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.437 \times 55) + (0.000 \times 100)] / 0.440$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

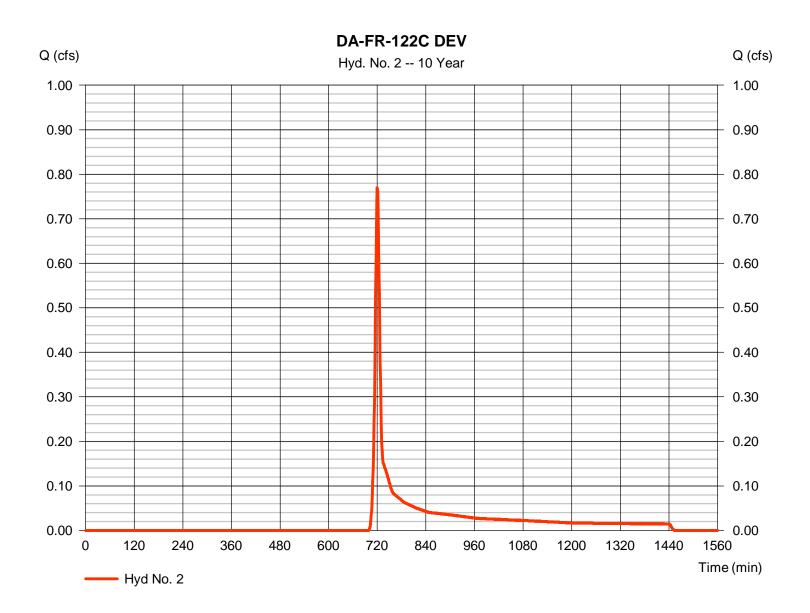
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-122C DEV

Hydrograph type = SCS Runoff Peak discharge = 0.770 cfsStorm frequency Time to peak = 10 yrs= 720 min Time interval = 1 min Hyd. volume = 1,925 cuftCurve number Drainage area = 0.440 ac= 53*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 8.90 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.189 \times 48) + (0.200 \times 58) + (0.000 \times 100) + (0.047 \times 55)] / 0.440$



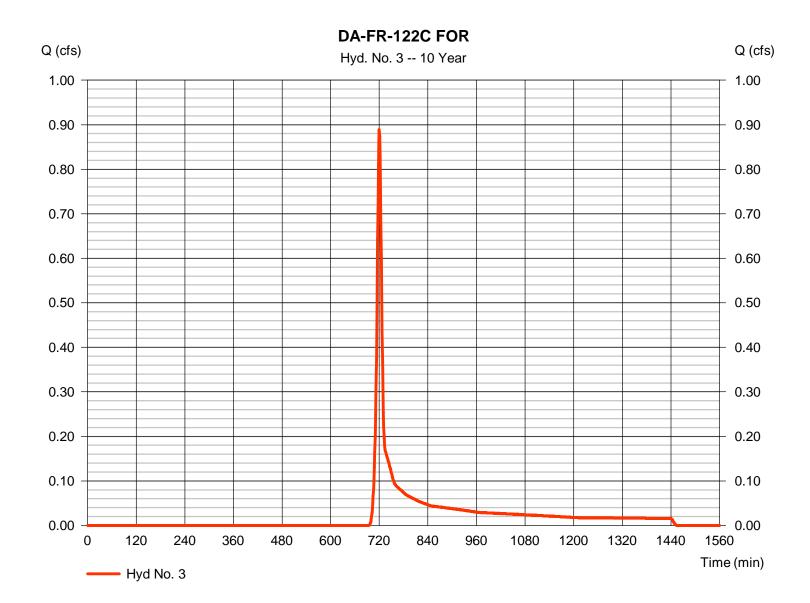
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-122C FOR

= SCS Runoff Hydrograph type Peak discharge = 0.889 cfsStorm frequency Time to peak = 720 min = 10 yrsTime interval = 1 min Hyd. volume = 2.154 cuftDrainage area Curve number = 0.440 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 8.90 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Return Period	Intensity-Du	ıration-Frequency Ed	quation Coefficients ((FHA)
(Yrs)	В	D	E	(N/A)
1	0.0000	0.0000	0.0000	
2	69.8703	13.1000	0.8658	
3	0.0000	0.0000	0.0000	
5	79.2597	14.6000	0.8369	
10	88.2351	15.5000	0.8279	
25	102.6072	16.5000	0.8217	
50	114.8193	17.2000	0.8199	
100	127.1596	17.8000	0.8186	

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return		Intensity Values (in/hr)													
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60			
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70			
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15			
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46			
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91			
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25			
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60			

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc|

		R	Rainfall F	Precipitat	tion Tab	le (in)		
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00



ENERGY BALANCE METHOD

	1-Y	r Event
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)
Pre-Developed Condition	0.386	1819
Developed Condition	0.217	1422
Pre-Developed (Forest) Condition	0.309	1641

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.8

<u>Calculations:</u>	Check #1:	$Q_{developed} \le IF \times [(Q_{pre-developed} \times RV_{pre-developed}) / RV_{developed}]$ >	0.217	≤ OK	0.395
	Check #2:	$Q_{developed} \le Q_{pre-developed}$	0.217	≤ OK	0.386
	Check #3:	$Q_{developed} \underline{shall not}$ be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed}$ >	0.217	shall not be required to be ≤	0.357

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n								
Grass:									
Average Grass Cover	0.40								
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40								
Light Turf	0.20								
Dense Turf	0.17 – 0.80								
Dense Grass	0.17 – 0.30								
Bermuda Grass	0.30 – 0.48								
Dense Shrubbery and Forest Litter	0.40								
Natural:									
Short Grass Prairie	0.10 – 0.20								
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40								
Sparse Vegetation	0.05 – 0.13								
Oak Grasslands, Open Grasslands	0.60								
Dense Cover of Trees and Bushes	0.80								
Rangeland:									
Typical	0.13								
No Debris Cover	0.09 – 0.34								
20% Debris Cover	0.05 – 0.25								
Woods:									
Light Underbrush	0.40								
Dense Underbrush	0.80								
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40								

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 – Manning's n Values for Open Channel Flow

Channel Type				Manning n	
			Min.	Normal	Max
1.	Exc	avated or Dredged Channels¹			
	a.	Earth, Straight, and Uniform:			
		Clean, recently completed	0.016	0.018	0.020
		Clean, after weathering	0.018	0.022	0.02
		Gravel, uniform section, clean	0.022	0.025	0.030
		With short grass, few weeds	0.022	0.027	0.03
	b.	Earth Winding and Sluggish:			
		No vegetation	0.023	0.025	0.030
		Grass, some weeds	0.025	0.030	0.03
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040
		Earth bottom and rubble sides	0.028	0.030	0.03
		Stony bottom and weedy banks	0.025	0.035	0.040
		Cobble bottom and clean sides	0.030	<mark>0.040</mark>	0.050
	c.	Dragline-Excavated or Dredged:		•	
		No vegetation	0.025	0.028	0.03
		Light brush on banks	0.035	0.050	0.06
	d.	Rock Cuts:			
		Smooth and uniform	0.025	0.035	0.040
		Jagged and irregular	0.035	0.040	0.050
	e.	Channels not Maintained, Weeds and Brush Uncut:			
		Dense weeds, high as flow depth	0.050	0.080	0.120
		Clean bottom, brush on sides	0.040	0.050	0.080
		Same as above, highest stage of flow	0.045	0.070	0.110
		Dense brush, high stage	0.080	0.100	0.140
2.	Mai	n Channels²			
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.03
	b.	Same as above, but more stones and weeds	0.030	0.035	0.04
	c.	Clean, winding, some pools and shoals	0.033	0.040	0.04
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.05
	f.	Same as (d) with more stones	0.045	0.050	0.06
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

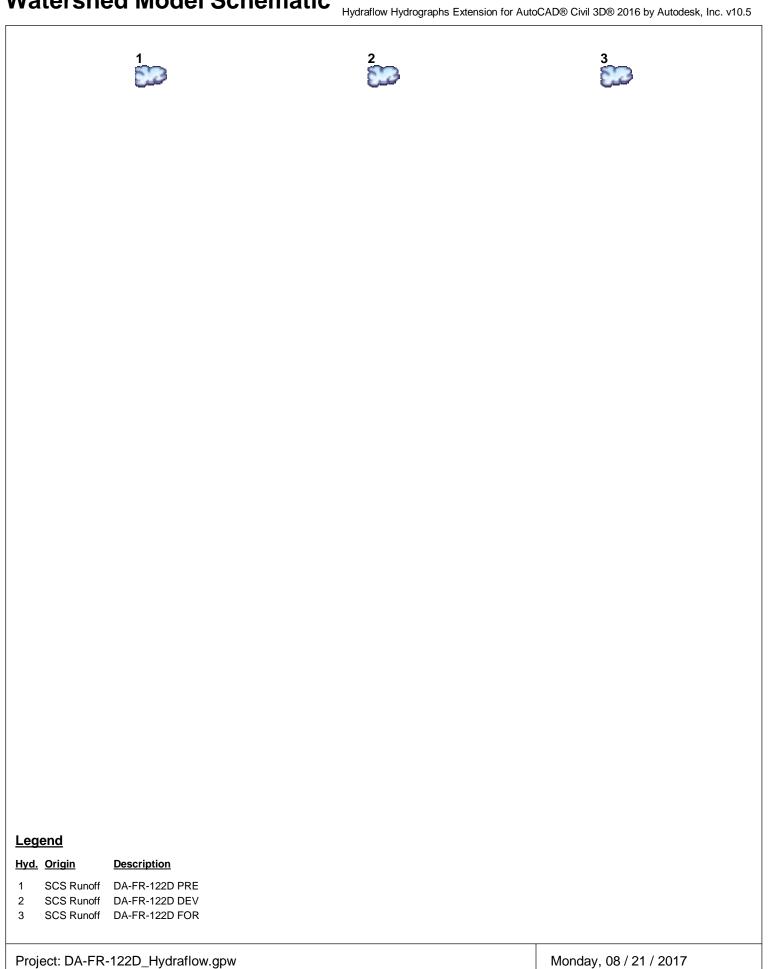
-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	. 3
Hydrograph Reports	. 4
Hydrograph No. 1, SCS Runoff, DA-FR-122D PRE	. 4
TR-55 Tc Worksheet	
Hydrograph No. 2, SCS Runoff, DA-FR-122D DEV	. 6
TR-55 Tc Worksheet	
Hydrograph No. 3, SCS Runoff, DA-FR-122D FOR	
TR-55 Tc Worksheet	. 9
2 - Year	
	40
Summary Report	10
Hydrograph Reports	11
Hydrograph No. 2, SCS Runoff, DA-FR-122D DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-122D FOR	
Trydrograph No. 5, 500 Ruhon, DA-1 R-122D 1 OR	13
10 - Year	
Summary Report	14
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-122D PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-122D DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-122D FOR	17
IDE Papart	10



Hydrograph Return Period Recap

	Hydrograph	Inflow				Hydrograph					
No.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		0.386	0.722			3.198				DA-FR-122D PRE
2	SCS Runoff		0.217	0.466			2.557				DA-FR-122D DEV
3	SCS Runoff		0.309	0.615			2.991				DA-FR-122D FOR

Proj. file: DA-FR-122D_Hydraflow.gpw

Monday, 08 / 21 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description		
1	SCS Runoff	0.386	1	723	1,819				DA-FR-122D PRE		
2	SCS Runoff	0.217	1	725	1,422				DA-FR-122D DEV		
DA	DA-FR-122D_Hydraflow.gpw				Return F	Period: 1 Ye	ear	Monday, 08 / 21 / 2017			

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

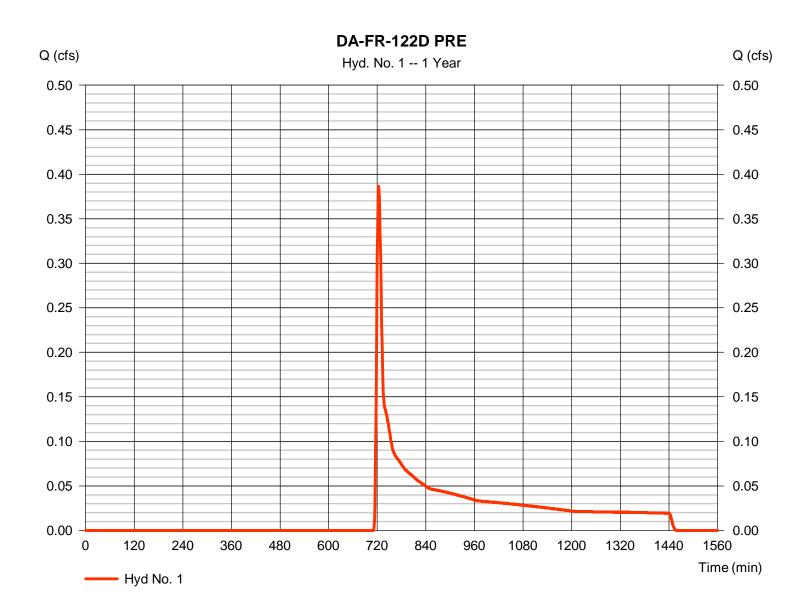
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-122D PRE

Hydrograph type = SCS Runoff Peak discharge = 0.386 cfsStorm frequency Time to peak = 723 min = 1 yrsTime interval = 1 min Hyd. volume = 1.819 cuftDrainage area Curve number = 1.580 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 11.50 min Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.093 x 58) + (0.014 x 100) + (1.473 x 55)] / 1.580



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1

DA-FR-122D PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 9.41		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 10.75	+	0.00	+	0.00	=	10.75
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 267.64 = 12.69 = Unpaved =5.75	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.78	+	0.00	+	0.00	=	0.78
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							11.50 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

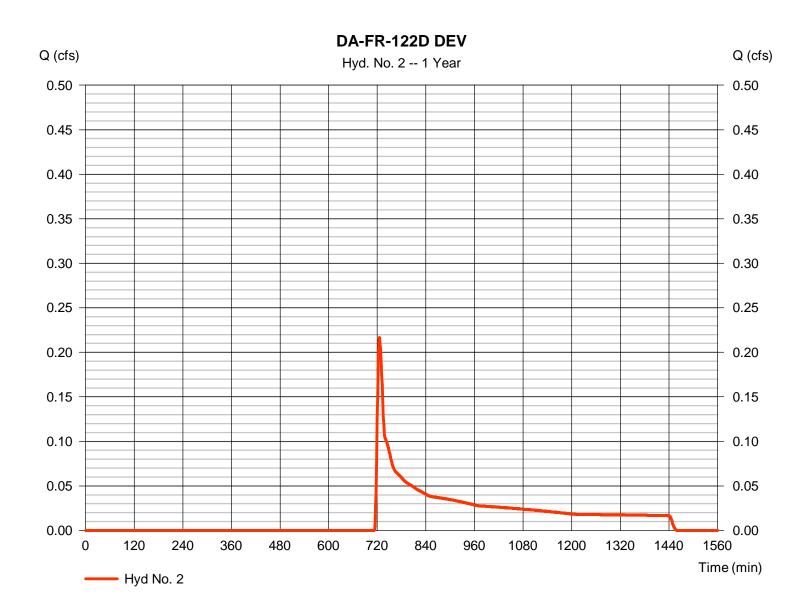
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-122D DEV

Hydrograph type = SCS Runoff Peak discharge = 0.217 cfsStorm frequency Time to peak = 1 yrs $= 725 \min$ Time interval = 1 min Hyd. volume = 1,422 cuftDrainage area Curve number = 1.580 ac= 54*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 12.00 min Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.605 \times 48) + (0.688 \times 58) + (0.014 \times 100) + (0.274 \times 55)] / 1.580$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2

DA-FR-122D DEV

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 9.41 = 10.75	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	_	10.75
, ,	- 10.75	т	0.00	т	0.00	_	10.75
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 219.47 = 6.34 = Unpave =4.06	d	88.01 13.62 Unpave 5.95	ed	0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.90	+	0.25	+	0.00	=	1.15
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})31.6		0.0		0.0		
Travel Time (min)	= 0.11	+	0.00	+	0.00	=	0.11
Total Travel Time, Tc							12.00 min

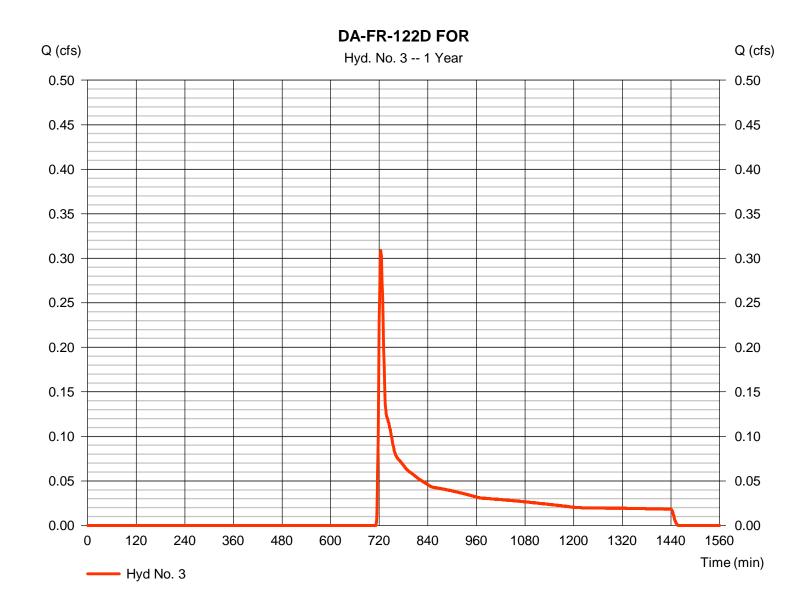
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-122D FOR

Hydrograph type = SCS Runoff Peak discharge = 0.309 cfsStorm frequency Time to peak = 724 min = 1 yrsTime interval = 1 min Hyd. volume = 1.641 cuftDrainage area Curve number = 1.580 ac= 55 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 11.50 min Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



 $\label{thm:condition} \mbox{Hydrographs Extension for AutoCAD} \mbox{\@none} \mbox{\$

Hyd. No. 3

DA-FR-122D FOR

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 9.41		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 10.75	+	0.00	+	0.00	=	10.75
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 267.64 = 12.69 = Unpave =5.75	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.78	+	0.00	+	0.00	=	0.78
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s) Flow length (ft)	= 0.00 = 0.00 = 0.015 = 0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015 0.00		
Flow length (it)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							11.50 min

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

lyd. lo.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.722	1	723	2,648				DA-FR-122D PRE
2	SCS Runoff	0.466	1	724	2,139				DA-FR-122D DEV
2 3									
DA	 -FR-122D_H	_ ydraflow.g	gpw		Return F	eriod: 2 Ye	i ear	Monday, 08	 3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

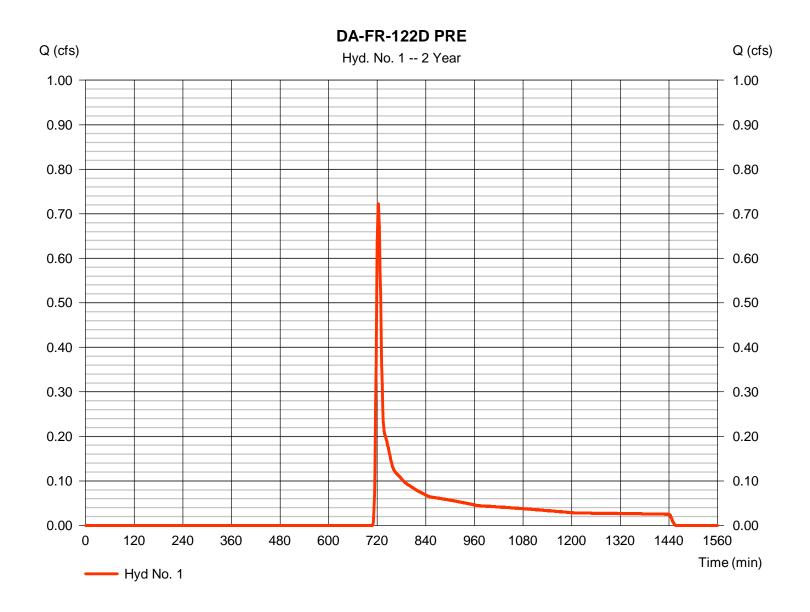
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-122D PRE

Hydrograph type = SCS Runoff Peak discharge = 0.722 cfsStorm frequency Time to peak = 723 min = 2 yrsTime interval = 1 min Hyd. volume = 2.648 cuftDrainage area Curve number = 1.580 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 11.50 min Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = [(0.093 x 58) + (0.014 x 100) + (1.473 x 55)] / 1.580



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

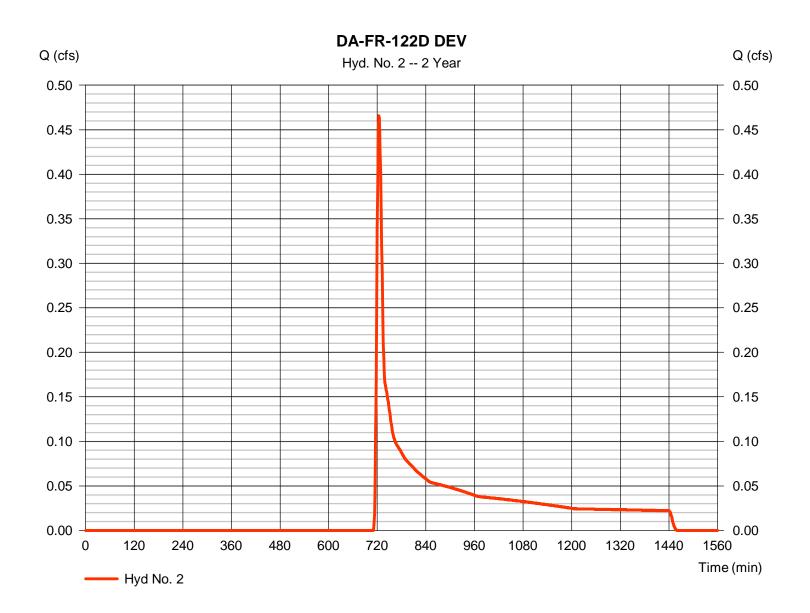
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-122D DEV

Hydrograph type = SCS Runoff Peak discharge = 0.466 cfsStorm frequency Time to peak = 724 min = 2 yrsTime interval = 1 min Hyd. volume = 2,139 cuftDrainage area Curve number = 1.580 ac= 54*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 12.00 min Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.605 \times 48) + (0.688 \times 58) + (0.014 \times 100) + (0.274 \times 55)] / 1.580$



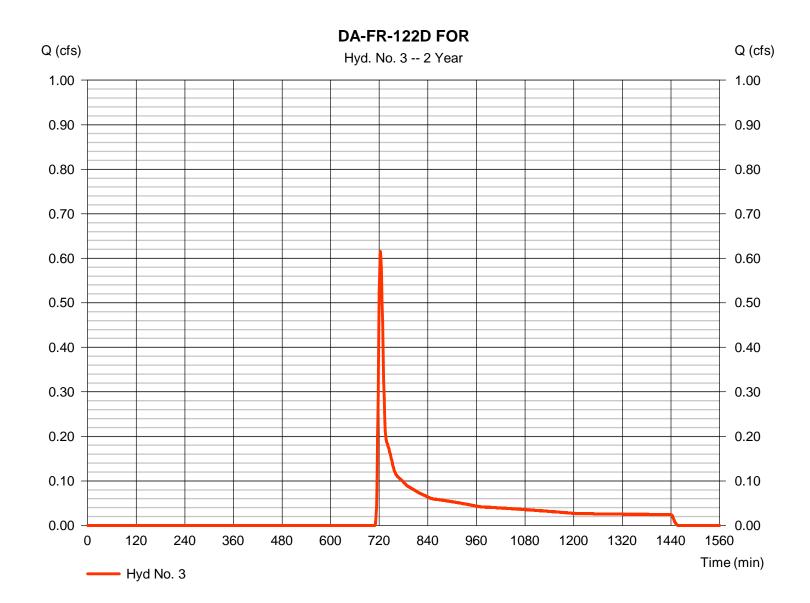
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-122D FOR

Hydrograph type = SCS Runoff Peak discharge = 0.615 cfsStorm frequency Time to peak = 723 min = 2 yrsTime interval = 1 min Hyd. volume = 2.426 cuftDrainage area Curve number = 1.580 ac= 55 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 11.50 min Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	3.198	1	721	8,302				DA-FR-122D PRE	
2	SCS Runoff	2.557	1	722	7,205				DA-FR-122D DEV	
2										
DA	-FR-122D_H _y	/draflow.g	jpw		Return Period: 10 Year			Monday, 08 / 21 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

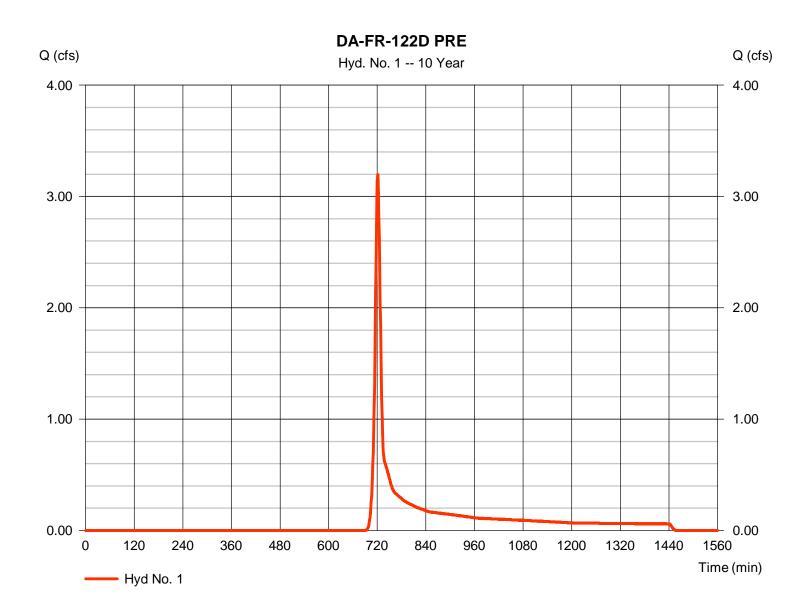
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-122D PRE

= SCS Runoff Hydrograph type Peak discharge = 3.198 cfsStorm frequency Time to peak = 721 min = 10 yrsTime interval = 1 min Hyd. volume = 8.302 cuftDrainage area Curve number = 1.580 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 11.50 min Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.093 x 58) + (0.014 x 100) + (1.473 x 55)] / 1.580



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

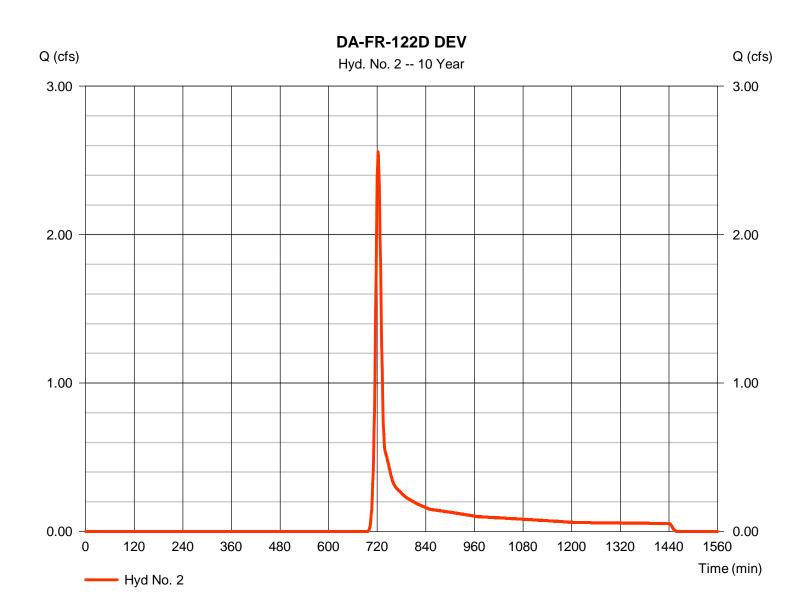
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-122D DEV

= SCS Runoff Hydrograph type Peak discharge = 2.557 cfsStorm frequency Time to peak = 722 min = 10 yrsTime interval = 1 min Hyd. volume = 7.205 cuftCurve number Drainage area = 1.580 ac= 54*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 12.00 min Total precip. = 5.70 inDistribution = Type II Storm duration Shape factor = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.605 \times 48) + (0.688 \times 58) + (0.014 \times 100) + (0.274 \times 55)] / 1.580$



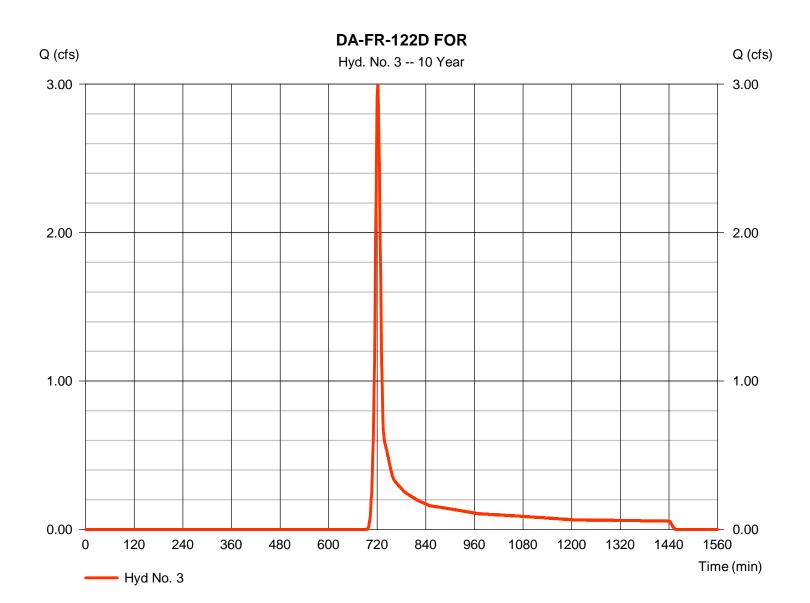
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-122D FOR

= SCS Runoff Hydrograph type Peak discharge = 2.991 cfsStorm frequency Time to peak = 721 min = 10 yrsTime interval = 1 min Hyd. volume = 7.872 cuftDrainage area Curve number = 1.580 ac= 55 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 11.50 min Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Return Period	Intensity-Du	Intensity-Duration-Frequency Equation Coefficients (FHA)										
(Yrs)	В	D	E	(N/A)								
1	0.0000	0.0000 0.0000 0.00										
2	69.8703	13.1000	0.8658									
3	0.0000	0.0000	0.0000									
5	79.2597	14.6000	0.8369									
10	88.2351	15.5000	0.8279									
25	102.6072	16.5000	0.8217									
50	114.8193	17.2000	0.8199									
100	127.1596	17.8000	0.8186									

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return		Intensity Values (in/hr)													
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60			
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70			
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15			
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46			
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91			
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25			
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60			

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc|

		Rainfall Precipitation Table (in)										
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr				
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00				
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				



ENERGY BALANCE METHOD

	1-Yr Event					
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)				
Pre-Developed Condition	2.894	10299				
Developed Condition	2.894	10299				
Pre-Developed (Forest) Condition	0.892	5520				

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF =

Calculations:	Check #1:	$Q_{\text{developed}} \leq \text{IF x [(}Q_{\text{pre-developed}} \times \text{RV}_{\text{pre-developed}}) / \text{RV}_{\text{developed}}] \qquad>$	2.894	≤ OK	2.894
	Check #2:	$Q_{developed} \le Q_{pre-developed}$	2.894	≤ OK	2.894
	Check #3:	$Q_{developed} shall not$ be required to be $\leq (Q_{forest} x RV_{forest}) / RV_{developed} >$	2.894	<u>shall not</u> be required to be ≤	0.478

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 - 0.40
Light Turf	0.20
Dense Turf	0.17 - 0.80
Dense Grass	0.17 - 0.30
Bermuda Grass	0.30 - 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 - 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 - 0.40
Sparse Vegetation	0.05 - 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 - 0.34
20% Debris Cover	0.05 - 0.25
Woods:	_
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 – Manning's n Values for Open Channel Flow

nan	nel T	уре		Manning n			
			Min.	Normal	Max		
1.	Exc	avated or Dredged Channels¹					
	a.	Earth, Straight, and Uniform:					
		Clean, recently completed	0.016	0.018	0.020		
		Clean, after weathering	0.018	0.022	0.02		
		Gravel, uniform section, clean	0.022	0.025	0.030		
		With short grass, few weeds	0.022	0.027	0.03		
	b.	Earth Winding and Sluggish:					
		No vegetation	0.023	0.025	0.030		
		Grass, some weeds	0.025	0.030	0.03		
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040		
		Earth bottom and rubble sides	0.028	0.030	0.03		
		Stony bottom and weedy banks	0.025	0.035	0.040		
		Cobble bottom and clean sides	0.030	<mark>0.040</mark>	0.050		
	c.	Dragline-Excavated or Dredged:		•			
		No vegetation	0.025	0.028	0.03		
		Light brush on banks	0.035	0.050	0.06		
	d.	Rock Cuts:					
		Smooth and uniform	0.025	0.035	0.040		
		Jagged and irregular	0.035	0.040	0.050		
	e.	Channels not Maintained, Weeds and Brush Uncut:					
		Dense weeds, high as flow depth	0.050	0.080	0.120		
		Clean bottom, brush on sides	0.040	0.050	0.080		
		Same as above, highest stage of flow	0.045	0.070	0.110		
		Dense brush, high stage	0.080	0.100	0.140		
2.	Mai	n Channels²					
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.03		
	b.	Same as above, but more stones and weeds	0.030	0.035	0.04		
	c.	Clean, winding, some pools and shoals	0.033	0.040	0.04		
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050		
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.05		
	f.	Same as (d) with more stones	0.045	0.050	0.060		
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080		
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150		

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

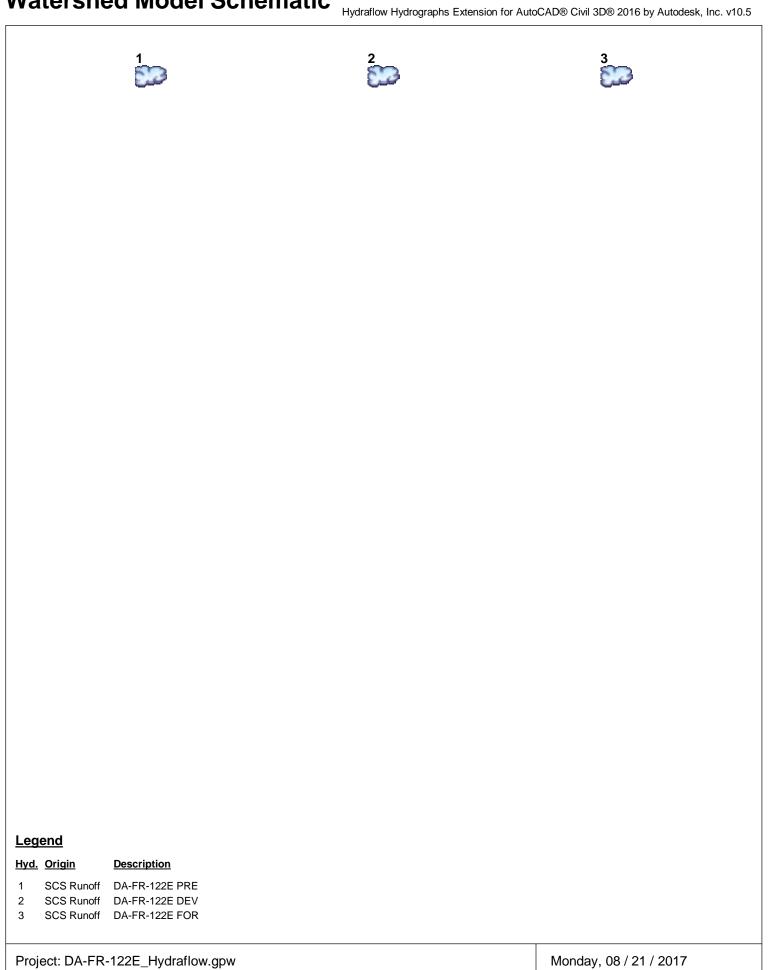
-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	. 2
1 - Year	
Summary Report	. 3
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-122E PRE	
TR-55 Tc Worksheet	
Hydrograph No. 2, SCS Runoff, DA-FR-122E DEV	
TR-55 Tc Worksheet Hydrograph No. 3, SCS Runoff, DA-FR-122E FOR	
TR-55 Tc Worksheet	
2 - Year Summary Report	10
Hydrograph Reports	10
Hydrograph No. 1, SCS Runoff, DA-FR-122E PRE	11
Hydrograph No. 2, SCS Runoff, DA-FR-122E DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-122E FOR	
10 - Year	
Summary Report	14
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-122E PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-122E DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-122E FOR	
IDE Papart	10



Hydrograph Return Period Recap Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

lyd. lo.		Inflow				Hydrograph Description					
Ю.		hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		2.894	4.325			13.34				DA-FR-122E PRE
2	SCS Runoff		2.894	4.325			13.34				DA-FR-122E DEV
3	SCS Runoff		0.892	1.773			8.862				DA-FR-122E FOR

Proj. file: DA-FR-122E_Hydraflow.gpw

Monday, 08 / 21 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description		
1	SCS Runoff	2.894	1	724	10,299				DA-FR-122E PRE		
2	SCS Runoff	2.894	1	724	10,299				DA-FR-122E DEV		
2											
DA	DA-FR-122E_Hydraflow.gpw					Return Period: 1 Year			Monday, 08 / 21 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

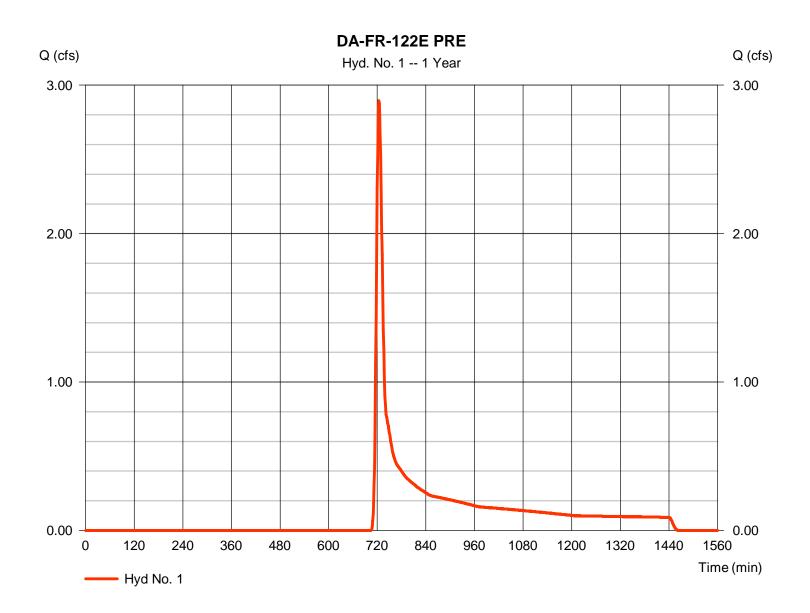
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-122E PRE

Hydrograph type = SCS Runoff Peak discharge = 2.894 cfs= 724 min Storm frequency Time to peak = 1 yrsTime interval = 1 minHyd. volume = 10.299 cuftCurve number Drainage area = 5.410 ac= 62*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 14.50 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Storm duration Shape factor = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.090 x 48) + (0.424 x 82) + (0.009 x 85) + (0.313 x 98) + (3.715 x 58) + (0.859 x 55)] / 5.410



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1

DA-FR-122E PRE

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 5.94 = 12.92	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	12.92
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 474.48 = 9.97 = Unpaved =5.09	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.55	+	0.00	+	0.00	=	1.55
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							14.50 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

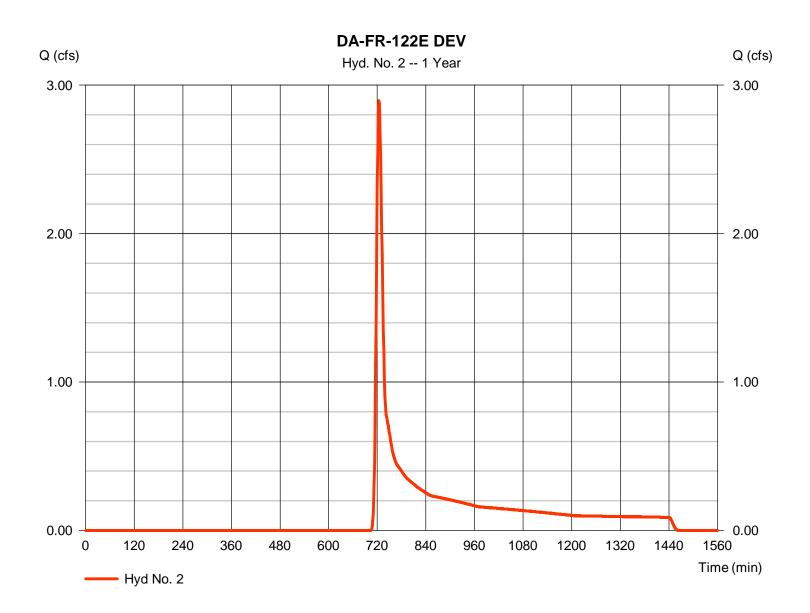
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-122E DEV

Hydrograph type = SCS Runoff Peak discharge = 2.894 cfs= 724 min Storm frequency Time to peak = 1 yrsTime interval = 1 minHyd. volume = 10.299 cuftCurve number Drainage area = 5.410 ac= 62*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 14.20 min Total precip. = 3.30 inDistribution = Type II Storm duration Shape factor = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.096 \times 48) + (0.424 \times 82) + (0.009 \times 85) + (0.313 \times 98) + (4.085 \times 58) + (0.483 \times 55)] / 5.410$



 $\label{thm:condition} \mbox{Hydrographs Extension for AutoCAD} \mbox{\@none} \mbox{\$

Hyd. No. 2

DA-FR-122E DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 5.94 = 12.92	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	12.92
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 233.55 = 8.98 = Unpave =4.83	d	143.58 16.60 Unpave 6.57	d	0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.81	+	0.36	+	0.00	=	1.17
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})21.3		0.0		0.0		
Travel Time (min)	= 0.07	+	0.00	+	0.00	=	0.07
Total Travel Time, Tc							14.20 min

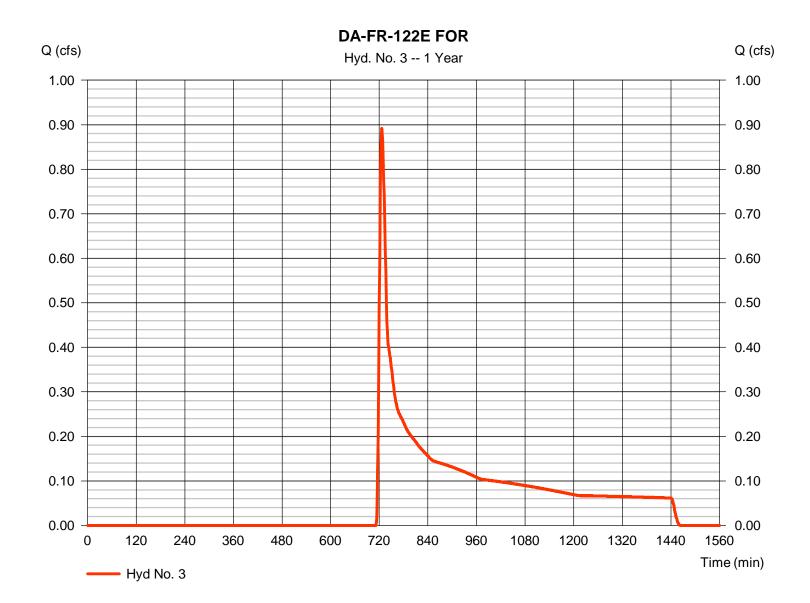
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-122E FOR

Hydrograph type = SCS Runoff Peak discharge = 0.892 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 min Hyd. volume = 5,520 cuftDrainage area Curve number = 5.410 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 14.50 \, \text{min}$ Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3

DA-FR-122E FOR

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 5.94		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 12.92	+	0.00	+	0.00	=	12.92
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 474.48 = 9.97 = Unpave =5.09	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.55	+	0.00	+	0.00	=	1.55
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							14.50 min

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

lyd. lo.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	4.325	1	724	13,973				DA-FR-122E PRE
2	SCS Runoff	4.325	1	724	13,973				DA-FR-122E DEV
	 -FR-122E_H _!	·droflov.			 	Period: 2 Ye			 3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

 $= 24 \, hrs$

Monday, 08 / 21 / 2017

= 484

Hyd. No. 1

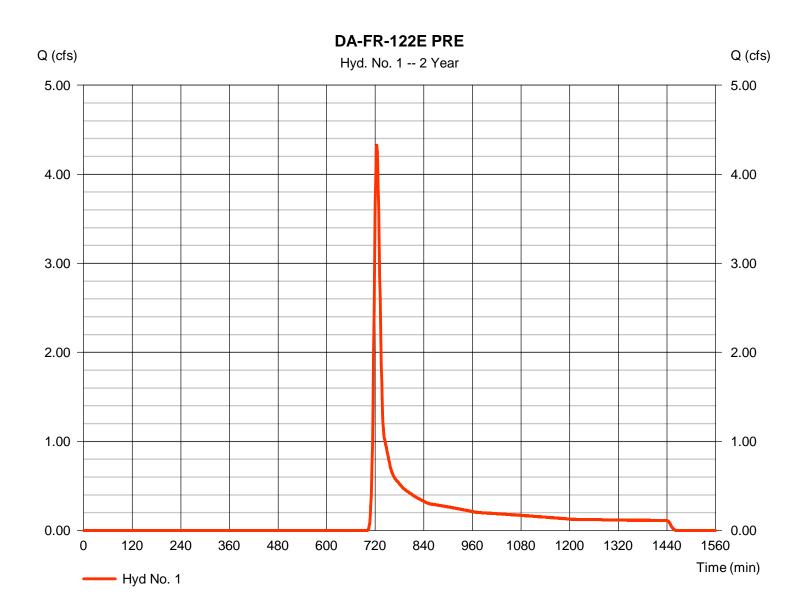
DA-FR-122E PRE

Storm duration

Hydrograph type = SCS Runoff Peak discharge = 4.325 cfsStorm frequency Time to peak = 724 min = 2 yrsTime interval = 1 minHyd. volume = 13.973 cuft Drainage area = 5.410 acCurve number = 62*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 14.50 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II

Shape factor

^{*} Composite (Area/CN) = [(0.090 x 48) + (0.424 x 82) + (0.009 x 85) + (0.313 x 98) + (3.715 x 58) + (0.859 x 55)] / 5.410



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

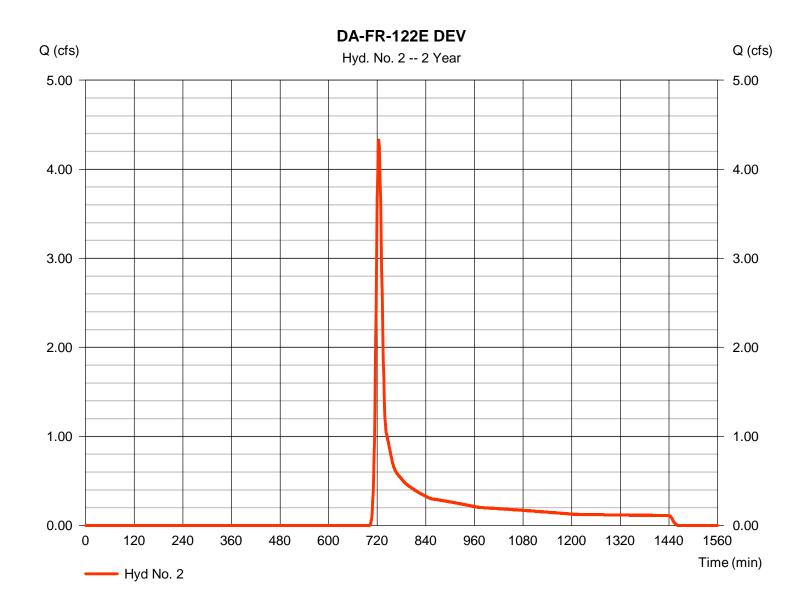
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-122E DEV

Hydrograph type = SCS Runoff Peak discharge = 4.325 cfsStorm frequency Time to peak = 724 min = 2 yrsTime interval = 1 minHyd. volume = 13.973 cuft Drainage area = 5.410 acCurve number = 62*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 14.20 min Total precip. = 3.70 inDistribution = Type II

^{*} Composite (Area/CN) = [(0.096 x 48) + (0.424 x 82) + (0.009 x 85) + (0.313 x 98) + (4.085 x 58) + (0.483 x 55)] / 5.410



Storm duration = 484 $= 24 \, hrs$ Shape factor

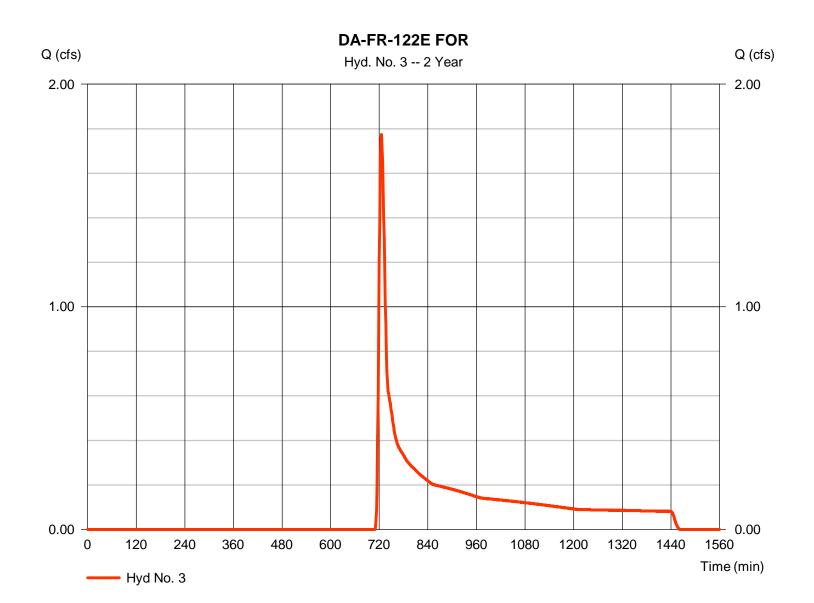
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-122E FOR

= SCS Runoff = 1.773 cfsHydrograph type Peak discharge Storm frequency Time to peak = 725 min = 2 yrsTime interval = 1 min Hyd. volume = 8,161 cuftDrainage area Curve number = 5.410 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 14.50 \, \text{min}$ Total precip. Distribution = 3.70 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	13.34	1	723	37,076				DA-FR-122E PRE	
2	SCS Runoff	13.34	1	723	37,076				DA-FR-122E DEV	
DA	DA-FR-122E_Hydraflow.gpw			Return F	Return Period: 10 Year			Monday, 08 / 21 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

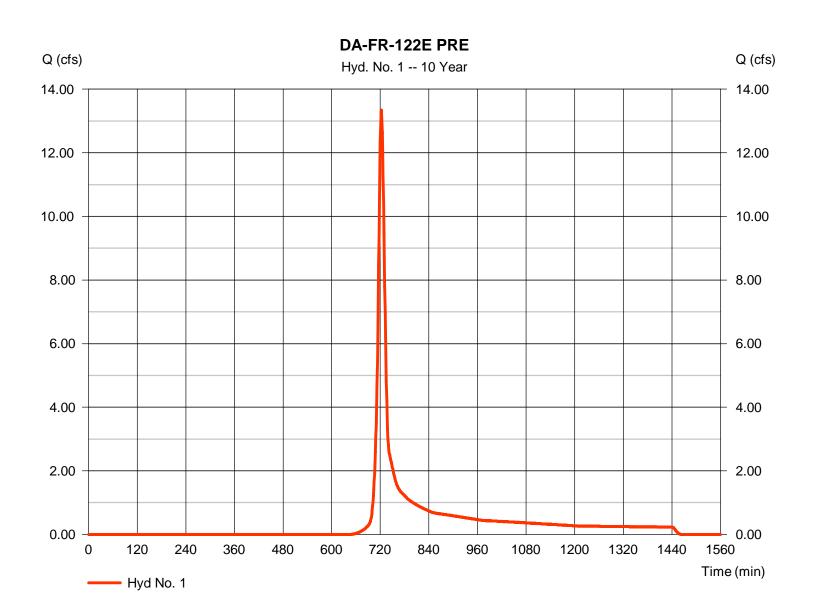
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-122E PRE

Hydrograph type = SCS Runoff Peak discharge = 13.34 cfsStorm frequency Time to peak = 723 min = 10 yrsTime interval = 1 min Hyd. volume = 37,076 cuftCurve number Drainage area = 5.410 ac= 62*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 14.50 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.090 \times 48) + (0.424 \times 82) + (0.009 \times 85) + (0.313 \times 98) + (3.715 \times 58) + (0.859 \times 55)] / 5.410$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

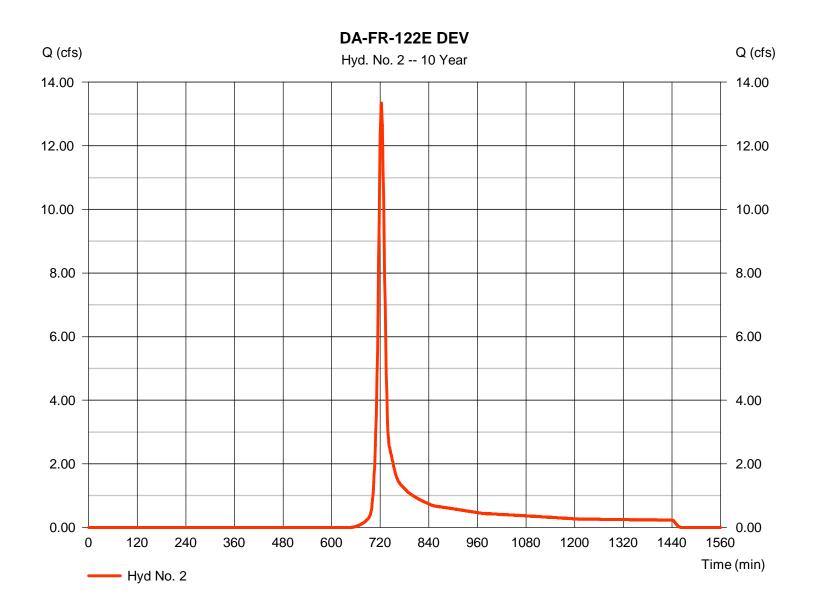
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-122E DEV

Hydrograph type = SCS Runoff Peak discharge = 13.34 cfsStorm frequency Time to peak = 723 min = 10 yrsTime interval = 1 min Hyd. volume = 37,076 cuftCurve number Drainage area = 5.410 ac= 62*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 14.20 min Total precip. = 5.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.096 \times 48) + (0.424 \times 82) + (0.009 \times 85) + (0.313 \times 98) + (4.085 \times 58) + (0.483 \times 55)] / 5.410$



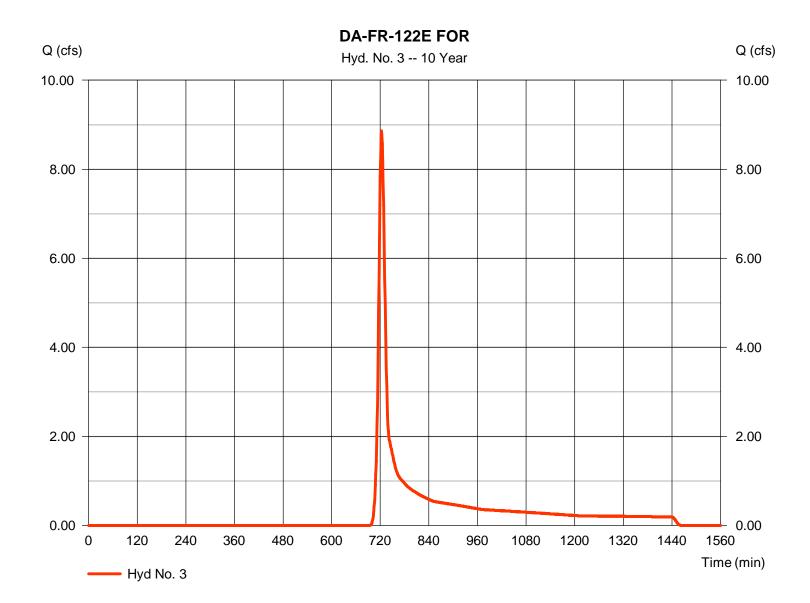
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-122E FOR

= SCS Runoff Hydrograph type Peak discharge = 8.862 cfsStorm frequency Time to peak = 723 min = 10 yrsTime interval = 1 min Hyd. volume = 26.482 cuftDrainage area Curve number = 5.410 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 14.50 \, \text{min}$ Total precip. Distribution = 5.70 in= Type II Storm duration = 484 = 24 hrs Shape factor



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)							
(Yrs)	В	D	E	(N/A)				
1	0.0000	0.0000	0.0000					
2	69.8703	13.1000	0.8658					
3	0.0000	0.0000	0.0000					
5	79.2597	14.6000	0.8369					
10	88.2351	15.5000	0.8279					
25	102.6072	16.5000	0.8217					
50	114.8193	17.2000	0.8199					
100	127.1596	17.8000	0.8186					

File name: SampleFHA.idf

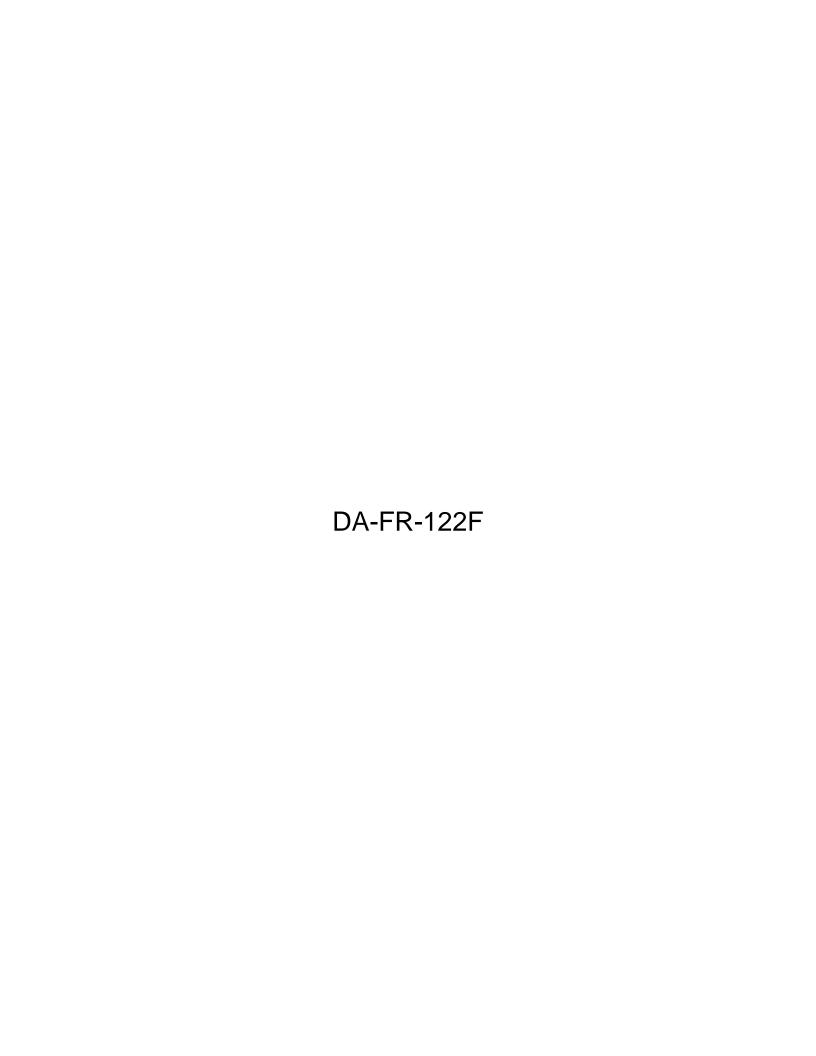
Intensity = $B/(Tc + D)^E$

Return		Intensity Values (in/hr)										
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc|

		Rainfall Precipitation Table (in)							
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00	
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	



ENERGY BALANCE METHOD

		ts

Calculations:

	1-Yr Event			
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)		
Pre-Developed Condition	1.311	5978		
Developed Condition	1.311	5978		
Pre-Developed (Forest) Condition	0.608	4081		

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF =

 Check #1:
 Q_{developed} ≤ IF x [[Q_{gre-developed} x RV_{pre-developed}] / RV_{developed}] ······>
 1.33

 Check #2:
 Q_{developed} ≤ Q_{gre-developed} ······>
 1.33

Check #3: $Q_{developed} \underline{shall \ not}$ be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed} \longrightarrow$

1.311	≤ OK	1.311
1.311	≤ OK	1.311
1.311	<u>shall not</u> be required to be ≤	0.415

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n						
Grass:							
Average Grass Cover	0.40						
Poor Grass Cover, Moderately Rough Surface	0.30 - 0.40						
Light Turf	0.20						
Dense Turf	0.17 - 0.80						
Dense Grass	0.17 - 0.30						
Bermuda Grass	0.30 - 0.48						
Dense Shrubbery and Forest Litter	0.40						
Natural:							
Short Grass Prairie	0.10 - 0.20						
Poor Grass Cover, Moderately Rough Surface	0.30 - 0.40						
Sparse Vegetation	0.05 - 0.13						
Oak Grasslands, Open Grasslands	0.60						
Dense Cover of Trees and Bushes	0.80						
Rangeland:							
Typical	0.13						
No Debris Cover	0.09 - 0.34						
20% Debris Cover	0.05 - 0.25						
Woods:	_						
Light Underbrush	0.40						
Dense Underbrush	0.80						
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40						

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 – Manning's n Values for Open Channel Flow

nannel Type				Manning n			
			Min.	Normal	Max		
1.	Exc	avated or Dredged Channels¹					
	a.	Earth, Straight, and Uniform:					
		Clean, recently completed	0.016	0.018	0.020		
		Clean, after weathering	0.018	0.022	0.02		
		Gravel, uniform section, clean	0.022	0.025	0.030		
		With short grass, few weeds	0.022	0.027	0.03		
	b.	Earth Winding and Sluggish:					
		No vegetation	0.023	0.025	0.030		
		Grass, some weeds	0.025	0.030	0.03		
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040		
		Earth bottom and rubble sides	0.028	0.030	0.03		
		Stony bottom and weedy banks	0.025	0.035	0.040		
		Cobble bottom and clean sides	0.030	<mark>0.040</mark>	0.050		
	c.	Dragline-Excavated or Dredged:		•			
		No vegetation	0.025	0.028	0.03		
		Light brush on banks	0.035	0.050	0.06		
	d.	Rock Cuts:					
		Smooth and uniform	0.025	0.035	0.040		
		Jagged and irregular	0.035	0.040	0.050		
	e.	Channels not Maintained, Weeds and Brush Uncut:					
		Dense weeds, high as flow depth	0.050	0.080	0.120		
		Clean bottom, brush on sides	0.040	0.050	0.080		
		Same as above, highest stage of flow	0.045	0.070	0.110		
		Dense brush, high stage	0.080	0.100	0.140		
2.	Mai	n Channels²					
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.03		
	b.	Same as above, but more stones and weeds	0.030	0.035	0.04		
	c.	Clean, winding, some pools and shoals	0.033	0.040	0.04		
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050		
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.05		
	f.	Same as (d) with more stones	0.045	0.050	0.060		
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080		
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150		

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

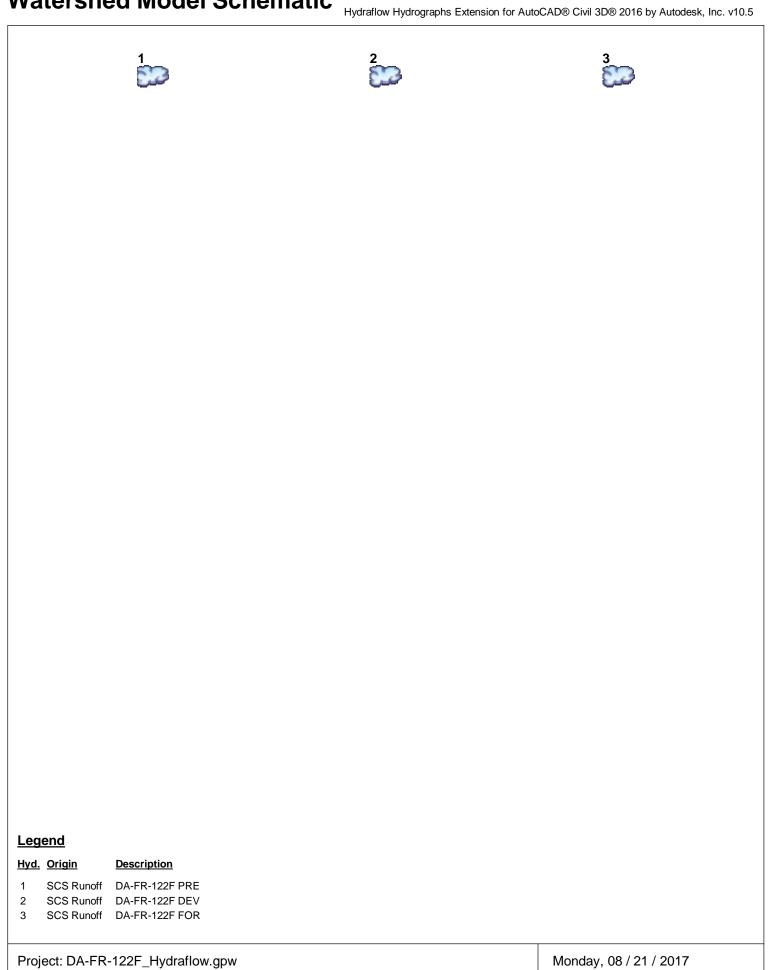
-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	4
Hydrograph No. 1, SCS Runoff, DA-FR-122F PRE	4
TR-55 Tc Worksheet	
Hydrograph No. 2, SCS Runoff, DA-FR-122F DEV	6
TR-55 Tc Worksheet	7
Hydrograph No. 3, SCS Runoff, DA-FR-122F FOR	
TR-55 Tc Worksheet	9
0 Vaan	
2 - Year	4.0
Summary Report	10
Hydrograph Reports	11
Hydrograph No. 1, SCS Runoff, DA-FR-122F PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-122F DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-122F FOR	13
10 - Year	
Summary Report	14
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-122F PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-122F DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-122F FOR	
IDE Papart	10
IIIL PARATT	7 2



Hydrograph Return Period Recap Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Inflow hyd(s)	Peak Outflow (cfs)								Hydrograph
			1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		1.311	2.145			7.886				DA-FR-122F PRE
2	SCS Runoff		1.311	2.145			7.886				DA-FR-122F DEV
3	SCS Runoff		0.608	1.207			6.104				DA-FR-122F FOR
				<u> </u>	<u></u> _		<u></u>			<u></u>	

Proj. file: DA-FR-122F_Hydraflow.gpw

Monday, 08 / 21 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	1.311	1	726	5,978				DA-FR-122F PRE
2	SCS Runoff	1.311	1	726	5,978				DA-FR-122F DEV
	\-FR-122F_Hy				Return F			Monday, 08	

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

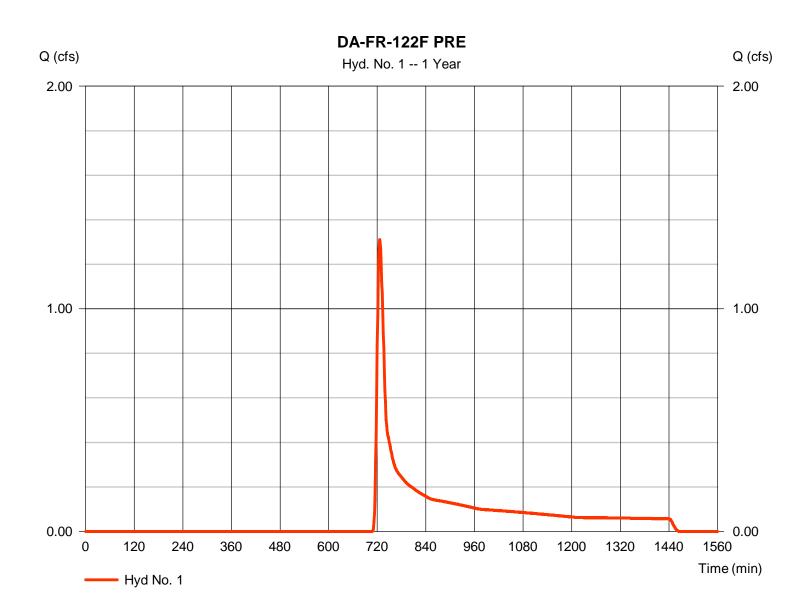
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-122F PRE

= SCS Runoff Hydrograph type Peak discharge = 1.311 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 minHyd. volume = 5,978 cuftDrainage area Curve number = 3.950 ac= 59*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.80 min Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.259 \times 48) + (0.215 \times 82) + (3.473 \times 58)] / 3.950$



Hyd. No. 1

DA-FR-122F PRE

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 4.12 = 14.96		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		14.96
Travel Time (min)	= 14.90	+	0.00	+	0.00	=	14.90
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 297.54 = 12.41 = Unpave =5.68	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.87	+	0.00	+	0.00	=	0.87
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							15.80 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

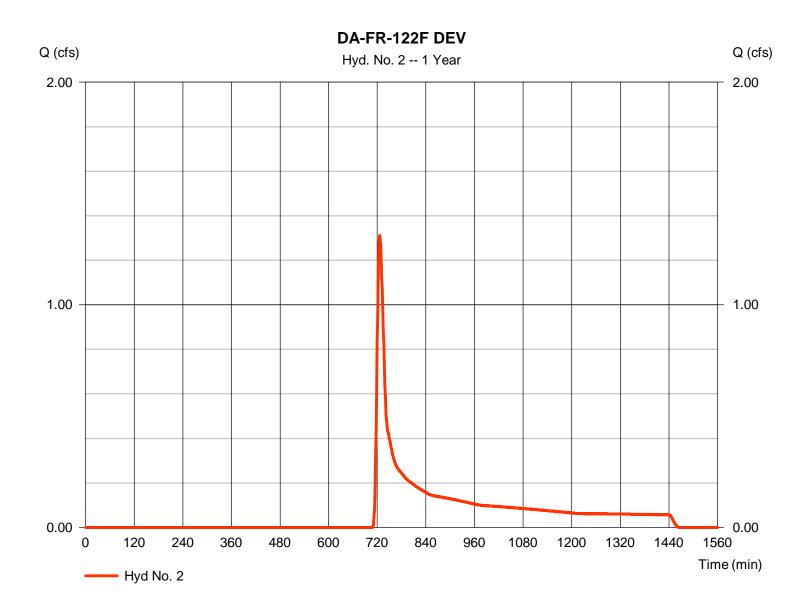
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-122F DEV

= SCS Runoff Hydrograph type Peak discharge = 1.311 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 minHyd. volume = 5,978 cuftDrainage area Curve number = 3.950 ac= 59*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 16.10 min Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.151 \times 48) + (0.215 \times 82) + (3.581 \times 58)] / 3.950$



Hyd. No. 2

DA-FR-122F DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 4.12		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 14.96	+	0.00	+	0.00	=	14.96
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 27.04 = 6.98 = Unpave =4.26	d	145.41 8.62 Unpave 4.74	d	123.95 16.16 Unpave 6.49	ed	
Travel Time (min)	= 0.11	+	0.51	+	0.32	=	0.94
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		2.00 4.47 5.00 0.040 4.86		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})55.8		5.8		0.0		
Travel Time (min)	= 0.19	+	0.02	+	0.00	=	0.21
Total Travel Time, Tc							16.10 min

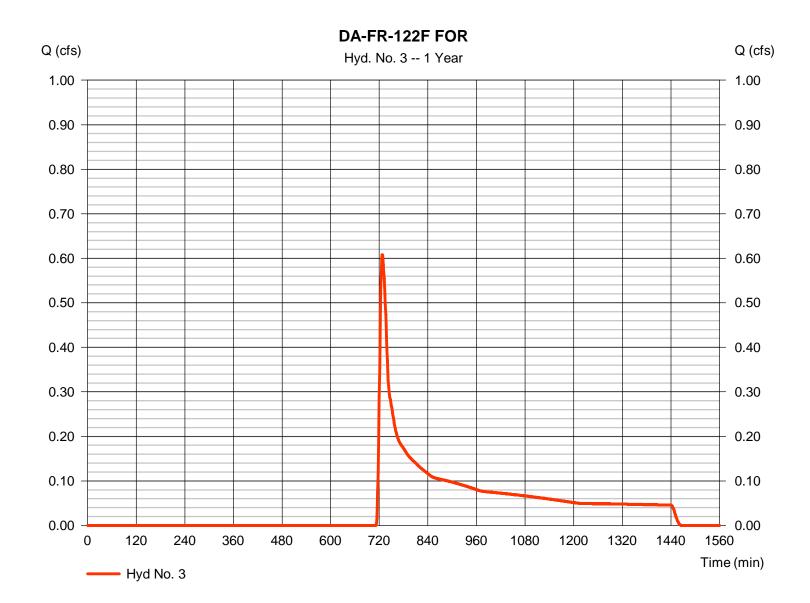
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-122F FOR

Hydrograph type = SCS Runoff Peak discharge = 0.608 cfsStorm frequency Time to peak = 727 min = 1 yrsTime interval = 1 min Hyd. volume = 4,081 cuftDrainage area Curve number = 3.950 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 15.80 min Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hyd. No. 3

DA-FR-122F FOR

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 4.12 = 14.96		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		14.96
Travel Time (min)	= 14.90	+	0.00	+	0.00	=	14.90
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 297.54 = 12.41 = Unpave =5.68	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.87	+	0.00	+	0.00	=	0.87
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							15.80 min

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	2.145	1	725	8,368				DA-FR-122F PRE	
2	SCS Runoff	2.145	1	725	8,368				DA-FR-122F DEV	
DA	-FR-122F_H _y	√draflow.g	jpw	ı	Return F	Period: 2 Ye	ear	Monday, 08 / 21 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

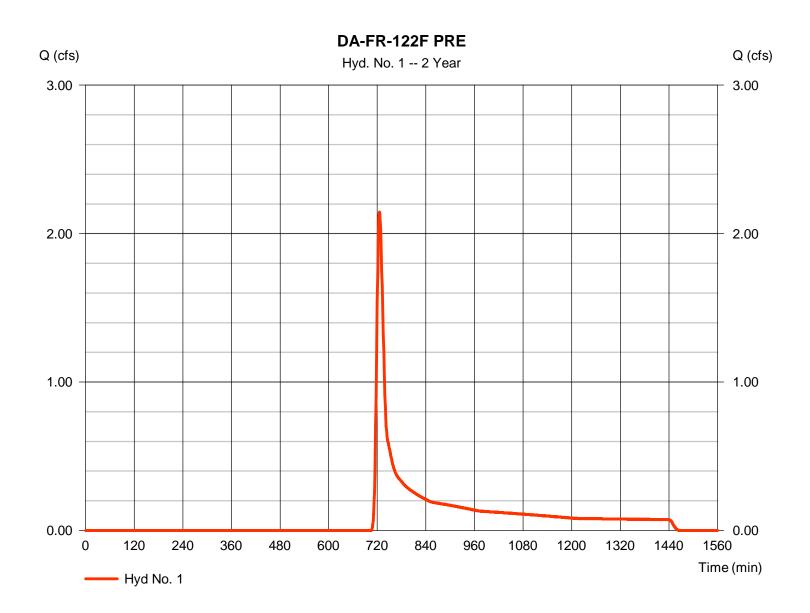
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-122F PRE

= SCS Runoff Hydrograph type Peak discharge = 2.145 cfsStorm frequency Time to peak = 725 min = 2 yrsTime interval = 1 minHyd. volume = 8,368 cuftDrainage area Curve number = 3.950 ac= 59*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.80 min Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.259 \times 48) + (0.215 \times 82) + (3.473 \times 58)] / 3.950$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

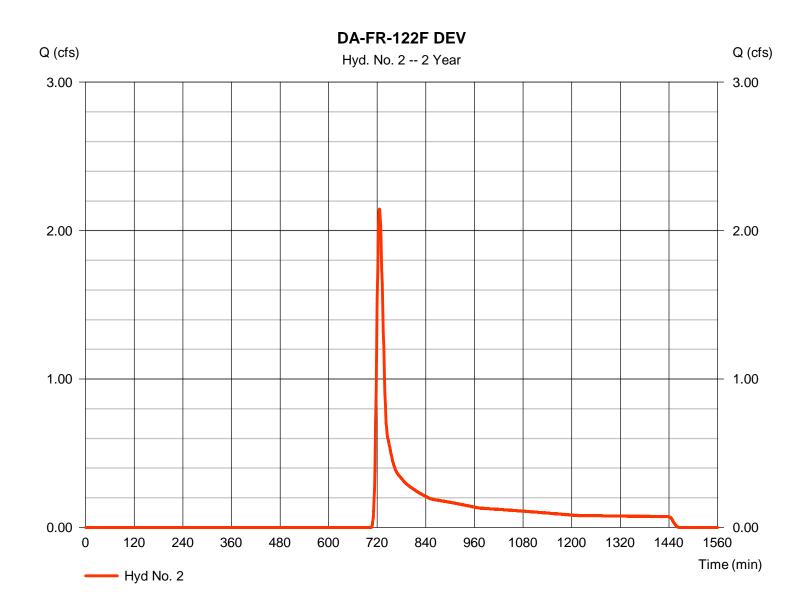
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-122F DEV

= SCS Runoff Hydrograph type Peak discharge = 2.145 cfsStorm frequency Time to peak = 725 min = 2 yrsTime interval = 1 minHyd. volume = 8,368 cuftCurve number Drainage area = 3.950 ac= 59*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 16.10 min Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.151 \times 48) + (0.215 \times 82) + (3.581 \times 58)] / 3.950$



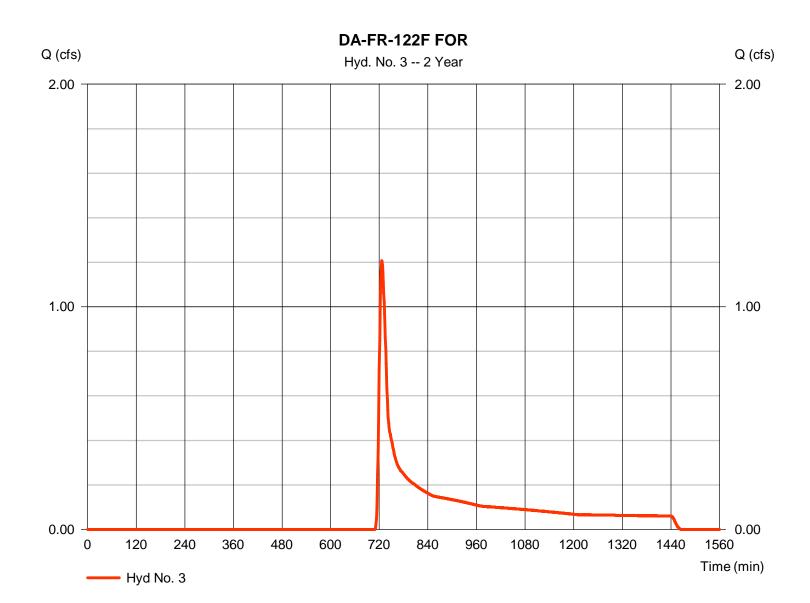
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-122F FOR

= SCS Runoff = 1.207 cfsHydrograph type Peak discharge Storm frequency Time to peak = 726 min = 2 yrsTime interval = 1 min Hyd. volume = 6,033 cuftDrainage area Curve number = 3.950 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 15.80 min Total precip. Distribution = 3.70 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	7.886	1	724	23,954				DA-FR-122F PRE
2	SCS Runoff	7.886	1	724	23,954				DA-FR-122F DEV
2									
DA-FR-122F_Hydraflow.gpw					Return F	eriod: 10 Y	l ′ear	Monday, 08	3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

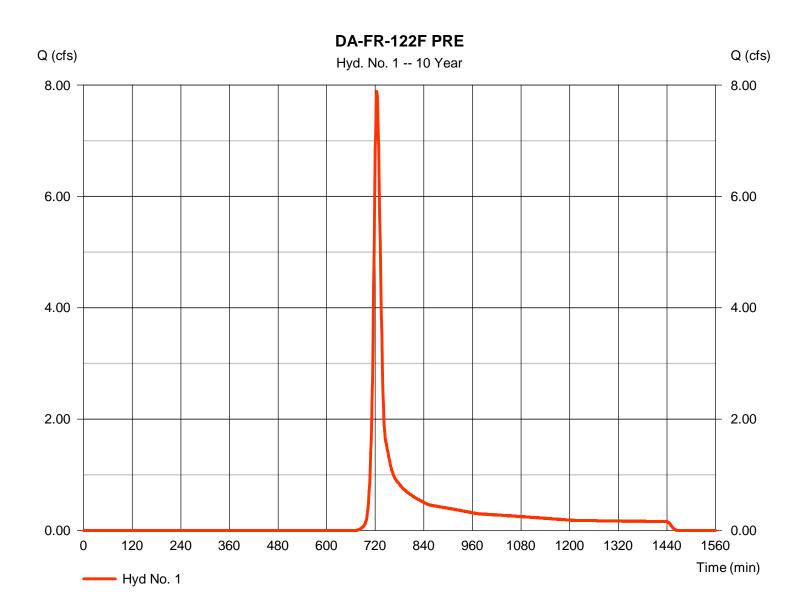
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-122F PRE

= SCS Runoff Hydrograph type Peak discharge = 7.886 cfsStorm frequency Time to peak = 724 min = 10 yrsTime interval = 1 min Hyd. volume = 23.954 cuft Drainage area Curve number = 3.950 ac= 59*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.80 min Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.259 \times 48) + (0.215 \times 82) + (3.473 \times 58)] / 3.950$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

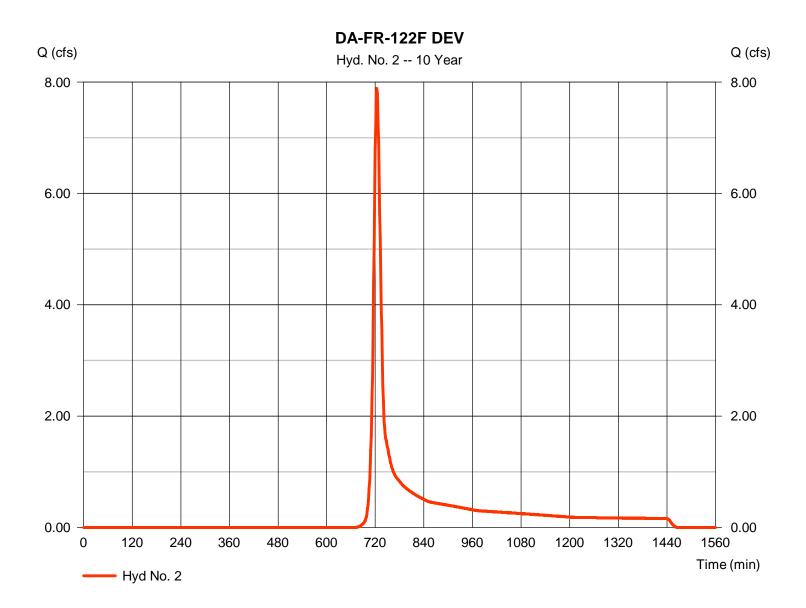
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-122F DEV

= SCS Runoff Hydrograph type Peak discharge = 7.886 cfsStorm frequency Time to peak = 724 min = 10 yrsTime interval = 1 min Hyd. volume = 23.954 cuft Drainage area Curve number = 3.950 ac= 59*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 16.10 min Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.151 \times 48) + (0.215 \times 82) + (3.581 \times 58)] / 3.950$



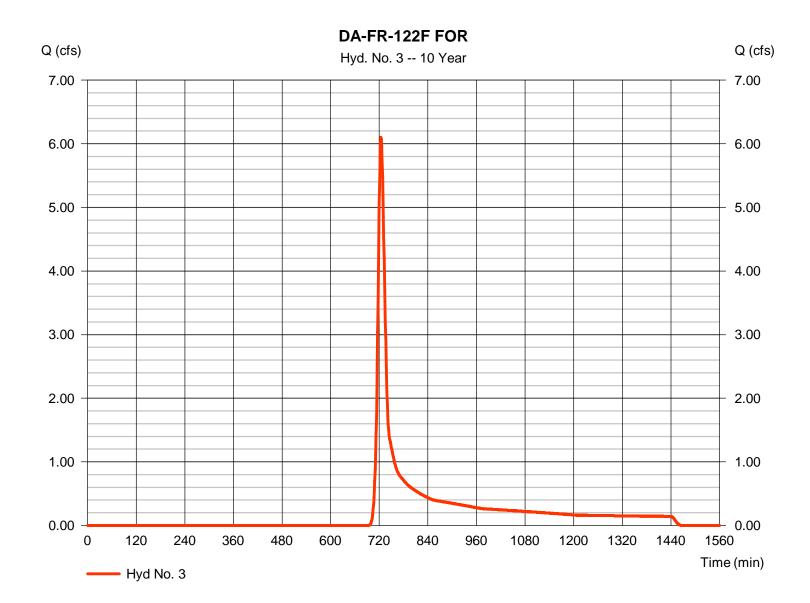
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-122F FOR

= SCS Runoff Hydrograph type Peak discharge = 6.104 cfsStorm frequency Time to peak = 724 min = 10 yrsTime interval = 1 min Hyd. volume = 19,577 cuftDrainage area Curve number = 3.950 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.80 min Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Return Period	Intensity-Du	Intensity-Duration-Frequency Equation Coefficients (FHA)									
(Yrs)	В	D	E	(N/A)							
1	0.0000	0.0000	0.0000								
2	69.8703	13.1000	0.8658								
3	0.0000	0.0000	0.0000								
5	79.2597	14.6000	0.8369								
10	88.2351	15.5000	0.8279								
25	102.6072	16.5000	0.8217								
50	114.8193	17.2000	0.8199								
100	127.1596	17.8000	0.8186								

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return					Intens	sity Values	(in/hr)					
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc

	Rainfall Precipitation Table (in)									
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr		
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00		
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		

DA-FR-123

DA-FR-123 is located in a meadow and forested areas with rolling slopes and contains agricultural land, Listening Hill Road, existing dirt road and gravel road. A stream crosses the drainage area. The total phosphorus load reduction required for DA-FR-123 is -0.52 lb/yr. Total phosphorus load reduction is not required for DA-FR-123. Multiple points of analysis were evaluated within DA-FR-123 to evaluate the effects on each receiving stream/channel following construction. Specifically, DA-FR-123 was subdivided into two sub-drainage areas (sub-areas A and B).

Sub-areas 123A and 123B contain both agricultural and non-agricultural areas within the limits of disturbance (LOD). Pre-construction agricultural areas will be returned to agricultural land use (i.e., returned to crop production, in identical condition) following construction. In non-agricultural areas, land use will be restored following construction as noted in the Stormwater Management (SWM) Narrative and the Annual Standards and Specifications. Agricultural areas within the LOD are included in the SWM quality analysis and the total permanent Right of Way (ROW) is analyzed via VRRM; in these calculations agricultural areas are considered "Forest/Open Space".

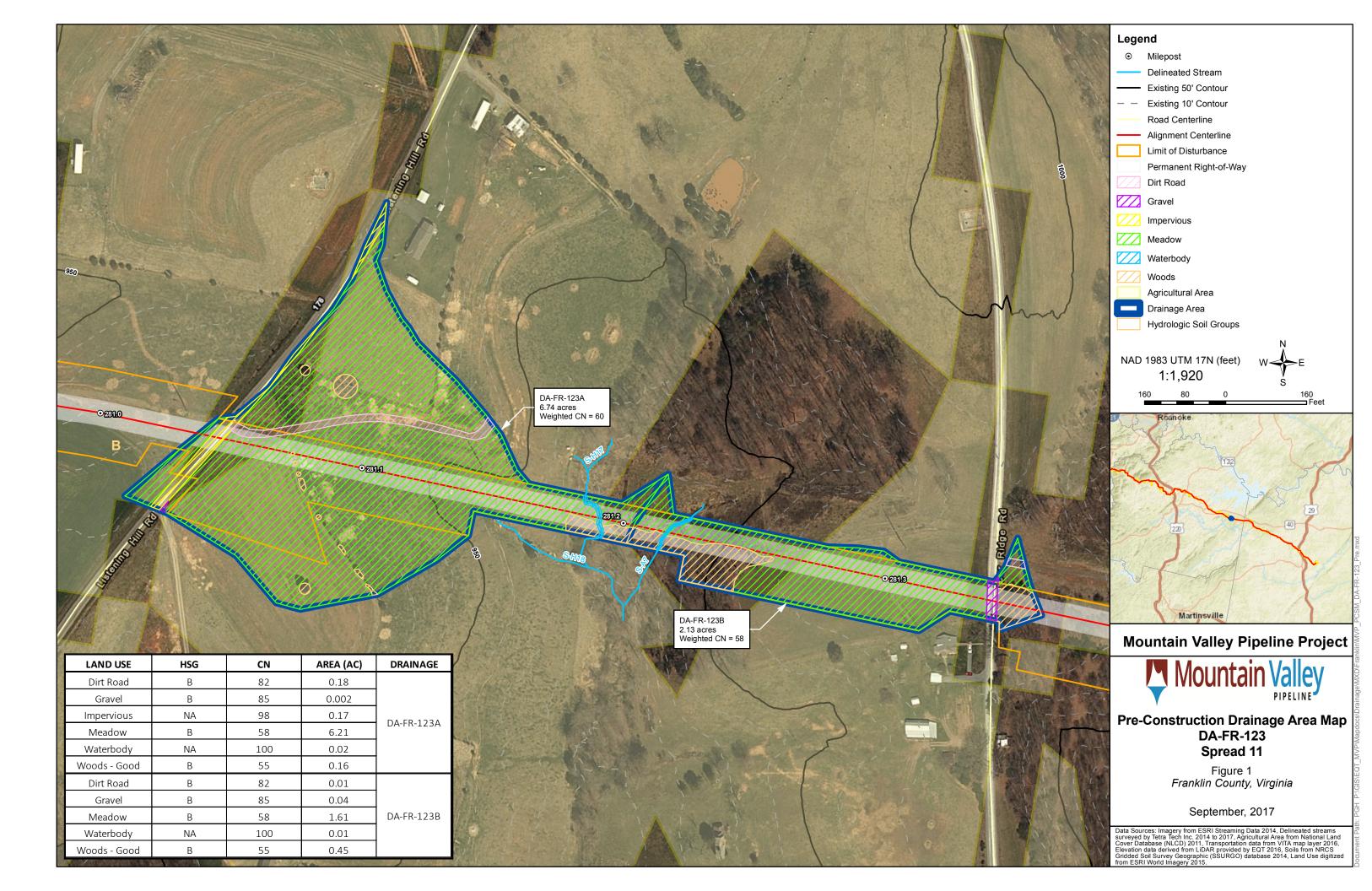
Stormwater quantity is met via the energy balance method for each of the two subareas – DA-FR-123A and DA-FR-123B. Agricultural areas within the study area are included in the SWM analysis, but an Improvement Factor (IF) of 1.0 is used when applying the Energy Balance Method. This improvement factor is used to account for the exemption of agricultural areas (§ 62.1-44.15:34 and 9VAC25-870-300) since such areas will be returned to agricultural land use (i.e., returned to crop production, in identical condition) following construction.

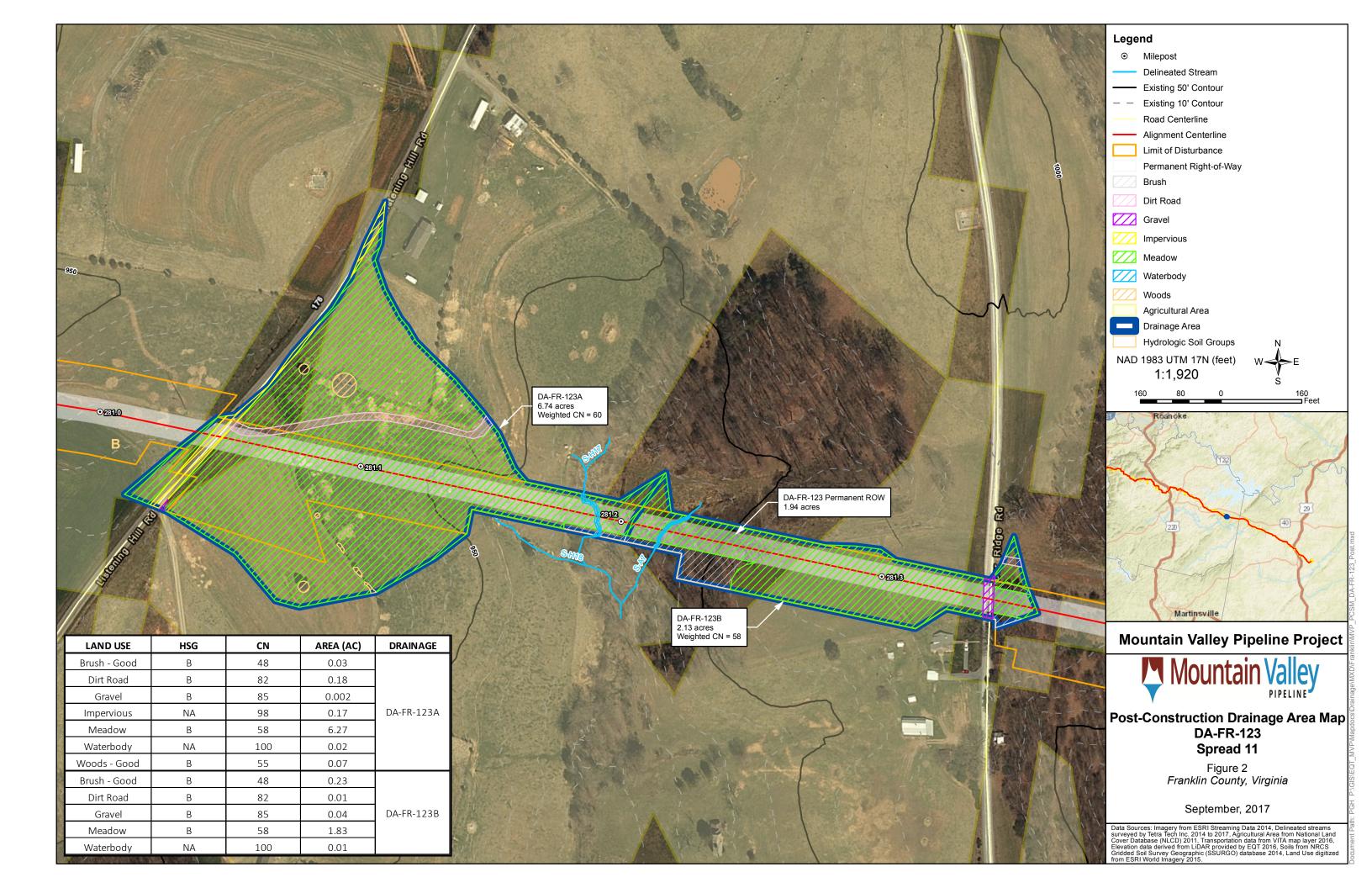
In addition, the Hydraflow Hydrograph's 10-year 24-hour peak discharge results indicate no reduction in flows for all drainage areas (as seen in table below).

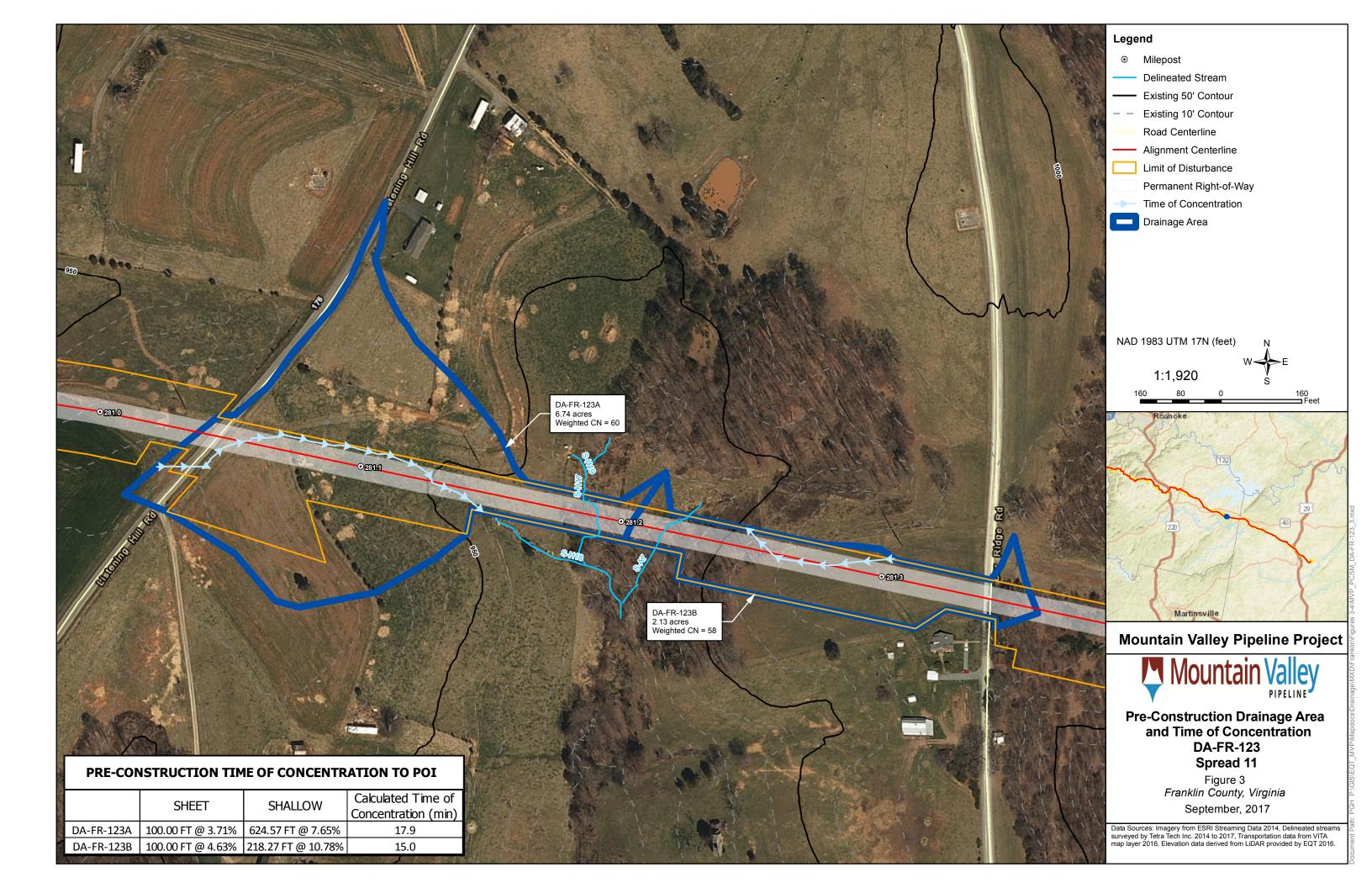
Sub Area	Pre Peak Flow,	Post Peak Flow, Q	Flow differential
	10-yr Q (cfs)	10-yr (cfs)	
DA-FR-123A	13.33	13.33	0
DA-FR-123B	4.24	4.24	0

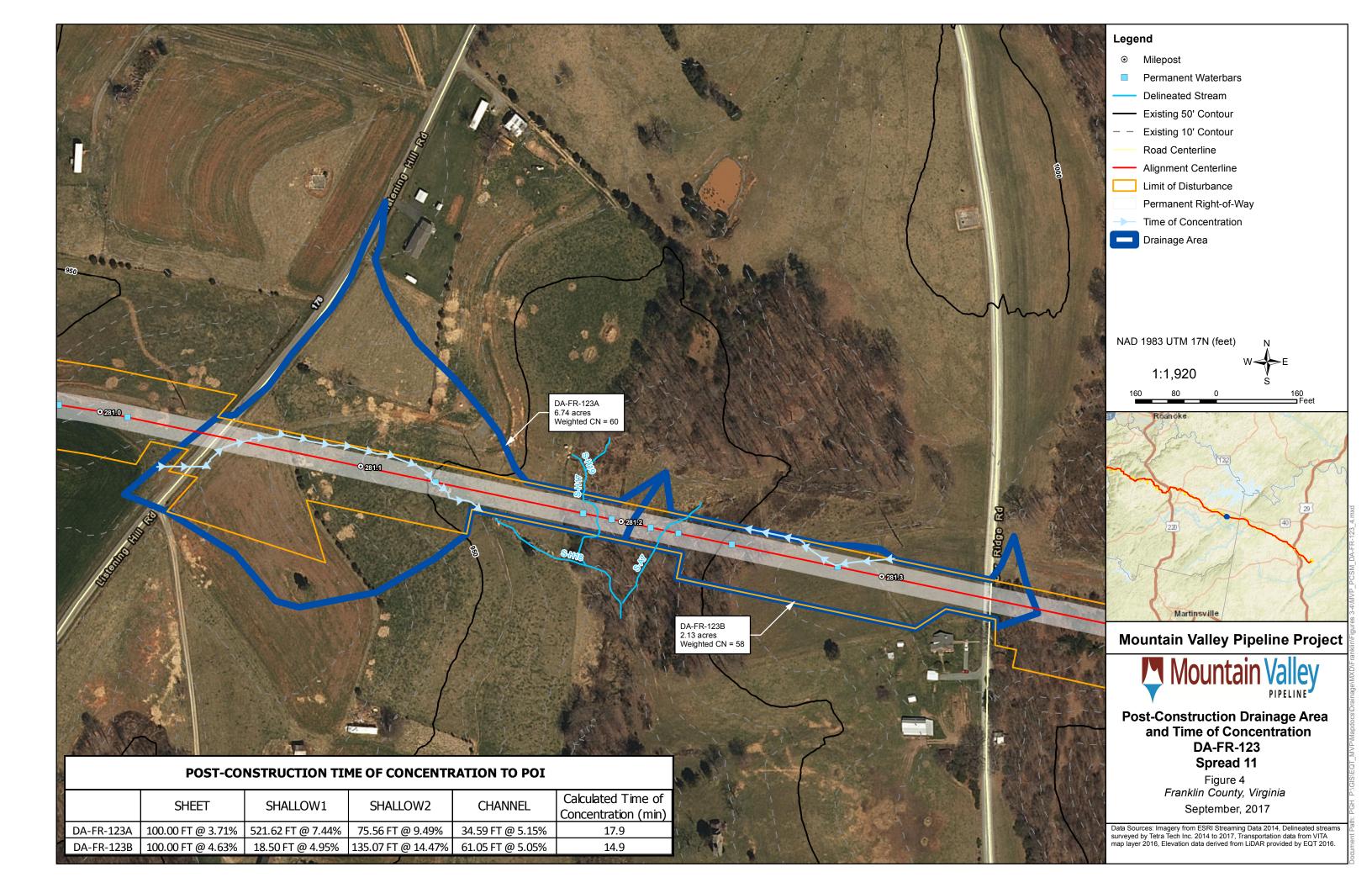
Figures and calculations for each of the sub-areas for DA-FR-123 follow. See Appendix D of the Annual Standards and Specifications for further detail on stormwater methodology.

Note that the sub-areas 123A and 123B had time of concentration flow paths for the Post-Construction condition that accounted for a permanent water bar that is not cited within the drainage area. This permanent water bar was removed because it was cited in an agricultural area, and resulting changes to the stormwater calculations were flagged during the QA/QC review process. This change was considered to be inconsequential because no stormwater BMPs are sited in DA-FR-123A or 123B, so the resulting changes were not made to the stormwater calculations prior to submittal.









DEQ Virginia Runoff Reduction Method Re-Development Compliance Spreadsheet - Version 3.0

BMP Design Specifications List: 2013 Draft Stds & Specs

Site Summary - Linear Development Project***

Total Rainfall (in):	43
Total Disturbed Acreage:	1.94

Site Land Cover Summary

Pre-ReDevelopment Land Cover (acres)

(4.6.5						
*************************************	A soils	B Soils	C Soils	D Soils	Totals	% of Total
Forest/Open (acres)	0.00	1.87	0.00	0.00	1.87	96
Managed Turf (acres)	0.00	0.00	0.00	0.00	0.00	0
Impervious Cover (acres)	0.00	0.07	0.00	0.00	0.07	4
					1.94	100

Post-ReDevelopment Land Cover (acres)

~	A soils	B Soils	C Soils	D Soils	Totals	% of Total
Forest/Open (acres)	0.00	1.87	0.00	0.00	1.87	96
Managed Turf (acres)	0.00	0.00	0.00	0.00	0.00	0
Impervious Cover (acres)	0.00	0.07	0.00	0.00	0.07	4
* Forest/Open Space areas must be protected in	1.94	100				

Site Tv and Land Cover Nutrient Loads

	Final Post-Development (Post-ReDevelopment & New Impervious)	Post- ReDevelopment	Post- Development (New Impervious)	Adjusted Pre- ReDevelopment
Site Rv	0.06	0.06		0.06
Treatment Volume (ft ³)	445	445		445
TP Load (lb/yr)	0.28	0.28		0.28
	Describes TD Least (II. /c.u.).	0.7054*	*Paduction balow now	dayalanmant laad limita

ReDevelopmen TP Load per acr (lb/acre/yr)	t TP Load per acre	Post-ReDevelopment TP Load per acre (lb/acre/yr)
0.14	0.14	0.14

Baseline TP Load (lb/yr):

0.7954*

*Reduction below new development load limitation not required

Total TP Load Reduction Required (lb/yr)	-0.52	N/A***	N/A***
--	-------	--------	--------

***This is a linear development project

	Final Post-Development Load (Post-ReDevelopment & New Impervious)	Pre- ReDevelopment
TN Load (lb/yr)	2.00	2.00

Site Compliance Summary - ***Linear Development Project

Maximum % Reduction Required Below	20%	* Note: % Reduction will reduce post-development TP load to less than or equal to baseline load of 0.8 lb/yr (0.41 lb/ac/yr)
Pre-ReDevelopment Load	20%	[Post-Dev Reduction Requirement = Post-Dev TP load - baseline load of 0.8 lb/yr], baseline load = site area x 0.41 lb/ac/yr

Total TP Load Reduction Achieved (lb/yr)	0.00
Total TN Load Reduction Achieved (lb/yr)	0.00
Remaining Post Development TP Load (lb/yr)	0.28
· ·	0.28 0.00

** TARGET TP REDUCTION EXCEEDED BY 0.52 LB/YEAR **

^{*}Reduction below new development load limitation not required



ENERGY BALANCE METHOD

In		

	1-Yr Event		
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)	
Pre-Developed Condition	2.417	10836	
Developed Condition	2.417	10836	
Pre-Developed (Forest) Condition	0.969	6799	

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 1

<u>Calculations:</u>	Check #1:	$Q_{\text{developed}} \le IF \times [(Q_{\text{pre-developed}} \times RV_{\text{pre-developed}}) / RV_{\text{developed}}]$ >	2.417	≤ OK	2.417
	Check #2:	$Q_{developed} \le Q_{pre-developed}$ >>	2.417	≤ OK	2.417
	Check #3:	$Q_{\text{developed } shall \ not}$ be required to be $\leq (Q_{\text{forest}} \times RV_{\text{forest}}) / RV_{\text{developed}}$ >	2.417	shall not be required to be ≤	0.608

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 - 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 - 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 - 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 – Manning's n Values for Open Channel Flow

Channel Type		Manning n				
			Min.	Normal	Max.	
1.	Exc	avated or Dredged Channels ¹				
	a.	Earth, Straight, and Uniform:				
		Clean, recently completed	0.016	0.018	0.020	
		Clean, after weathering	0.018	0.022	0.025	
		Gravel, uniform section, clean	0.022	0.025	0.030	
		With short grass, few weeds	0.022	0.027	0.033	
	b.	Earth Winding and Sluggish:				
		No vegetation	0.023	0.025	0.030	
		Grass, some weeds	0.025	0.030	0.033	
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040	
		Earth bottom and rubble sides	0.028	0.030	0.03	
		Stony bottom and weedy banks	0.025	0.035	0.040	
		Cobble bottom and clean sides	0.030	<mark>0.040</mark>	0.050	
	c.	Dragline-Excavated or Dredged:				
		No vegetation	0.025	0.028	0.03	
		Light brush on banks	0.035	0.050	0.060	
	d.	Rock Cuts:				
		Smooth and uniform	0.025	0.035	0.040	
		Jagged and irregular	0.035	0.040	0.050	
	e.	Channels not Maintained, Weeds and Brush Uncut:				
		Dense weeds, high as flow depth	0.050	0.080	0.120	
		Clean bottom, brush on sides	0.040	0.050	0.080	
		Same as above, highest stage of flow	0.045	0.070	0.110	
		Dense brush, high stage	0.080	0.100	0.140	
2.	Mai	n Channels²				
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.033	
	b.	Same as above, but more stones and weeds	0.030	0.035	0.040	
	C.	Clean, winding, some pools and shoals	0.033	0.040	0.04	
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050	
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.05	
	f.	Same as (d) with more stones	0.045	0.050	0.060	
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080	
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150	

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

Monday, 08 / 21 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	4
Hydrograph No. 1, SCS Runoff, DA-FR-123A PRE	4
TR-55 Tc Worksheet	
Hydrograph No. 2, SCS Runoff, DA-FR-123A DEV	6
TR-55 Tc Worksheet	7
Hydrograph No. 3, SCS Runoff, DA-FR-123A FOR	
TR-55 Tc Worksheet	9
2 - Year	
	40
Summary Report	10
Hydrograph Reports Hydrograph No. 1, SCS Runoff, DA-FR-123A PRE	II 11
Hydrograph No. 2, SCS Runoff, DA-FR-123A DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-123A FOR	
Trydrograph No. 3, 300 Kunon, DA-1 K-120A 1 OK	10
10 - Year	
Summary Report	14
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-123A PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-123A DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-123A FOR	
IDE Papart	10

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5 <u>Legend</u> Hyd. Origin **Description** SCS Runoff DA-FR-123A PRE 2 SCS Runoff DA-FR-123A DEV SCS Runoff DA-FR-123A FOR Monday, 08 / 21 / 2017 Project: DA-FR-123A_Hydraflow.gpw

Hydrograph Return Period Recap

		Inflow				Hydrograph					
lo.			hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
1	SCS Runoff		2.417	3.834			13.33				DA-FR-123A PRE
2	SCS Runoff		2.417	3.834			13.33				DA-FR-123A DEV
3	SCS Runoff		0.969	1.918			9.738				DA-FR-123A FOR

Proj. file: DA-FR-123A_Hydraflow.gpw

Monday, 08 / 21 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description		
1	SCS Runoff	2.417	1	727	10,836				DA-FR-123A PRE		
2	SCS Runoff	2.417	1	727	10,836				DA-FR-123A DEV		
2											
DA-FR-123A_Hydraflow.gpw					Return F	eriod: 1 Ye	ear	Monday, 08 / 21 / 2017			

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

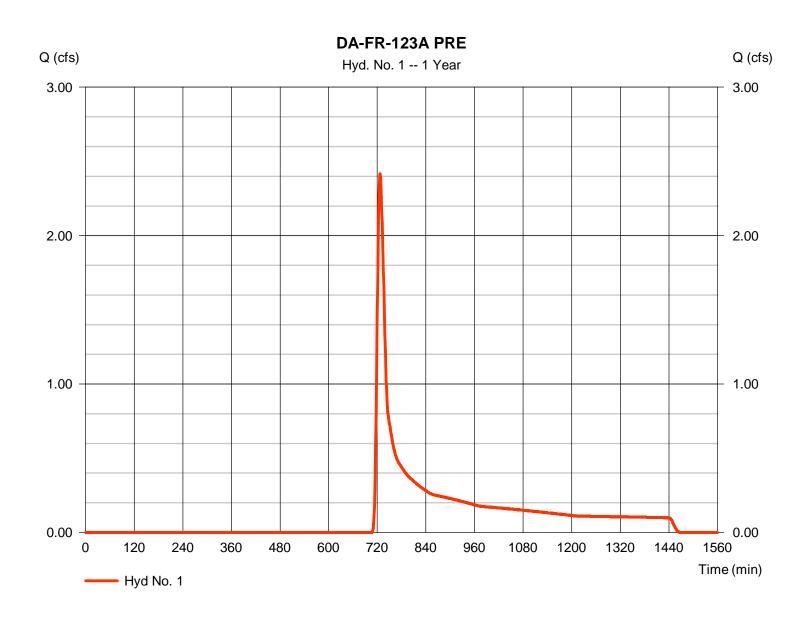
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-123A PRE

Hydrograph type = SCS Runoff Peak discharge = 2.417 cfsStorm frequency Time to peak = 727 min = 1 yrsTime interval = 1 min Hyd. volume = 10.836 cuft Curve number Drainage area = 6.740 ac= 60*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 17.90 min Total precip. = 3.30 inDistribution = Type II Storm duration Shape factor = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.183 \times 82) + (0.002 \times 85) + (0.171 \times 98) + (6.209 \times 58) + (0.018 \times 100) + (0.159 \times 55)] / 6.740$



Hyd. No. 1

DA-FR-123A PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 3.71 = 15.60	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	15.60
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 624.57 = 7.65 = Unpaved =4.46	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 2.33	+	0.00	+	0.00	=	2.33
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							17.90 min

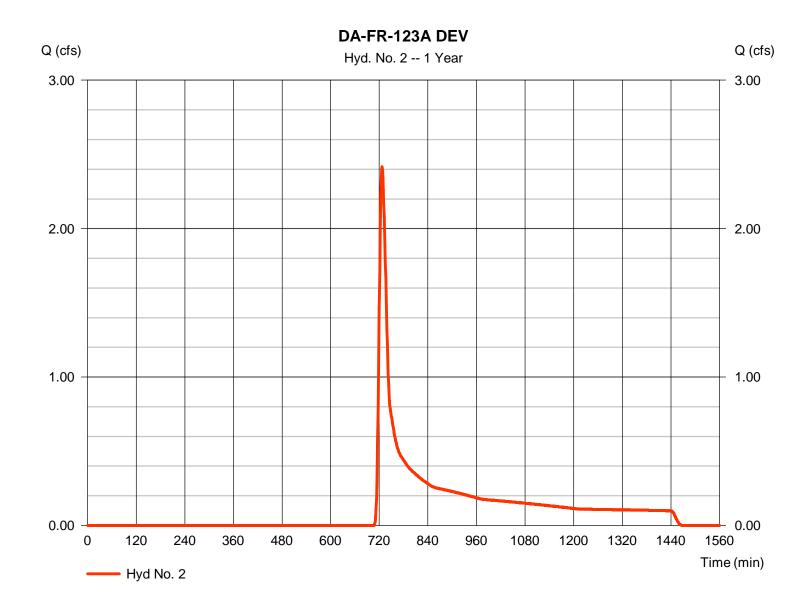
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-123A DEV

= SCS Runoff Hydrograph type Peak discharge = 2.417 cfsStorm frequency Time to peak = 727 min = 1 yrsTime interval = 1 min Hyd. volume = 10,836 cuftDrainage area Curve number = 6.740 ac= 60Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 17.90 min Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hyd. No. 2

DA-FR-123A DEV

Total Travel Time, Tc							17.90 min
Travel Time (min)	= 0.12	+	0.00	+	0.00	=	0.12
Flow length (ft)	({0})34.6		0.0		0.0		
			0.00		0.00		
Channel slope (%) Manning's n-value Velocity (ft/s)	= 5.00 = 0.040 =4.86		0.00 0.015		0.00 0.015		
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft)	= 2.00 = 4.47		0.00 0.00		0.00 0.00		
Travel Time (min)	= 1.98	+	0.25	+	0.00	=	2.23
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 521.62 = 7.44 = Unpaved =4.40	d	75.56 9.49 Unpave 4.97	ed	0.00 0.00 Paved 0.00		
Travel Time (min)	= 15.60	+	0.00	+	0.00	=	15.60
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 3.71		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>

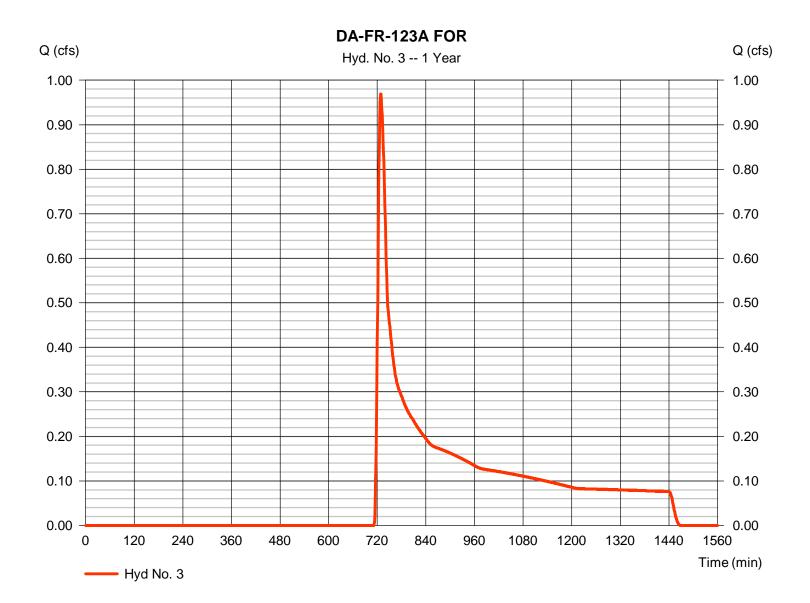
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-123A FOR

Hydrograph type = SCS Runoff Peak discharge = 0.969 cfsStorm frequency Time to peak = 729 min = 1 yrsTime interval = 1 min Hyd. volume = 6,799 cuftDrainage area Curve number = 6.740 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 17.90 min Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hyd. No. 3

DA-FR-123A FOR

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 3.71 = 15.60	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	15.60
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 624.57 = 7.65 = Unpaved =4.46	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 2.33	+	0.00	+	0.00	=	2.33
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							17.90 min

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description		
1	SCS Runoff	3.834	1	726	14,997				DA-FR-123A PRE		
2	SCS Runoff	3.834	1	726	14,997				DA-FR-123A DEV		
DA	-FR-123A_H	ydraflow.g	gpw		Return F	Return Period: 2 Year			Monday, 08 / 21 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

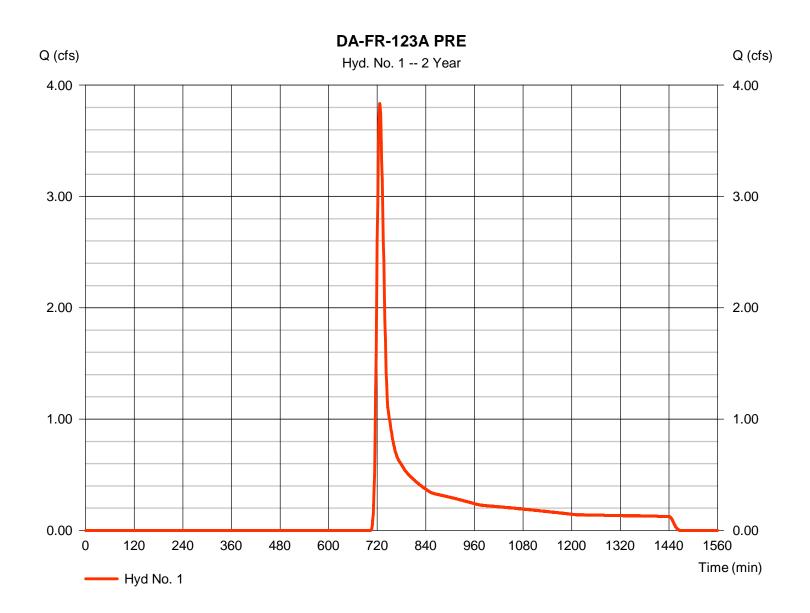
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-123A PRE

Hydrograph type = SCS Runoff Peak discharge = 3.834 cfsStorm frequency Time to peak = 726 min = 2 yrsTime interval = 1 min Hyd. volume = 14.997 cuftCurve number Drainage area = 6.740 ac= 60*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 17.90 min Total precip. = 3.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.183 \times 82) + (0.002 \times 85) + (0.171 \times 98) + (6.209 \times 58) + (0.018 \times 100) + (0.159 \times 55)] / 6.740$



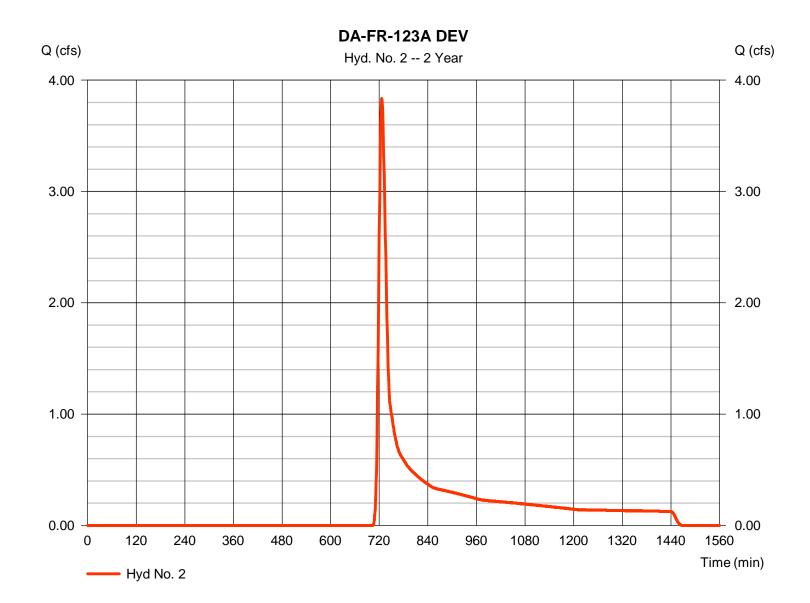
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-123A DEV

= SCS Runoff Hydrograph type Peak discharge = 3.834 cfsStorm frequency Time to peak = 726 min = 2 yrsTime interval = 1 min Hyd. volume = 14,997 cuftDrainage area Curve number = 6.740 ac= 60Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 17.90 min Total precip. Distribution = 3.70 in= Type II Storm duration = 24 hrs Shape factor = 484



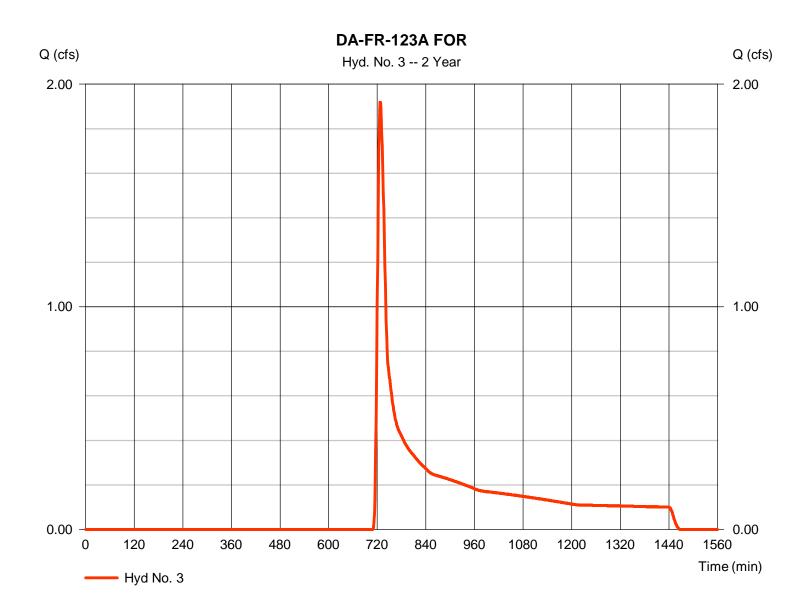
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-123A FOR

Hydrograph type = SCS Runoff Peak discharge = 1.918 cfsStorm frequency Time to peak = 727 min = 2 yrsTime interval = 1 min Hyd. volume = 10,052 cuftDrainage area Curve number = 6.740 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 17.90 min Total precip. Distribution = 3.70 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	13.33	1	725	41,802				DA-FR-123A PRE
2	SCS Runoff	13.33	1	725	41,802				DA-FR-123A DEV
2									
DA	-FR-123A_Hy	draflow.g	lbw		Return F	eriod: 10 Y	'ear	Monday, 08	3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

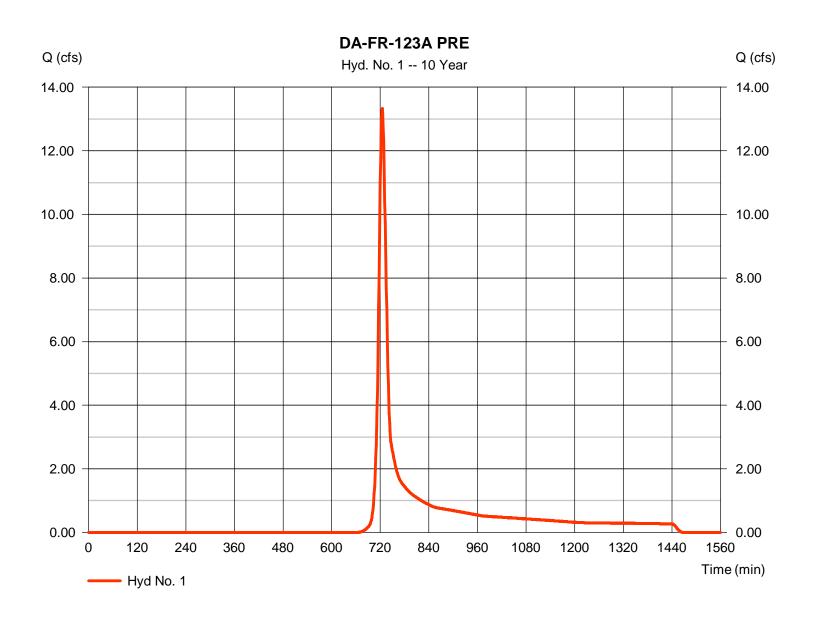
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-123A PRE

Hydrograph type = SCS Runoff Peak discharge = 13.33 cfsStorm frequency Time to peak = 725 min = 10 yrsTime interval = 1 min Hyd. volume = 41.802 cuft Drainage area Curve number = 6.740 ac= 60*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 17.90 min Total precip. = 5.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.183 \times 82) + (0.002 \times 85) + (0.171 \times 98) + (6.209 \times 58) + (0.018 \times 100) + (0.159 \times 55)] / 6.740$



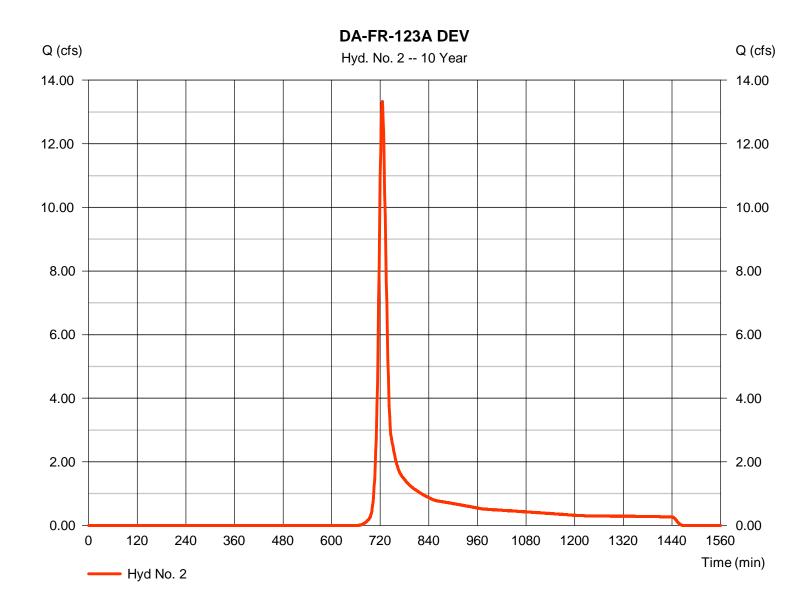
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-123A DEV

= SCS Runoff Hydrograph type Peak discharge = 13.33 cfsStorm frequency Time to peak = 725 min = 10 yrsTime interval = 1 min Hyd. volume = 41,802 cuftDrainage area Curve number = 6.740 ac= 60Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 17.90 min Total precip. Distribution = 5.70 in= Type II Storm duration = 24 hrs Shape factor = 484



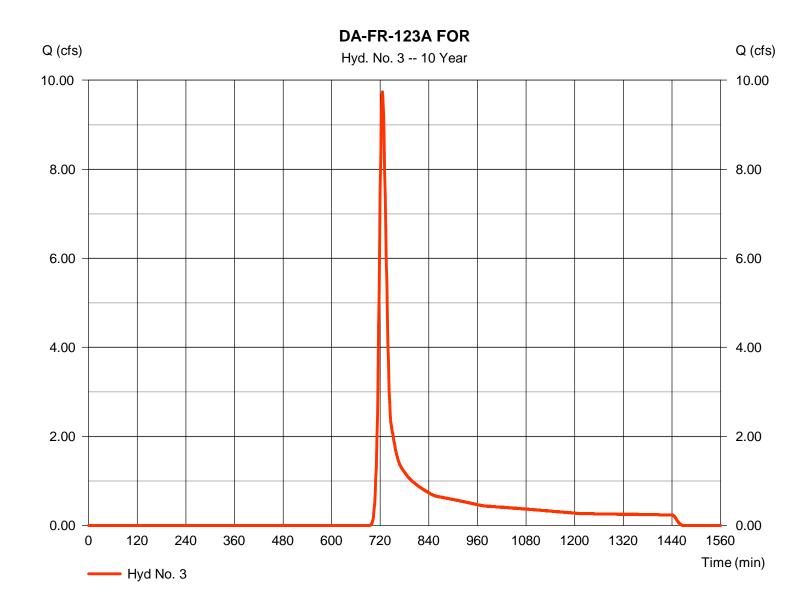
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-123A FOR

= SCS Runoff = 9.738 cfsHydrograph type Peak discharge Storm frequency Time to peak = 725 min = 10 yrsTime interval = 1 min Hyd. volume = 32.618 cuft Drainage area Curve number = 6.740 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 17.90 min Total precip. Distribution = 5.70 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)								
(Yrs)	В	D	E	(N/A)					
1	0.0000	0.0000	0.0000						
2	69.8703	13.1000	0.8658						
3	0.0000	0.0000	0.0000						
5	79.2597	14.6000	0.8369						
10	88.2351	15.5000	0.8279						
25	102.6072	16.5000	0.8217						
50	114.8193	17.2000	0.8199						
100	127.1596	17.8000	0.8186						

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return	Intensity Values (in/hr)											
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc

	Rainfall Precipitation Table (in)									
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr		
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00		
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		



ENERGY BALANCE METHOD

	1-Yr Event				
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)			
Pre-Developed Condition	0.643	2915			
Developed Condition	0.643	2915			
Pre-Developed (Forest) Condition	0.351	2173			

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF =

<u>Calculations:</u>	Check #1:	$Q_{developed} \le IF \times \{[Q_{pre-developed} \times RV_{pre-developed}] / RV_{developed}\}$	0.643	≤ OK	0.643
	Check #2:	$Q_{developed} \le Q_{pre-developed}$ >	0.643	≤ OK	0.643
	Check #3:	$Q_{developed} \underline{Shall not}$ be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed} \longrightarrow$	0.643	<u>shall not</u> be required to be ≤	0.262

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 – Manning's n Values for Open Channel Flow

nan	nel T	уре	Manning n			
			Min.	Normal	Max.	
1.	Exc	avated or Dredged Channels ¹				
	a.	Earth, Straight, and Uniform:				
		Clean, recently completed	0.016	0.018	0.020	
		Clean, after weathering	0.018	0.022	0.025	
		Gravel, uniform section, clean	0.022	0.025	0.030	
		With short grass, few weeds	0.022	0.027	0.033	
	b.	Earth Winding and Sluggish:				
		No vegetation	0.023	0.025	0.030	
		Grass, some weeds	0.025	0.030	0.033	
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040	
		Earth bottom and rubble sides	0.028	0.030	0.03	
		Stony bottom and weedy banks	0.025	0.035	0.040	
		Cobble bottom and clean sides	0.030	<mark>0.040</mark>	0.050	
	c.	Dragline-Excavated or Dredged:				
		No vegetation	0.025	0.028	0.03	
		Light brush on banks	0.035	0.050	0.060	
	d.	Rock Cuts:				
		Smooth and uniform	0.025	0.035	0.040	
		Jagged and irregular	0.035	0.040	0.050	
	e.	Channels not Maintained, Weeds and Brush Uncut:				
		Dense weeds, high as flow depth	0.050	0.080	0.120	
		Clean bottom, brush on sides	0.040	0.050	0.080	
		Same as above, highest stage of flow	0.045	0.070	0.110	
		Dense brush, high stage	0.080	0.100	0.140	
2.	Mai	n Channels²				
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.033	
	b.	Same as above, but more stones and weeds	0.030	0.035	0.040	
	C.	Clean, winding, some pools and shoals	0.033	0.040	0.04	
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050	
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.05	
	f.	Same as (d) with more stones	0.045	0.050	0.060	
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080	
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150	

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-123B PRE	
TR-55 Tc Worksheet	
Hydrograph No. 2, SCS Runoff, DA-FR-123B DEV	
TR-55 Tc Worksheet	
Hydrograph No. 3, SCS Runoff, DA-FR-123B FOR	
TR-55 Tc Worksheet	9
2 - Year	
Summary Report1	10
Hydrograph Reports 1	11
Hydrograph No. 1, SCS Runoff, DA-FR-123B PRE1	
Hydrograph No. 2, SCS Runoff, DA-FR-123B DEV1	
Hydrograph No. 3, SCS Runoff, DA-FR-123B FOR 1	13
10 - Year	
Summary Report 1	14
Hydrograph Reports1	
Hydrograph No. 1, SCS Runoff, DA-FR-123B PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-123B DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-123B FOR	
IDE Papart 1	ıΩ

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5 <u>Legend</u> Hyd. Origin **Description** SCS Runoff DA-FR-123B PRE 2 SCS Runoff DA-FR-123B DEV SCS Runoff DA-FR-123B FOR Monday, 08 / 21 / 2017 Project: DA-FR-123B_Hydraflow.gpw

Hydrograph Return Period Recap

		Inflow	Peak Outflow (cfs)								Hydrograph	
lo.	type (origin)	hyd(s)	1-yr 2-yr		3-yr 5-yr		10-yr	10-yr 25-yr 50-yr 10		100-yr	Description	
1	SCS Runoff		0.643	1.090			4.236				DA-FR-123B PRE	
2	SCS Runoff		0.643	1.090			4.236				DA-FR-123B DEV	
3	SCS Runoff		0.351	0.698			3.489				DA-FR-123B FOR	

Proj. file: DA-FR-123B_Hydraflow.gpw

Monday, 08 / 21 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	0.643	1	725	2,915				DA-FR-123B PRE	
2	SCS Runoff	0.643	1	725	2,915				DA-FR-123B DEV	
2										
DA	-FR-123B_Hy	draflow.g	lpw		Return F	Period: 1 Ye	ear	Monday, 08 / 21 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

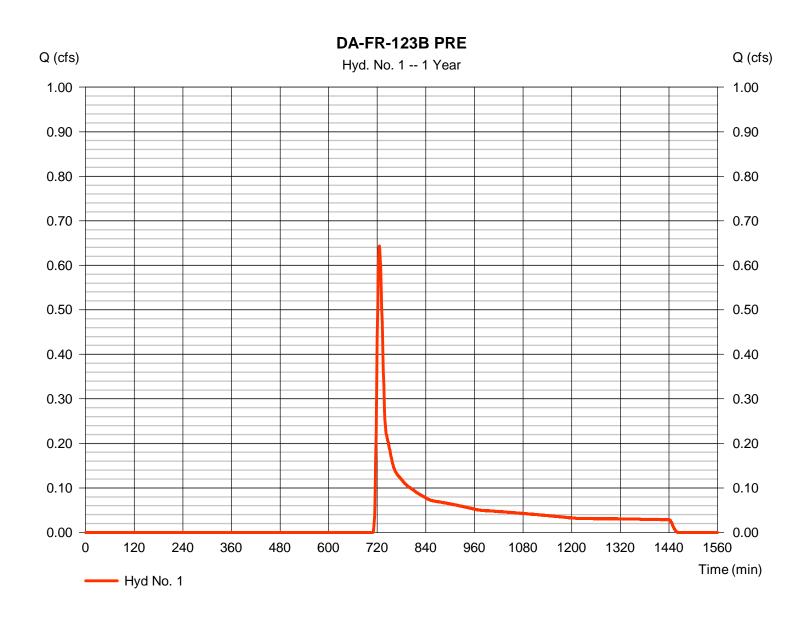
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-123B PRE

Hydrograph type = SCS Runoff Peak discharge = 0.643 cfsStorm frequency Time to peak = 1 yrs $= 725 \min$ Time interval = 1 minHyd. volume = 2,915 cuftDrainage area Curve number = 2.130 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.00 min Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.012 \times 82) + (0.037 \times 85) + (1.614 \times 58) + (0.013 \times 100) + (0.453 \times 55)] / 2.130$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1

DA-FR-123B PRE

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>		
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 4.63		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00				
Travel Time (min)	= 14.27	+	0.00	+	0.00	=	14.27		
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 218.27 = 10.78 = Unpaved =5.30	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00				
Travel Time (min)	= 0.69	+	0.00	+	0.00	=	0.69		
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015				
Flow length (ft)	({0})0.0		0.0		0.0				
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00		
Total Travel Time, Tc									

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

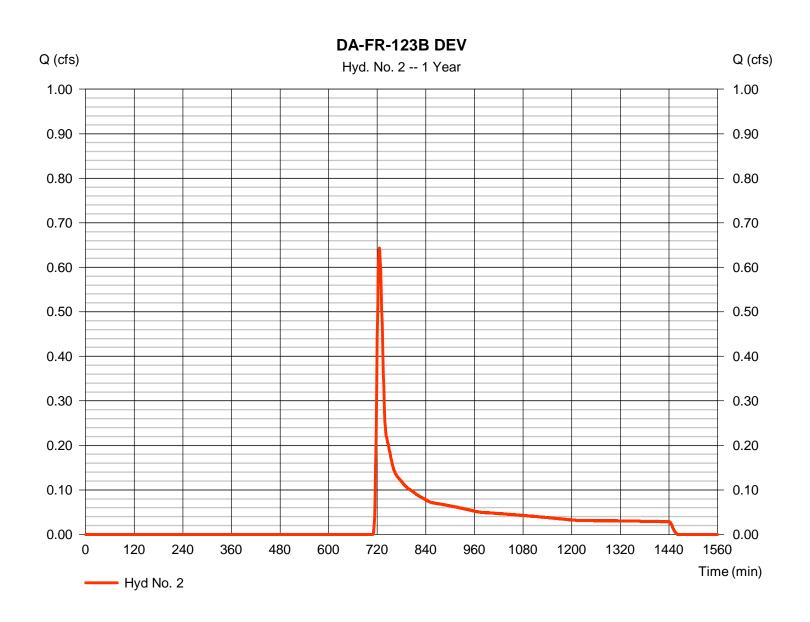
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-123B DEV

Hydrograph type = SCS Runoff Peak discharge = 0.643 cfsStorm frequency Time to peak = 1 yrs $= 725 \min$ Time interval = 1 minHyd. volume = 2.915 cuftDrainage area Curve number = 2.130 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 14.90 min Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.233 x 48) + (0.012 x 82) + (0.037 x 85) + (1.834 x 58) + (0.013 x 100)] / 2.130



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2

DA-FR-123B DEV

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>			
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 4.63		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00					
Travel Time (min)	= 14.27	+	0.00	+	0.00	=	14.27			
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 18.50 = 4.95 = Unpave =3.59	ed	135.07 14.47 Unpave 6.14	ed	0.00 0.00 Paved 0.00					
Travel Time (min)	= 0.09	+	0.37	+	0.00	=	0.45			
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015					
Flow length (ft)	({0})61.0		0.0		0.0					
Travel Time (min)	= 0.21	+	0.00	+	0.00	=	0.21			
Total Travel Time, Tc										

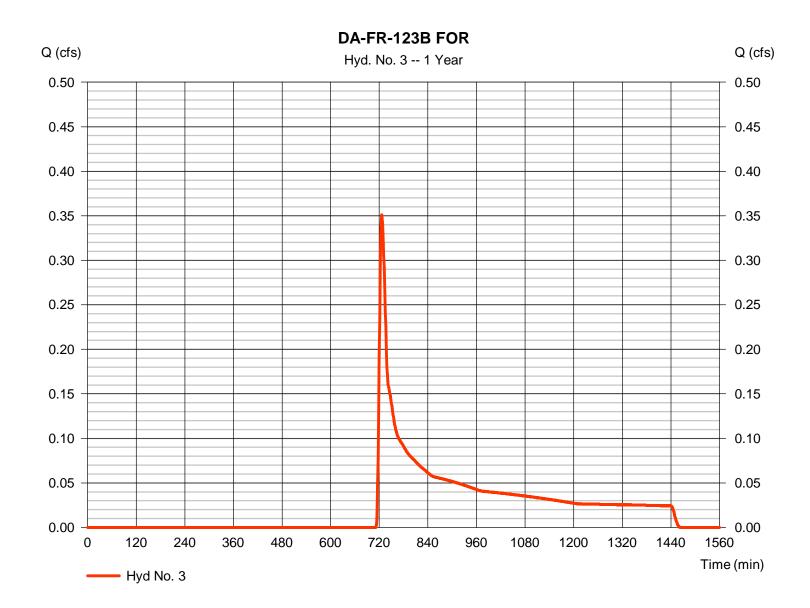
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-123B FOR

Hydrograph type = SCS Runoff Peak discharge = 0.351 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 min Hyd. volume = 2,173 cuftDrainage area Curve number = 2.130 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 15.00 min Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3

DA-FR-123B FOR

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>				
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 4.63 = 14.27	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	14.27				
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 218.27 = 10.78 = Unpaved =5.30	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00						
Travel Time (min)	= 0.69	+	0.00	+	0.00	=	0.69				
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015						
Flow length (ft)	({0})0.0		0.0		0.0						
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00				
Total Travel Time, Tc	Total Travel Time, Tc										

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	1.090	1	724	4,129				DA-FR-123B PRE	
2	SCS Runoff	1.090	1	724	4,129				DA-FR-123B DEV	
DA-FR-123B_Hydraflow.gpw					Return F	Period: 2 Ye	ear	Monday, 08 / 21 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

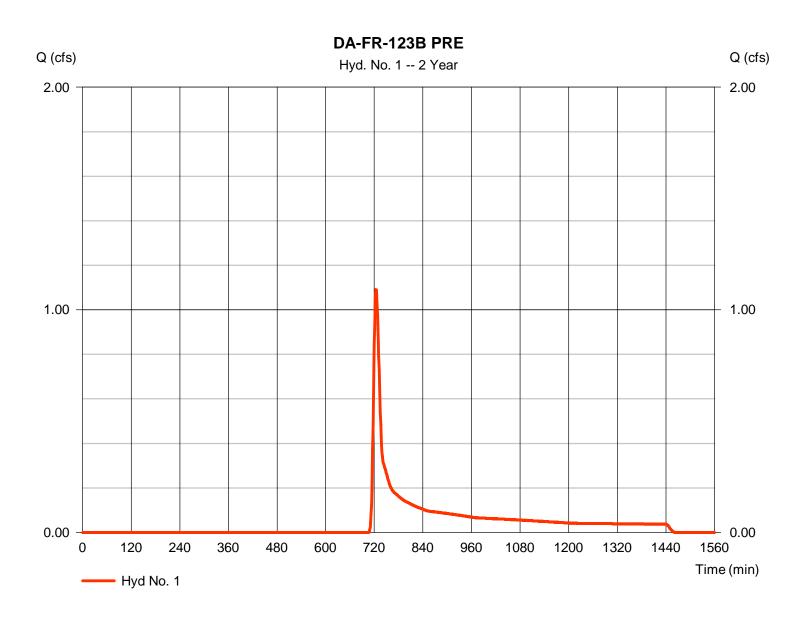
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-123B PRE

= SCS Runoff Hydrograph type Peak discharge = 1.090 cfsStorm frequency Time to peak = 724 min = 2 yrsTime interval = 1 minHyd. volume = 4,129 cuftDrainage area Curve number = 2.130 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.00 min Total precip. = 3.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.012 \times 82) + (0.037 \times 85) + (1.614 \times 58) + (0.013 \times 100) + (0.453 \times 55)] / 2.130$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

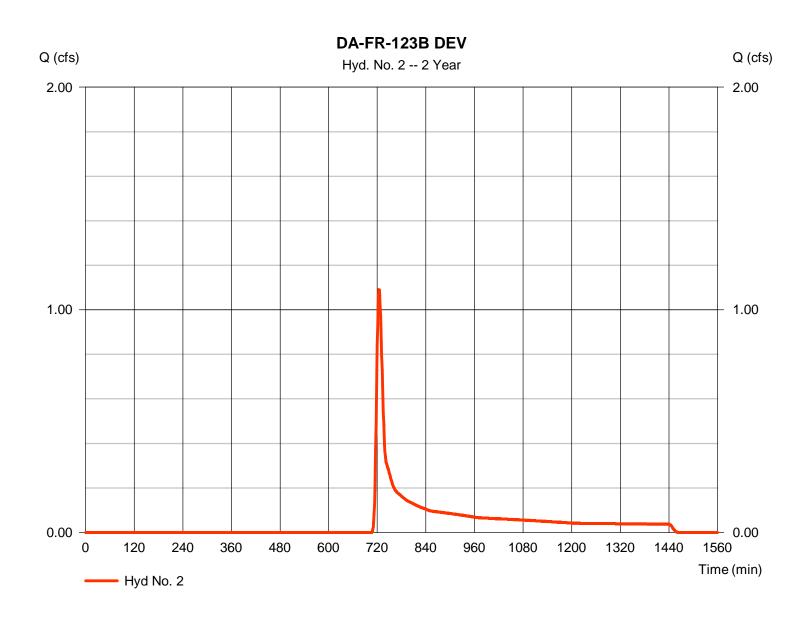
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-123B DEV

= SCS Runoff Hydrograph type Peak discharge = 1.090 cfsStorm frequency Time to peak = 724 min = 2 yrsTime interval = 1 minHyd. volume = 4,129 cuftDrainage area Curve number = 2.130 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 14.90 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.233 x 48) + (0.012 x 82) + (0.037 x 85) + (1.834 x 58) + (0.013 x 100)] / 2.130



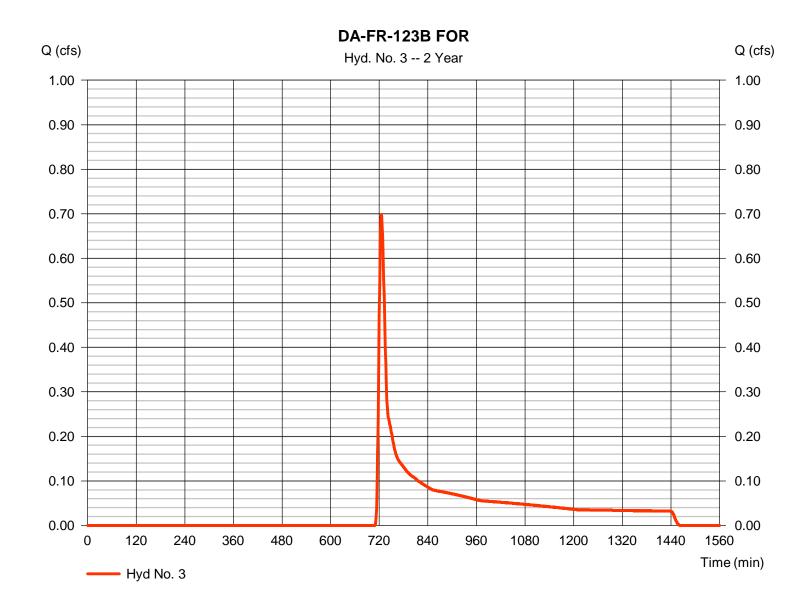
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-123B FOR

Hydrograph type = SCS Runoff Peak discharge = 0.698 cfsStorm frequency Time to peak = 725 min = 2 yrsTime interval = 1 min Hyd. volume = 3,213 cuftDrainage area Curve number = 2.130 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.00 min Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

1 SCS Runoff 4.236 1 723 12,161 DA-FR-123B PRE 2 SCS Runoff 4.236 1 723 12,161 DA-FR-123B DEV 3 SCS Runoff 3.489 1 723 10,427 DA-FR-123B FOR	Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
	1	SCS Runoff	4.236	1	723	12,161				DA-FR-123B PRE
3 SCS Runoff 3.489 1 723 10,427 DA-FR-123B FOR	2	SCS Runoff	4.236	1	723	12,161				DA-FR-123B DEV
	2	SCS Runoff	4.236	1	723	12,161				DA-FR-123B DEV
DA-FR-123B_Hydraflow.gpw Return Period: 10 Year Monday, 08 / 21 / 2017										

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

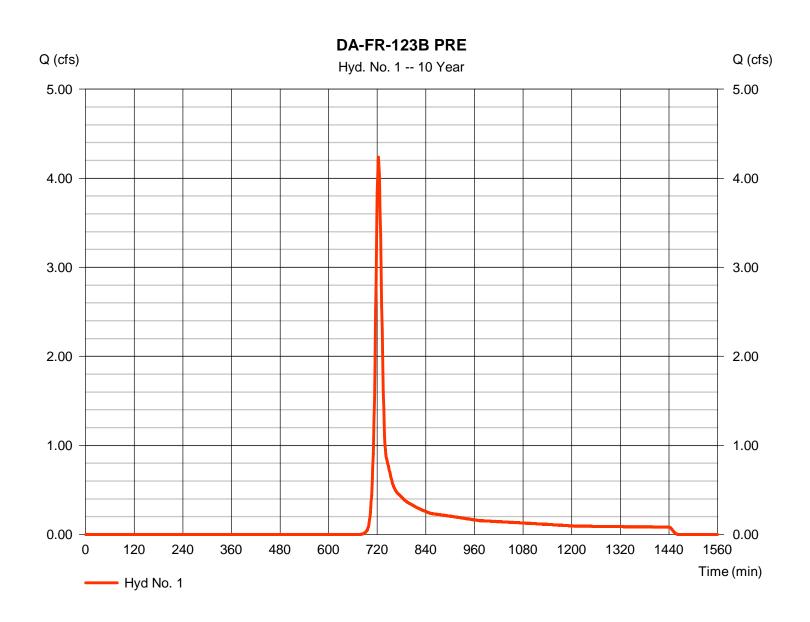
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-123B PRE

= SCS Runoff Hydrograph type Peak discharge = 4.236 cfsStorm frequency Time to peak = 723 min = 10 yrsTime interval = 1 min Hyd. volume = 12.161 cuft Drainage area = 2.130 acCurve number = 58* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.00 min Total precip. = 5.70 inDistribution = Type II Storm duration = 484 $= 24 \, hrs$ Shape factor

^{*} Composite (Area/CN) = $[(0.012 \times 82) + (0.037 \times 85) + (1.614 \times 58) + (0.013 \times 100) + (0.453 \times 55)] / 2.130$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

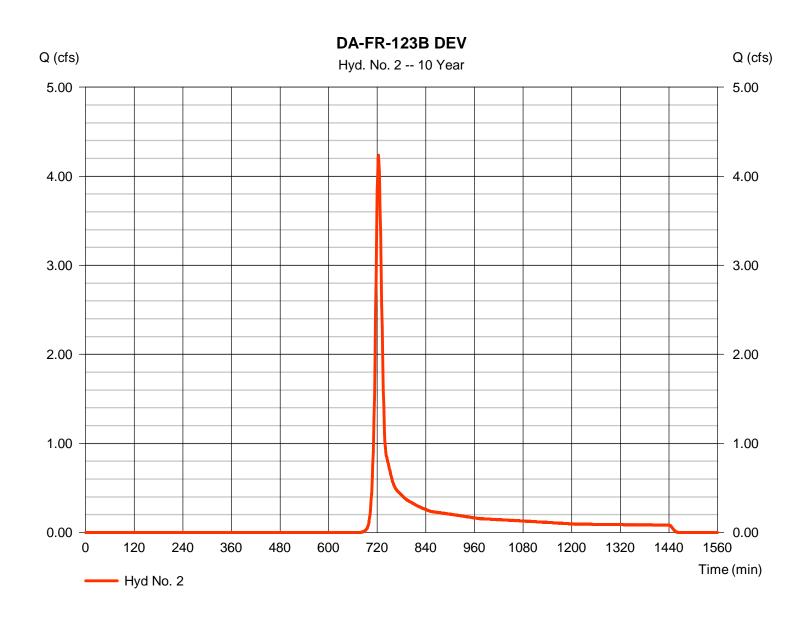
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-123B DEV

= SCS Runoff Hydrograph type Peak discharge = 4.236 cfsStorm frequency Time to peak = 723 min = 10 yrsTime interval = 1 min Hyd. volume = 12.161 cuft Drainage area = 2.130 acCurve number = 58* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 14.90 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration = 484 $= 24 \, hrs$ Shape factor

^{*} Composite (Area/CN) = [(0.233 x 48) + (0.012 x 82) + (0.037 x 85) + (1.834 x 58) + (0.013 x 100)] / 2.130



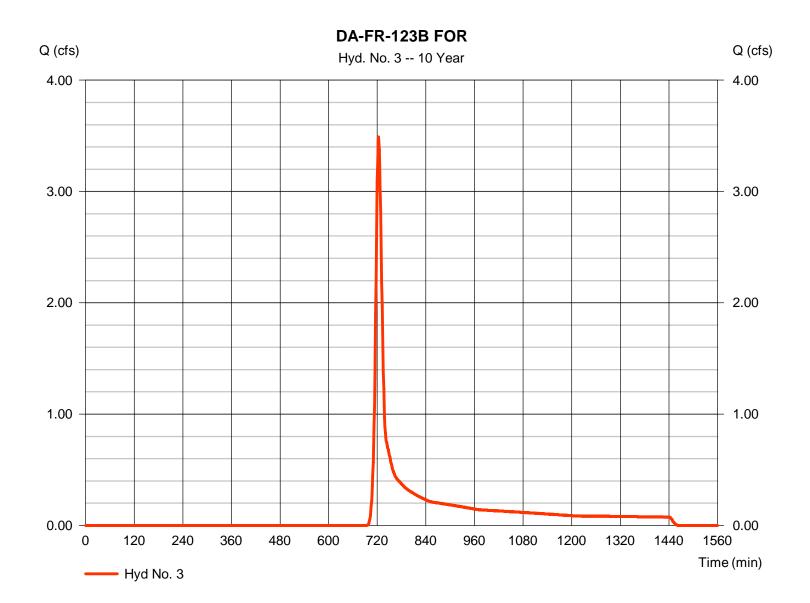
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-123B FOR

= SCS Runoff Hydrograph type Peak discharge = 3.489 cfsStorm frequency Time to peak = 723 min = 10 yrsTime interval = 1 min Hyd. volume = 10,427 cuftDrainage area Curve number = 2.130 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.00 min Total precip. Distribution = 5.70 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)										
(Yrs)	В	D	E	(N/A)							
1	0.0000	0.0000	0.0000								
2	69.8703	13.1000	0.8658								
3	0.0000	0.0000	0.0000								
5	79.2597	14.6000	0.8369								
10	88.2351	15.5000	0.8279								
25	102.6072	16.5000	0.8217								
50	114.8193	17.2000	0.8199								
100	127.1596	17.8000	0.8186								

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return Period (Yrs)		Intensity Values (in/hr)												
	5 min	10	15	20	25	30	35	40	45	50	55	60		
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70		
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15		
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46		
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91		
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25		
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60		

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc

		Rainfall Precipitation Table (in)										
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr				
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00				
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				

DA-FR-124

DA-FR-124 is located in a meadow and forested areas with hilly slopes. No new impervious area is proposed within DA-FR-124. The total phosphorus load reduction required for DA-FR-124 is -0.62 lb/yr. Multiple points of analysis were evaluated within DA-FR-124 to evaluate the effects on each receiving stream/channel following construction. Specifically, DA-FR-124 was sub-divided into two sub-drainage areas (sub areas A and B).

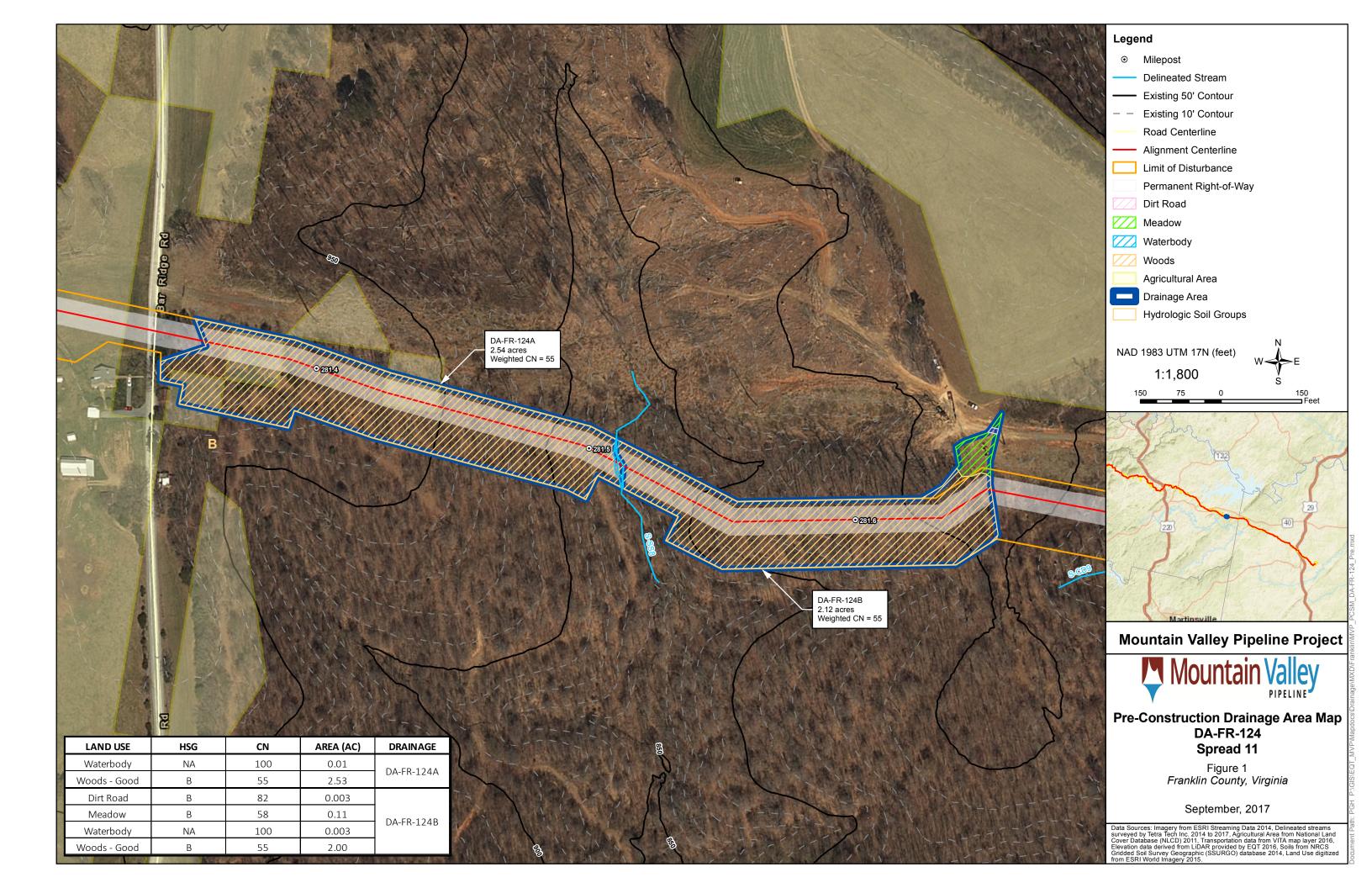
Sub-area 124A contains both agricultural and non-agricultural areas within the limits of disturbance (LOD). Pre-construction agricultural areas will be returned to agricultural land use (i.e., returned to crop production, in identical condition) following construction. In non-agricultural areas, land use will be restored following construction as noted in the Stormwater Management (SWM) Narrative and the Annual Standards and Specifications. Agricultural areas within the LOD are included in the SWM quality analysis and the total permanent Right of Way (ROW) is analyzed via VRRM; in these calculations agricultural areas are considered "Forest/Open Space".

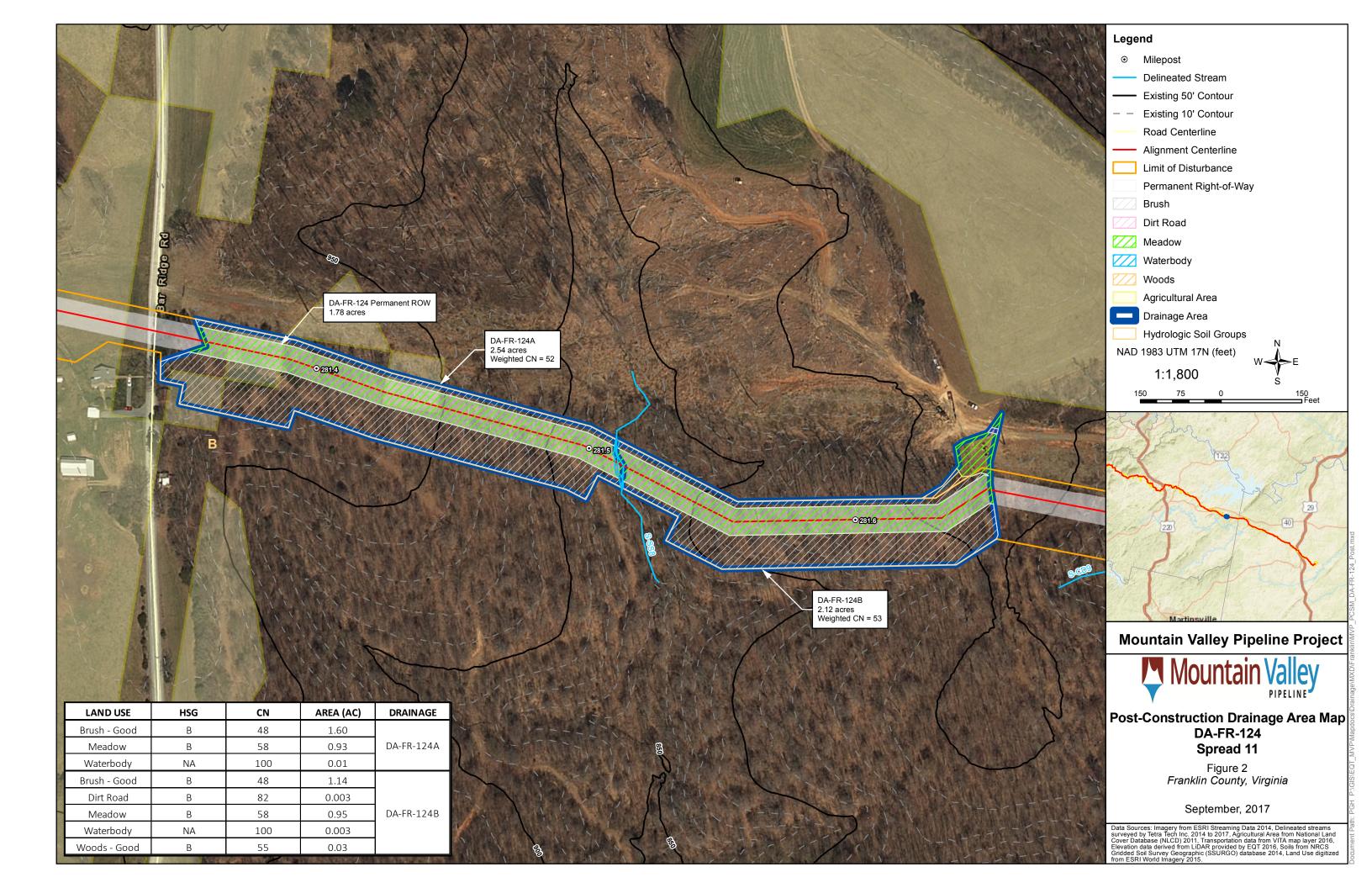
Stormwater quantity is met via the energy balance method for each of the two subareas DA-FR-124A and DA-FR-124B. Agricultural areas within the study area are included in the SWM analysis, but an Improvement Factor (IF) of 1.0 is used when applying the Energy Balance Method. This improvement factor is used to account for the exemption of agricultural areas (§ 62.1-44.15:34 and 9VAC25-870-300) since such areas will be returned to agricultural land use (i.e., returned to crop production, in identical condition) following construction.

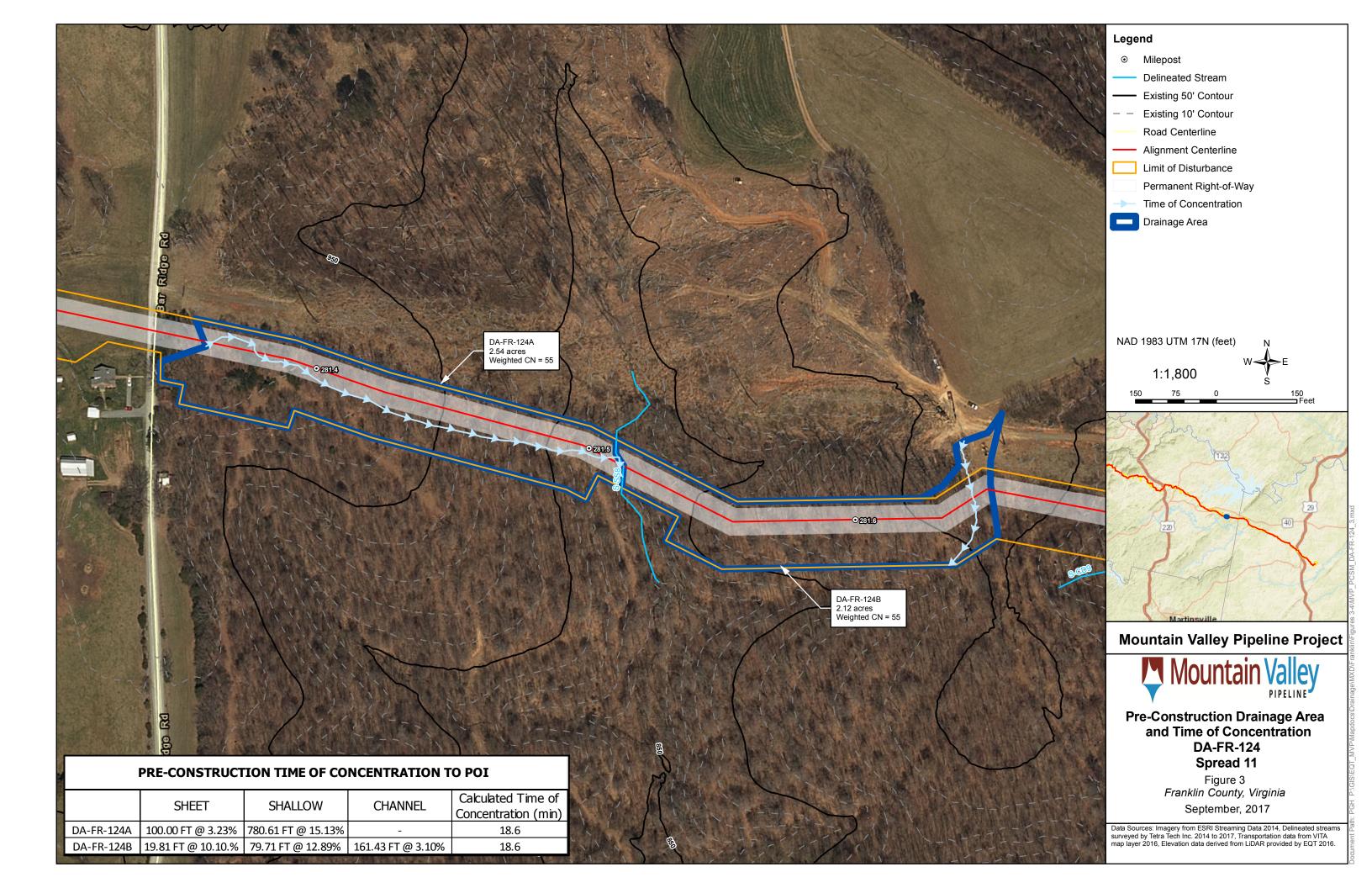
In addition, the Hydraflow Hydrograph's 10-year 24-hour peak discharge results indicate a reduction in flows ranging from 0.67 to 0.93 cfs for all drainage areas (as seen in table below).

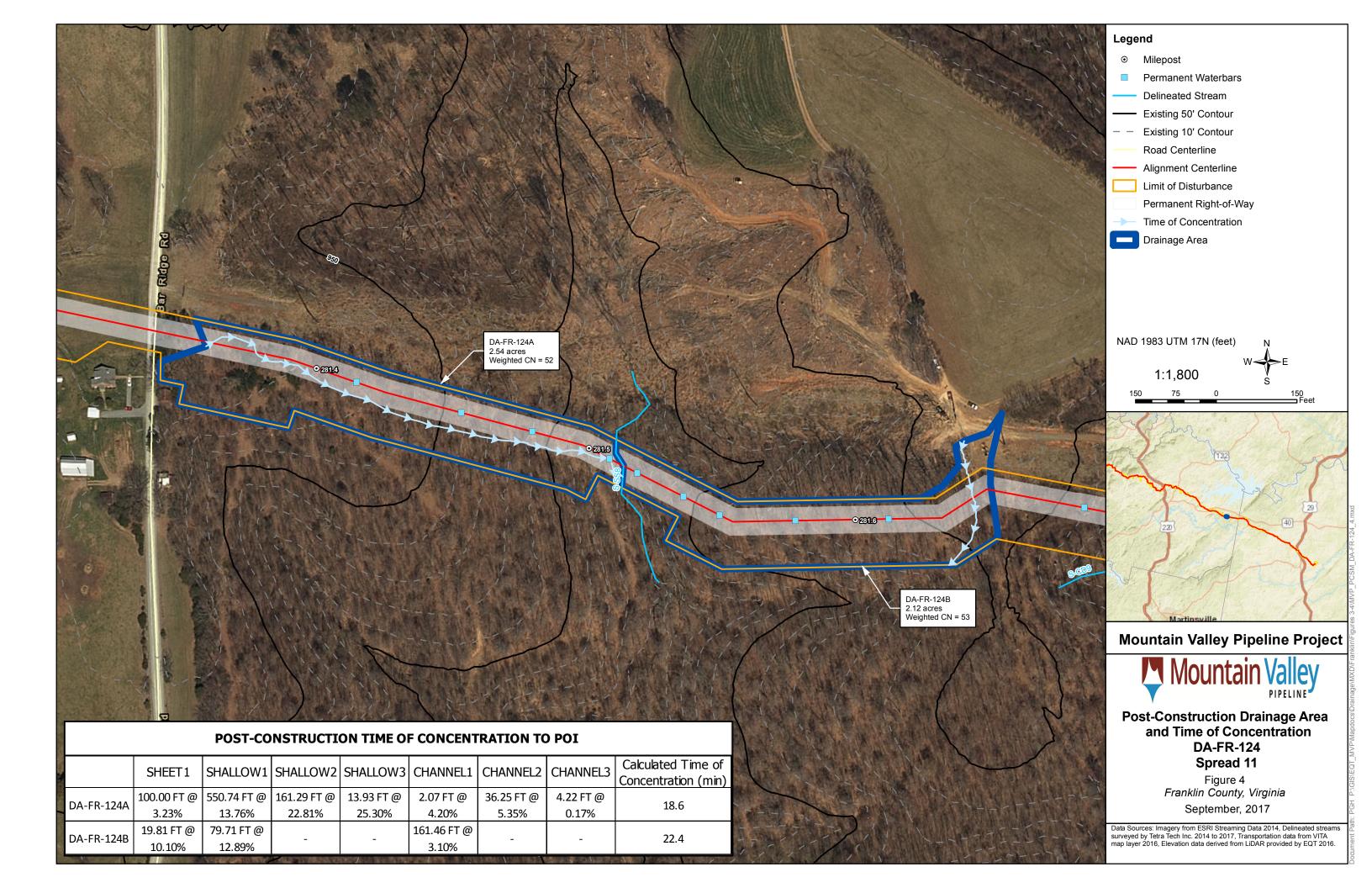
Sub Area	Pre Peak Flow,	Post Peak Flow, Q	Flow differential
	10-yr Q (cfs)	10-yr (cfs)	
DA-FR-124A	3.67	2.74	-0.93
DA-FR-124B	2.91	2.24	-0.67

Figures and calculations for each of the sub-areas for DA-FR-124 follow. See Appendix D of the Annual Standards and Specifications for further detail on stormwater methodology.









Virginia Runoff Reduction Method New Development Worksheet - v2.8 - June 2014

Site Data Summary

Total Rainfall = 45 inches

Site Land Cover Summary

	A Soils	B Soils	C Soils	D Soils	Total	% of Total
Forest (acres)	0.00	1.78	0.07	0.00	1.85	100.00
Turf (acres)	0.00	0.00	0.00	0.00	0.00	0.00
Impervious (acres)	0.00	0.00	0.00	0.00	0.00	0.00
					1.85	100.00

Site Rv	0.03
Post Development Treatment Volume (ft3)	204
Post Development TP Load (lb/yr)	0.13
Post Development TN Load (lb/yr)	0.96
Total TP Load Reduction Required (lb/yr)	-0.62

Total Runoff Volume Reduction (ft ³)	0
Total TP Load Reduction Achieved (lb/yr)	0
Total TN Load Reduction Achieved (lb/yr)	0.00
Adjusted Post Development TP Load (lb/yr)	0.13
Remaining Phosphorous Load Reduction (Lb/yr) Required	0.00

Drainage Area Summary

	D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	Total
Forest (acres)	1.78	0.00	0.00	0.00	0.00	1.78
Turf (acres)	0.00	0.00	0.00	0.00	0.00	0.00
Impervious (acres)	0.00	0.00	0.00	0.00	0.00	0.00
						1.78

Drainage Area Compliance Summary

	D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	Total
TP Load Red. (lb/yr)	0.00	0.00	0.00	0.00	0.00	0.00
TN Load Red. (lb/yr)	0.00	0.00	0.00	0.00	0.00	0.00



ENERGY BALANCE METHOD

		ts

	1-Y	r Event
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)
Pre-Developed Condition	0.365	2562
Developed Condition	0.149	1823
Pre-Developed (Forest) Condition	0.365	2562

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.8

<u>Calculations:</u>	Check #1:	$Q_{\text{developed}} \leq \text{IF x } \left[\left\{ Q_{\text{pre-developed}} \times \text{RV}_{\text{pre-developed}} \right\} / \text{RV}_{\text{developed}} \right] $	0.149	≤ N/A - See Check #3	0.410
	Check #2:	Q _{developed} ≤ Q _{pre-developed} →	0.149	≤ N/A - See Check #3	0.365
	Check #3:	$Q_{developed} \frac{shall\ not}{shall\ not}$ be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed}$ >	0.149	shall not be required to be ≤	0.513

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	·
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 – Manning's n Values for Open Channel Flow

nan	nel T	уре	Manning n				
			Min.	Normal	Max.		
1.	Exc	avated or Dredged Channels ¹					
	a.	Earth, Straight, and Uniform:					
		Clean, recently completed	0.016	0.018	0.020		
		Clean, after weathering	0.018	0.022	0.025		
		Gravel, uniform section, clean	0.022	0.025	0.030		
		With short grass, few weeds	0.022	0.027	0.033		
	b.	Earth Winding and Sluggish:					
		No vegetation	0.023	0.025	0.030		
		Grass, some weeds	0.025	0.030	0.033		
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040		
		Earth bottom and rubble sides	0.028	0.030	0.03		
		Stony bottom and weedy banks	0.025	0.035	0.040		
		Cobble bottom and clean sides	0.030	<mark>0.040</mark>	0.050		
	c.	Dragline-Excavated or Dredged:					
		No vegetation	0.025	0.028	0.03		
		Light brush on banks	0.035	0.050	0.060		
	d.	Rock Cuts:					
		Smooth and uniform	0.025	0.035	0.040		
		Jagged and irregular	0.035	0.040	0.050		
	e.	Channels not Maintained, Weeds and Brush Uncut:					
		Dense weeds, high as flow depth	0.050	0.080	0.120		
		Clean bottom, brush on sides	0.040	0.050	0.080		
		Same as above, highest stage of flow	0.045	0.070	0.110		
		Dense brush, high stage	0.080	0.100	0.140		
2.	Mai	n Channels²					
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.033		
	b.	Same as above, but more stones and weeds	0.030	0.035	0.040		
	C.	Clean, winding, some pools and shoals	0.033	0.040	0.04		
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050		
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.05		
	f.	Same as (d) with more stones	0.045	0.050	0.060		
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080		
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150		

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

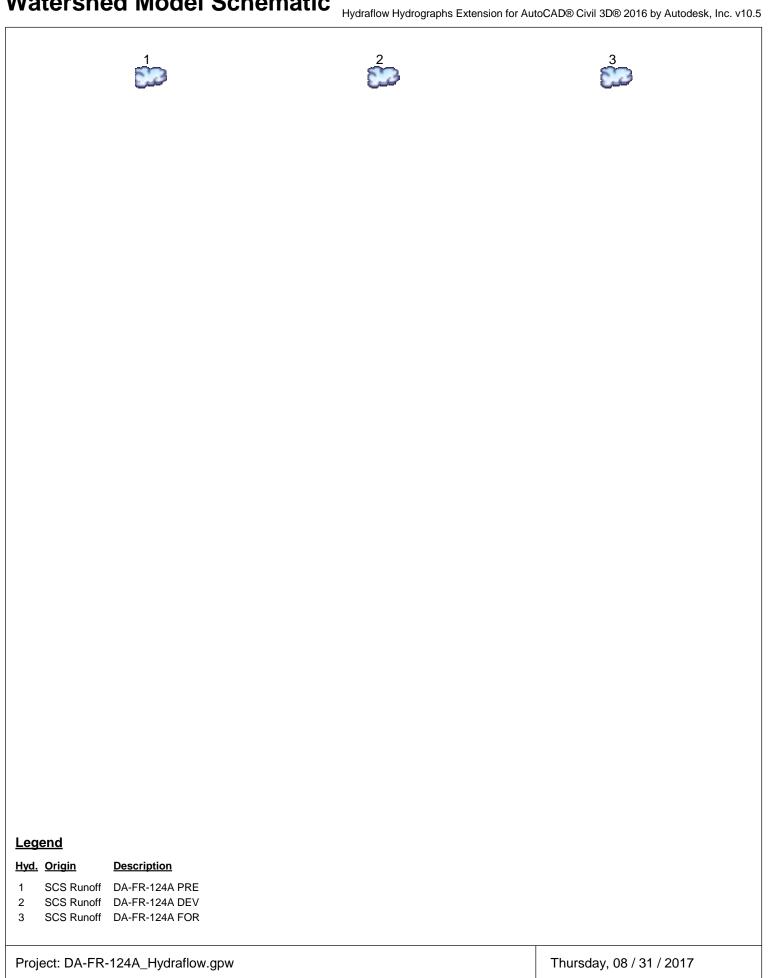
-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Thursday, 08 / 31 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-124A PRE	4
TR-55 Tc Worksheet	5
Hydrograph No. 2, SCS Runoff, DA-FR-124A DEV	6
TR-55 Tc Worksheet	
Hydrograph No. 3, SCS Runoff, DA-FR-124A FOR	
TR-55 Tc Worksheet	9
2 - Year	
Summary Report	10
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-124A PRE	11
Hydrograph No. 2, SCS Runoff, DA-FR-124A DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-124A FOR	
10 - Year	
Summary Report	14
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-124A PRE	15
Hydrograph No. 2, SCS Runoff, DA-FR-124A DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-124A FOR	
IDF Report	18



Hydrograph Return Period Recap Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	Hydrograph	Inflow	Peak Outflow (cfs)								Hydrograph	
No.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description	
1	SCS Runoff		0.365	0.723			3.670				DA-FR-124A PRE	
2	SCS Runoff		0.149	0.374			2.735				DA-FR-124A DEV	
2 3	SCS Runoff SCS Runoff		0.149	0.374			2.735 3.670				DA-FR-124A FOR	

Proj. file: DA-FR-124A_Hydraflow.gpw

Thursday, 08 / 31 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description		
1	SCS Runoff	0.365	1	729	2,562				DA-FR-124A PRE		
2	SCS Runoff	0.149	1	736	1,823				DA-FR-124A DEV		
3	SCS Runoff	0.365	1	729	2,562				DA-FR-124A FOR		
DA	-FR-124A_H	ydraflow.g	gpw		Return F	Period: 1 Ye	ear	Thursday, 0	Thursday, 08 / 31 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

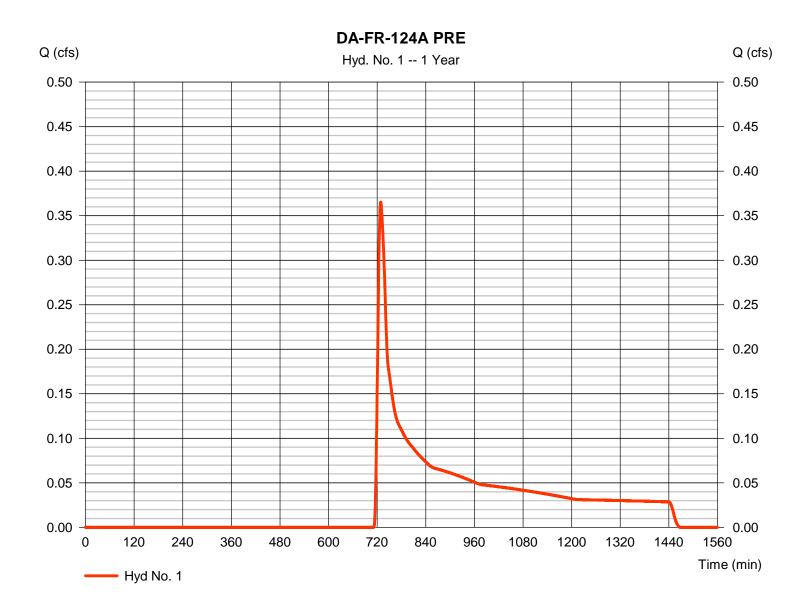
Thursday, 08 / 31 / 2017

Hyd. No. 1

DA-FR-124A PRE

Hydrograph type = SCS Runoff Peak discharge = 0.365 cfsStorm frequency Time to peak = 729 min = 1 yrsTime interval = 1 minHyd. volume = 2.562 cuftDrainage area Curve number = 2.540 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 18.50 \, \text{min}$ = TR55 Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.009 \times 100) + (2.532 \times 55)] / 2.540$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1 DA-FR-124A PRE

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 3.23 = 16.49	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	16.49
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 741.93 = 15.70 = Unpaved =6.39		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.93	+	0.00	+	0.00	=	1.93
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.93 = 7.93 = 4.32 = 0.030 =5.30		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})38.7		0.0		0.0		
Travel Time (min)	= 0.12	+	0.00	+	0.00	=	0.12
Total Travel Time, Tc							18.50 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

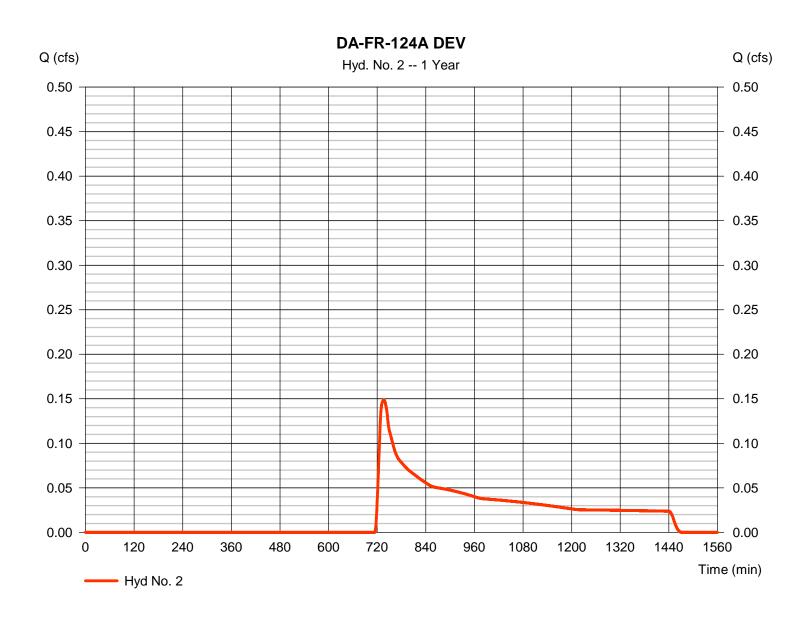
Thursday, 08 / 31 / 2017

Hyd. No. 2

DA-FR-124A DEV

Hydrograph type = SCS Runoff Peak discharge = 0.149 cfsStorm frequency Time to peak = 736 min = 1 yrsTime interval = 1 minHyd. volume = 1.823 cuftDrainage area Curve number = 2.540 ac= 52*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 18.60 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(1.603 x 48) + (0.929 x 58) + (0.009 x 100)] / 2.540



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2

DA-FR-124A DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>		
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 3.23 = 16.49	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	16.49		
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 550.74 = 13.76 = Unpaved =5.99	d	161.29 22.81 Unpave 7.71	d	13.93 25.30 Unpave 8.12	ed			
Travel Time (min)	= 1.53	+	0.35	+	0.03	=	1.91		
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		2.00 4.47 5.00 0.040 4.86		2.93 7.93 0.17 0.030				
Flow length (ft)	({0})2.1		36.3		4.2				
Travel Time (min)	= 0.01	+	0.12	+	0.07	=	0.20		
Total Travel Time, Tc									

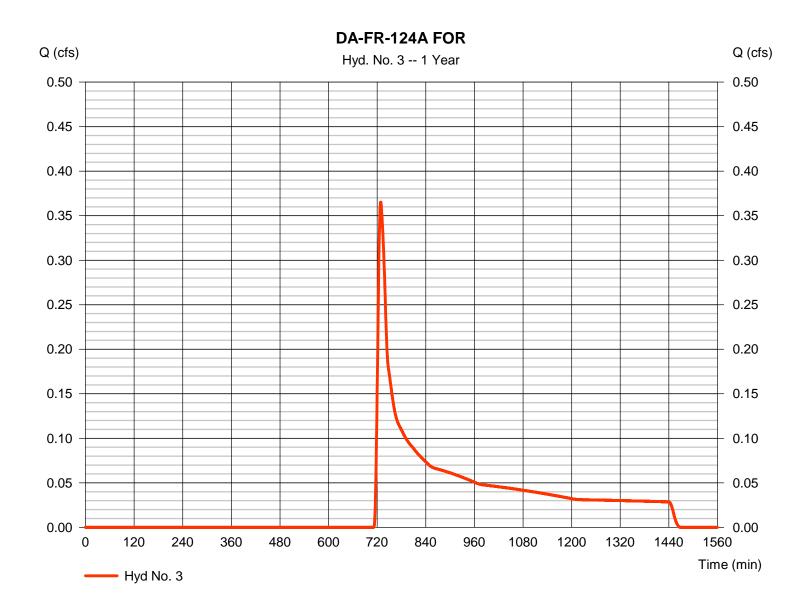
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Thursday, 08 / 31 / 2017

Hyd. No. 3

DA-FR-124A FOR

Hydrograph type = SCS Runoff Peak discharge = 0.365 cfsStorm frequency Time to peak = 729 min = 1 yrsTime interval = 1 min Hyd. volume = 2.562 cuftDrainage area Curve number = 2.540 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 18.50 \, \text{min}$ = TR55 Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3

DA-FR-124A FOR

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>			
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 3.23 = 16.49	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	16.49			
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 741.93 = 15.70 = Unpaved =6.39	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00					
Travel Time (min)	= 1.93	+	0.00	+	0.00	=	1.93			
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.93 = 7.93 = 4.32 = 0.030 =5.30		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015					
Flow length (ft)	({0})38.7		0.0		0.0					
Travel Time (min)	= 0.12	+	0.00	+	0.00	=	0.12			
Total Travel Time, Tc										

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

yd. o.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.723	1	727	3,788				DA-FR-124A PRE
2	SCS Runoff	0.374	1	730	2,858				DA-FR-124A DEV
2 3									
)Δ.	 -FR-124A_H _!	udraflow c	ınw.		Poturo I	Period: 2 Ye		Thursday	 08 / 31 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

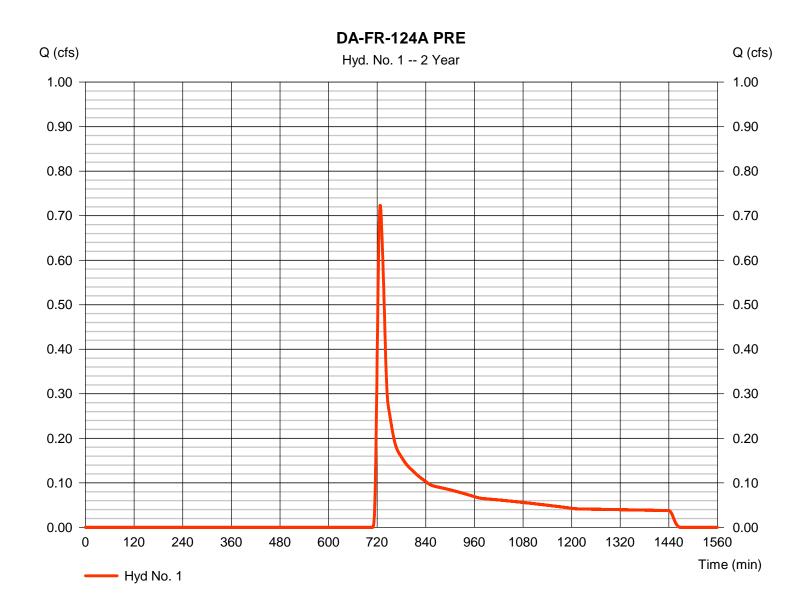
Thursday, 08 / 31 / 2017

Hyd. No. 1

DA-FR-124A PRE

Hydrograph type = SCS Runoff Peak discharge = 0.723 cfsStorm frequency = 2 yrsTime to peak = 727 min Time interval = 1 minHyd. volume = 3.788 cuftDrainage area Curve number = 2.540 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 18.50 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.009 x 100) + (2.532 x 55)] / 2.540



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

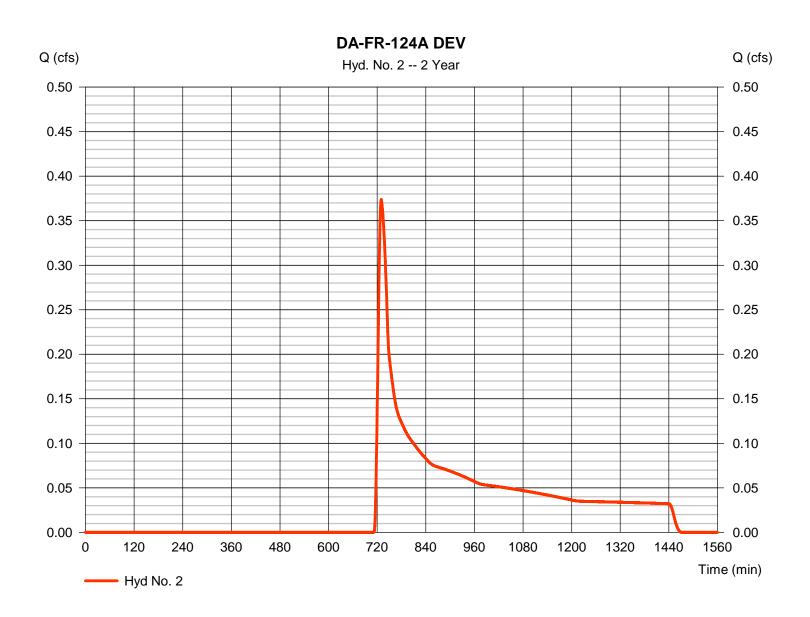
Thursday, 08 / 31 / 2017

Hyd. No. 2

DA-FR-124A DEV

Hydrograph type = SCS Runoff Peak discharge = 0.374 cfsStorm frequency = 2 yrsTime to peak = 730 min Time interval = 1 minHyd. volume = 2.858 cuftDrainage area Curve number = 2.540 ac= 52*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 18.60 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(1.603 x 48) + (0.929 x 58) + (0.009 x 100)] / 2.540



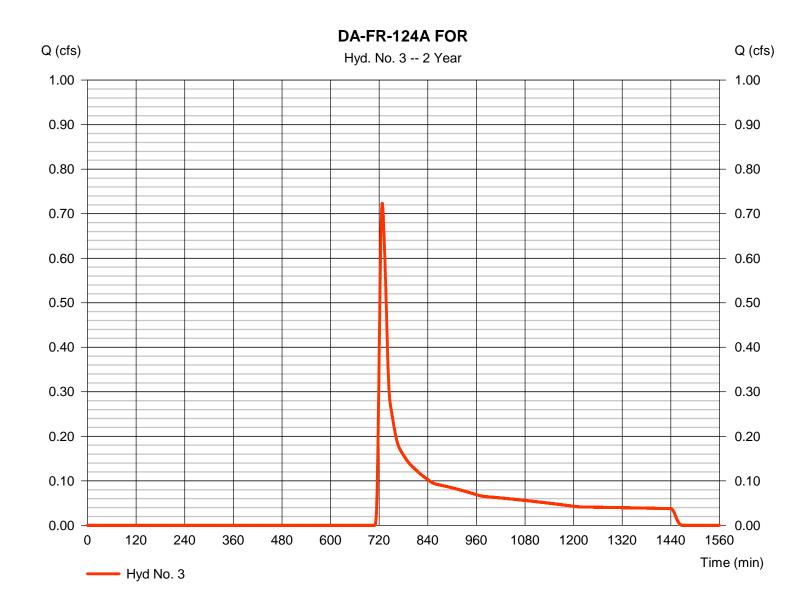
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Thursday, 08 / 31 / 2017

Hyd. No. 3

DA-FR-124A FOR

Hydrograph type = SCS Runoff Peak discharge = 0.723 cfsStorm frequency = 2 yrsTime to peak = 727 min Time interval = 1 min Hyd. volume = 3.788 cuftDrainage area Curve number = 2.540 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 18.50 \, \text{min}$ = TR55 Total precip. = 3.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

lyd. Io.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	3.670	1	725	12,292				DA-FR-124A PRE
2	SCS Runoff	2.735	1	727	10,465				DA-FR-124A DEV
1 2 3									
DΑ	⊥ -FR-124A_H <u>y</u>	ydraflow.g	Jpw		Return F	Period: 10 Y	⊥ ∕ear	Thursday, (08 / 31 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

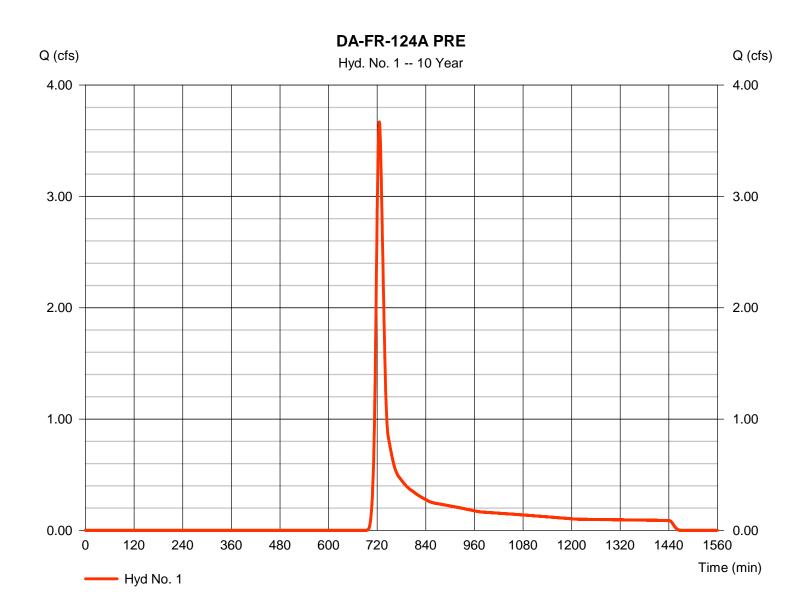
Thursday, 08 / 31 / 2017

Hyd. No. 1

DA-FR-124A PRE

Hydrograph type = SCS Runoff Peak discharge = 3.670 cfsStorm frequency = 10 yrsTime to peak = 725 min Time interval = 1 minHyd. volume = 12.292 cuft Drainage area Curve number = 2.540 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 18.50 \, \text{min}$ = TR55 Total precip. = 5.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.009 x 100) + (2.532 x 55)] / 2.540



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

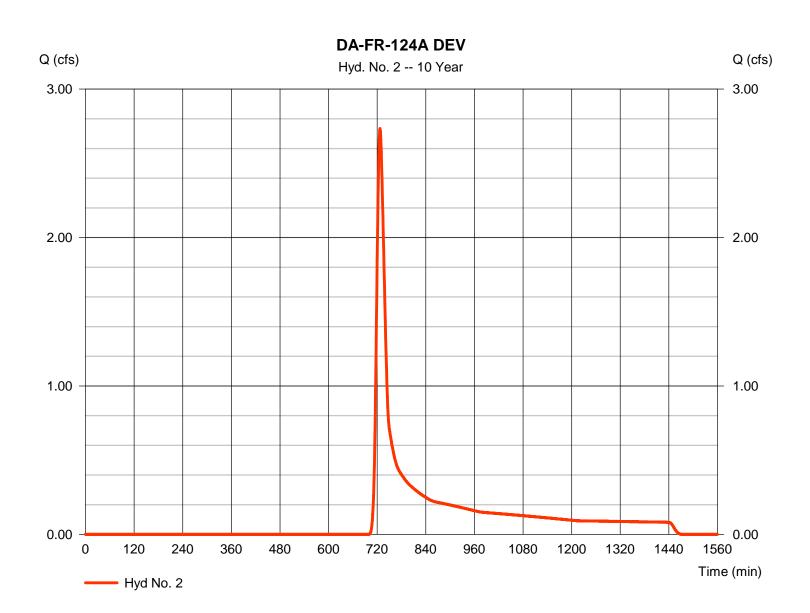
Thursday, 08 / 31 / 2017

Hyd. No. 2

DA-FR-124A DEV

Hydrograph type = SCS Runoff Peak discharge = 2.735 cfsStorm frequency = 10 yrsTime to peak = 727 min Time interval = 1 minHyd. volume = 10.465 cuftCurve number Drainage area = 2.540 ac= 52*Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) = 18.60 min Tc method = TR55 Total precip. Distribution = Type II = 5.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(1.603 x 48) + (0.929 x 58) + (0.009 x 100)] / 2.540



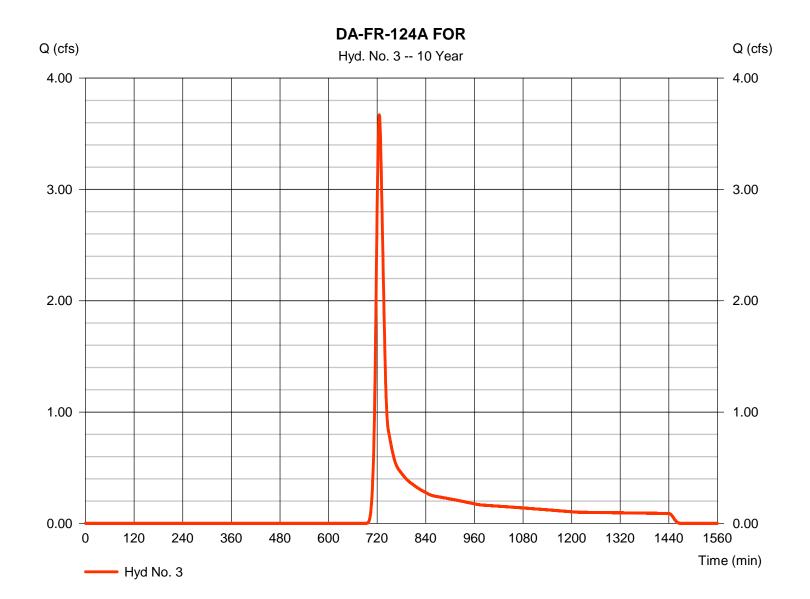
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Thursday, 08 / 31 / 2017

Hyd. No. 3

DA-FR-124A FOR

Hydrograph type = SCS Runoff Peak discharge = 3.670 cfsStorm frequency = 10 yrsTime to peak = 725 min Time interval = 1 minHyd. volume = 12,292 cuft Drainage area Curve number = 2.540 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 18.50 \, \text{min}$ = TR55 Total precip. = 5.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Thursday, 08 / 31 / 2017

Return Period	Intensity-Du	Intensity-Duration-Frequency Equation Coefficients (FHA)									
(Yrs)	В	D	E	(N/A)							
1	0.0000	0.0000	0.0000								
2	69.8703	13.1000	0.8658								
3	0.0000	0.0000	0.0000								
5	79.2597	14.6000	0.8369								
10	88.2351	15.5000	0.8279								
25	102.6072	16.5000	0.8217								
50	114.8193	17.2000	0.8199								
100	127.1596	17.8000	0.8186								

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return		Intensity Values (in/hr)													
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60			
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70			
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15			
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46			
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91			
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25			
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60			

Tc = time in minutes. Values may exceed 60.

ling\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.p

		R	ainfall P	recipitat	ion Tab	le (in)		
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00



ENERGY BALANCE METHOD

		ts

	1-Y	r Event
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)
Pre-Developed Condition	0.289	2163
Developed Condition	0.156	1710
Pre-Developed (Forest) Condition	0.250	2163

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.8

<u>Calculations:</u>	Check #1:	$Q_{\text{developed}} \leq \text{IF x } \left[\left\{ Q_{\text{pre-developed}} \times \text{RV}_{\text{pre-developed}} \right\} / \text{RV}_{\text{developed}} \right] $	0.156	≤ N/A - See Check #3	0.292
	Check #2:	Q _{developed} ≤ Q _{pre-developed} →	0.156	≤ N/A - See Check #3	0.289
	Check #3:	$Q_{developed} \frac{shall\ not}{shall\ not}$ be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed}$ >	0.156	shall not be required to be ≤	0.316

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 - 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 - 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 - 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 - Manning's n Values for Open Channel Flow

hannel Type				Manning n				
			Min.	Normal	Max.			
1.	Exc	avated or Dredged Channels ¹						
	a.	Earth, Straight, and Uniform:						
		Clean, recently completed	0.016	0.018	0.020			
		Clean, after weathering	0.018	0.022	0.025			
		Gravel, uniform section, clean	0.022	0.025	0.030			
		With short grass, few weeds	0.022	0.027	0.033			
	b.	Earth Winding and Sluggish:						
		No vegetation	0.023	0.025	0.030			
		Grass, some weeds	0.025	0.030	0.033			
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040			
		Earth bottom and rubble sides	0.028	0.030	0.035			
		Stony bottom and weedy banks	0.025	0.035	0.040			
		Cobble bottom and clean sides	0.030	0.040	0.050			
	c.	Dragline-Excavated or Dredged:		•				
(No vegetation	0.025	0.028	0.033			
		Light brush on banks	0.035	0.050	0.060			
	d.	Rock Cuts:		•				
		Smooth and uniform	0.025	0.035	0.040			
		Jagged and irregular	0.035	0.040	0.050			
	e.	Channels not Maintained, Weeds and Brush Uncut:		•				
		Dense weeds, high as flow depth	0.050	0.080	0.120			
		Clean bottom, brush on sides	0.040	0.050	0.080			
		Same as above, highest stage of flow	0.045	0.070	0.110			
		Dense brush, high stage	0.080	0.100	0.140			
2.	Mai	n Channels²		•				
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.033			
	b.	Same as above, but more stones and weeds	0.030	0.035	0.040			
	c.	Clean, winding, some pools and shoals	0.033	0.040	0.045			
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050			
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.055			
	f.	Same as (d) with more stones	0.045	0.050	0.060			
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080			
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150			

Notes

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs.

Sources:

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

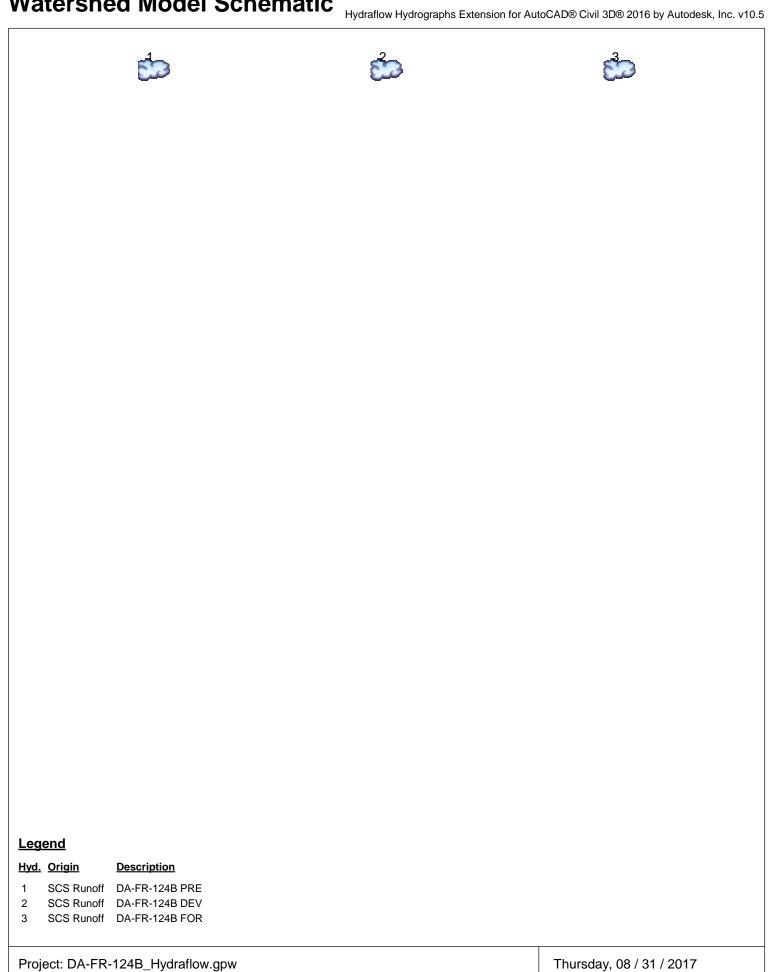
-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Thursday, 08 / 31 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-124B PRE	
TR-55 Tc Worksheet	
Hydrograph No. 2, SCS Runoff, DA-FR-124B DEV	6
TR-55 Tc Worksheet	7
Hydrograph No. 3, SCS Runoff, DA-FR-124B FOR	8
TR-55 Tc Worksheet	9
2 - Year	
Summary Report	10
Hydrograph Reports	11
Hydrograph No. 1, SCS Runoff, DA-FR-124B PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-124B DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-124B FOR	13
10 - Year	
	4.4
Summary Report	
Hydrograph No. 4, CCC Dunoff, DA ED 404B DDE	
Hydrograph No. 1, SCS Runoff, DA-FR-124B PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-124B DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-124B FOR	17
IDE Penort	10



Hydrograph Return Period Recap Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

		Inflow hyd(s)				Hydrograph					
No.			1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		0.289	0.570			2.907				DA-FR-124B PRE
2	SCS Runoff		0.156	0.351			2.235				DA-FR-124B DEV
	SCS Runoff		0.250	0.489			2.503				DA-FR-124B FOR

Proj. file: DA-FR-124B_Hydraflow.gpw

Thursday, 08 / 31 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description		
1	SCS Runoff	0.289	1	730	2,163				DA-FR-124B PRE		
2	SCS Runoff	0.156	1	737	1,710				DA-FR-124B DEV		
3	SCS Runoff	0.250	1	734	2,163				DA-FR-124B FOR		
DA	 -FR-124B_H <u>y</u>	/draflow.g	gpw		Return P	eriod: 1 Ye	ear	Thursday, 08 / 31 / 2017			

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

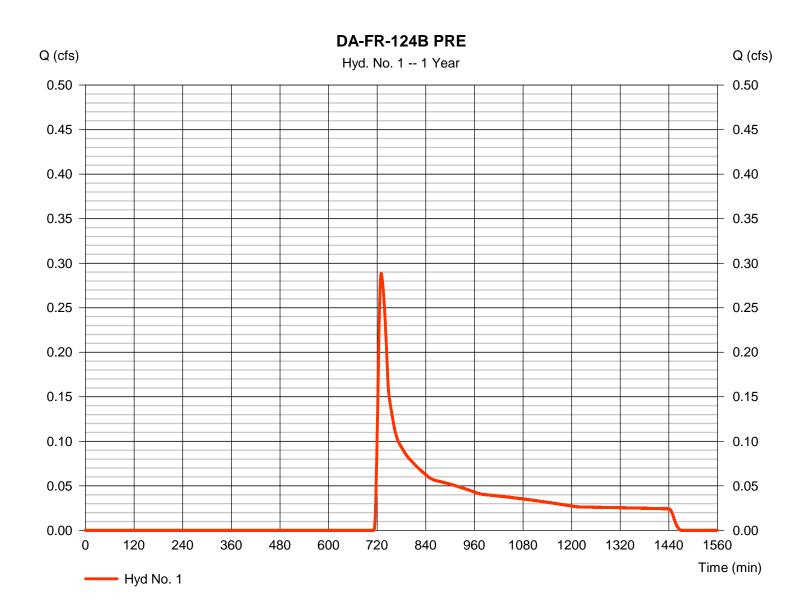
Thursday, 08 / 31 / 2017

Hyd. No. 1

DA-FR-124B PRE

Hydrograph type = SCS Runoff Peak discharge = 0.289 cfsStorm frequency Time to peak = 730 min = 1 yrsTime interval = 1 minHyd. volume = 2,163 cuftCurve number Drainage area = 2.120 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 18.60 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.003 \times 82) + (0.109 \times 58) + (0.003 \times 100) + (2.003 \times 55)] / 2.120$



TR55 Tc Worksheet

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1 DA-FR-124B PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 19.8 = 3.70 = 10.10	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	_	2.86
Traver Time (IIIII)	- 2.00	т	0.00	т	0.00	_	2.00
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 79.71 = 12.89 = Unpaved =5.79	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.23	+	0.00	+	0.00	=	0.23
Channel Flow							
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.01 = 3.47 = 3.10 = 0.030 =0.17		0.00 0.00 0.00 0.015		0.00 0.00 0.00 0.015		
					0.00		
Flow length (ft)	({0})161.5		0.0		0.0		
Travel Time (min)	= 15.50	+	0.00	+	0.00	=	15.50
Total Travel Time, Tc							18.60 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

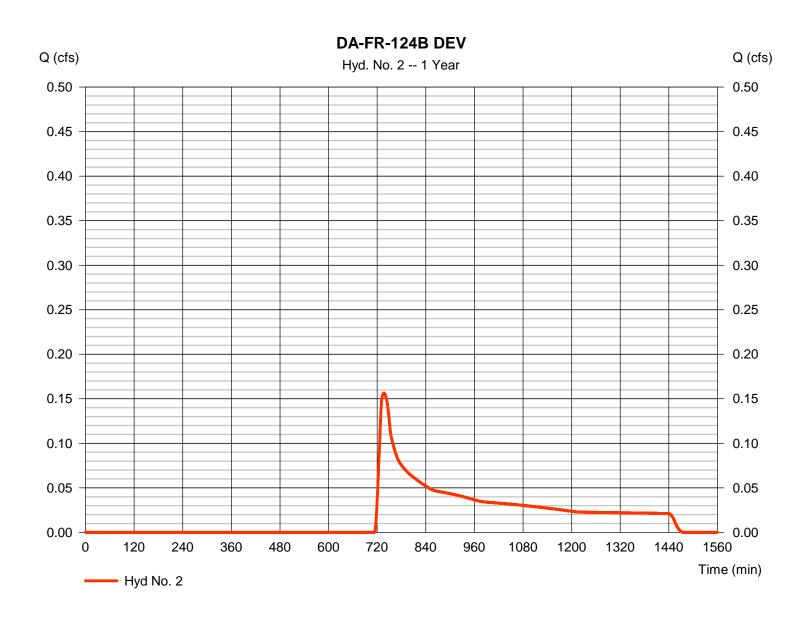
Thursday, 08 / 31 / 2017

Hyd. No. 2

DA-FR-124B DEV

Hydrograph type = SCS Runoff Peak discharge = 0.156 cfsStorm frequency Time to peak = 737 min = 1 yrsTime interval = 1 minHyd. volume = 1,710 cuftCurve number Drainage area = 2.120 ac= 53*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 22.40 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(1.137 \times 48) + (0.003 \times 82) + (0.948 \times 58) + (0.003 \times 100) + (0.028 \times 55)] / 2.120$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2

DA-FR-124B DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 19.8 = 3.70 = 10.10		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 2.86	+	0.00	+	0.00	=	2.86
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 79.71 = 12.89 = Unpaved =5.79	t	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.23	+	0.00	+	0.00	=	0.23
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.01 = 3.47 = 3.10 = 0.030 =0.14		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value	= 3.47 = 3.10 = 0.030		0.00 0.00 0.015		0.00 0.00 0.015		
X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 3.47 = 3.10 = 0.030 =0.14	+	0.00 0.00 0.015 0.00	+	0.00 0.00 0.015 0.00	=	19.32

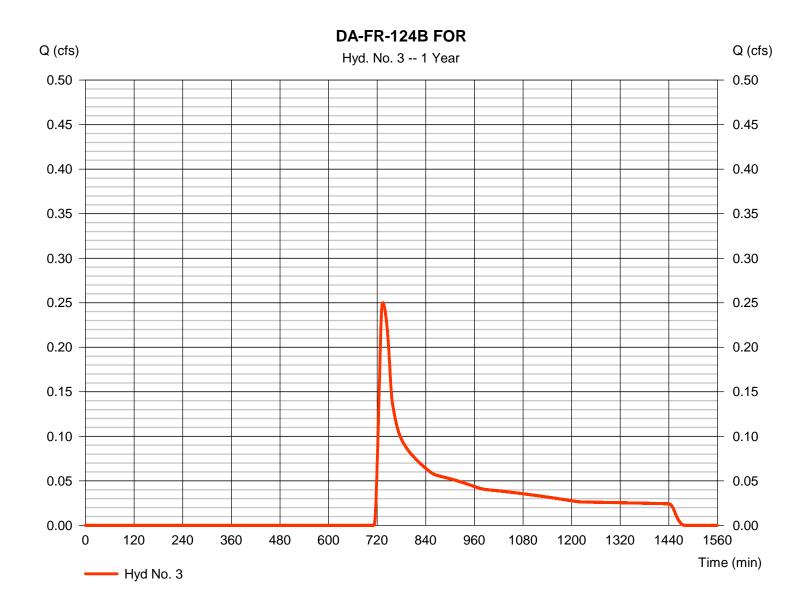
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Thursday, 08 / 31 / 2017

Hyd. No. 3

DA-FR-124B FOR

Hydrograph type = SCS Runoff Peak discharge = 0.250 cfsStorm frequency Time to peak = 734 min = 1 yrsTime interval = 1 minHyd. volume = 2,163 cuftDrainage area Curve number = 2.120 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 23.80 \, \text{min}$ = TR55 Total precip. = 3.30 inDistribution = Type II Storm duration Shape factor = 24 hrs = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3

DA-FR-124B FOR

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 19.8 = 3.70 = 10.10		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 2.86	+	0.00	+	0.00	=	2.86
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 79.71 = 12.89 = Unpaved =5.79	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.23	+	0.00	+	0.00	=	0.23
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.01 = 3.47 = 3.10 = 0.040 =0.13		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})161.5		0.0		0.0		
Travel Time (min)	= 20.67	+	0.00	+	0.00	=	20.67
Total Travel Time, Tc							23.80 min

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.570	1	729	3,198				DA-FR-124B PRE
2	SCS Runoff	0.351	1	732	2,621				DA-FR-124B DEV
3	SCS Runoff	0.489	1	732	3,198				DA-FR-124B FOR
DA	-FR-124B_H	ydraflow.g	gpw		Return F	eriod: 2 Ye	ear	Thursday, 0	08 / 31 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

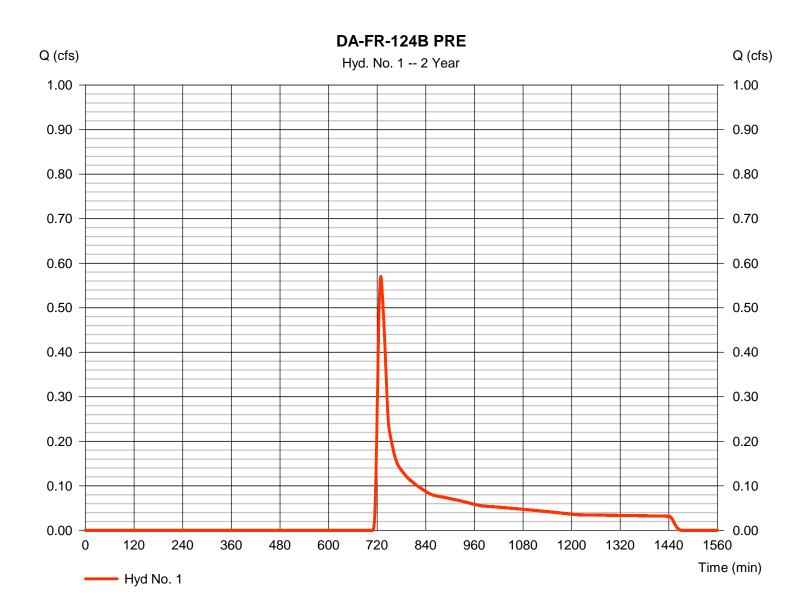
Thursday, 08 / 31 / 2017

Hyd. No. 1

DA-FR-124B PRE

Hydrograph type = SCS Runoff Peak discharge = 0.570 cfsStorm frequency = 2 yrsTime to peak = 729 min Time interval = 1 min Hyd. volume = 3,198 cuftDrainage area Curve number = 2.120 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 18.60 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484= 24 hrs

^{*} Composite (Area/CN) = $[(0.003 \times 82) + (0.109 \times 58) + (0.003 \times 100) + (2.003 \times 55)] / 2.120$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

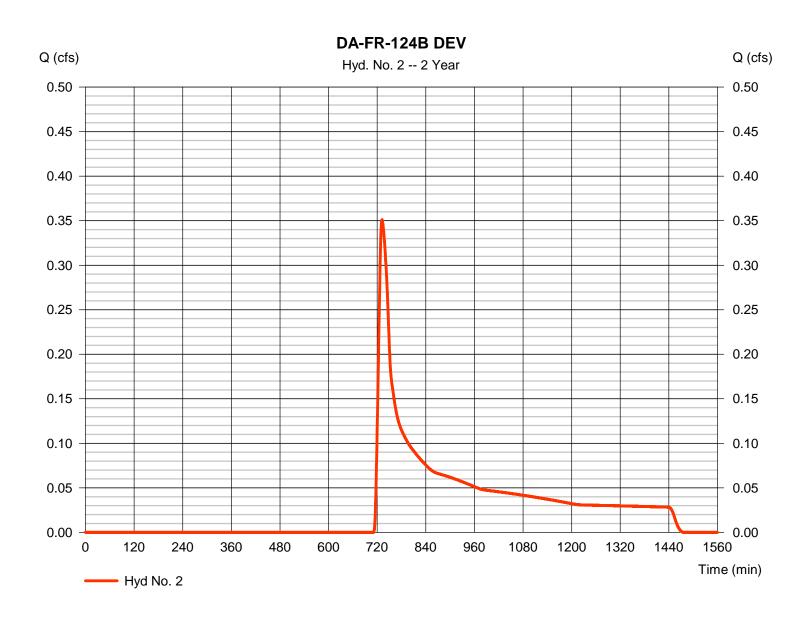
Thursday, 08 / 31 / 2017

Hyd. No. 2

DA-FR-124B DEV

Hydrograph type = SCS Runoff Peak discharge = 0.351 cfsStorm frequency = 2 yrsTime to peak = 732 min Time interval = 1 min Hyd. volume = 2.621 cuftCurve number Drainage area = 2.120 ac= 53*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 22.40 \, \text{min}$ Total precip. Distribution = Type II = 3.70 inShape factor Storm duration = 484= 24 hrs

^{*} Composite $(Area/CN) = [(1.137 \times 48) + (0.003 \times 82) + (0.948 \times 58) + (0.003 \times 100) + (0.028 \times 55)] / 2.120$



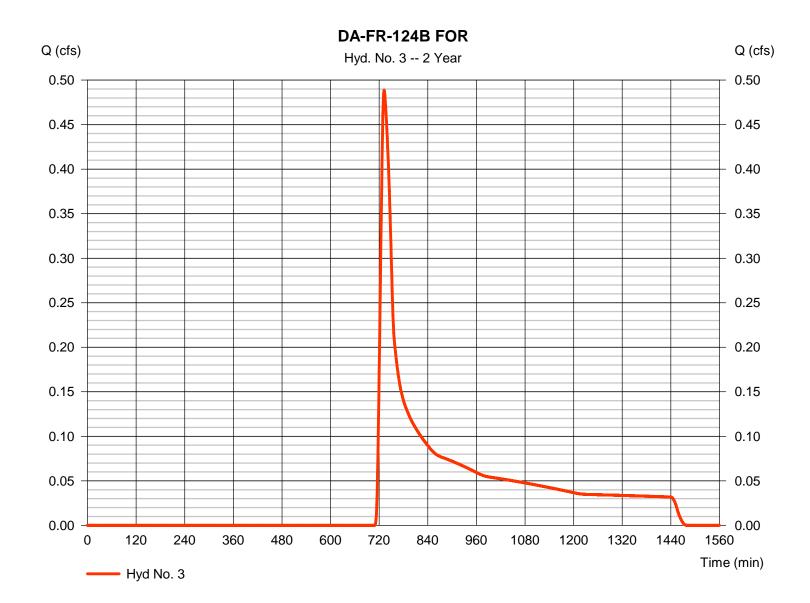
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Thursday, 08 / 31 / 2017

Hyd. No. 3

DA-FR-124B FOR

Hydrograph type = SCS Runoff Peak discharge = 0.489 cfsStorm frequency = 2 yrsTime to peak = 732 min Time interval = 1 min Hyd. volume = 3,198 cuftDrainage area Curve number = 2.120 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 23.80 \, \text{min}$ = TR55 Total precip. = 3.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	2.907	1	726	10,378				DA-FR-124B PRE
2	SCS Runoff	2.235	1	729	9,190				DA-FR-124B DEV
3	SCS Runoff	2.503	1	730	10,378				DA-FR-124B FOR
D4	_FR_124B LI	vdraflow a	NDW.		Ratura P	eriod: 10 Y	/ear	Thursday (08 / 31 / 2017
DA	-FR-124B_H	yuranow.g	βρw		Retuin	enou. To t	c ai	i iluisuay, C	00 / 31 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

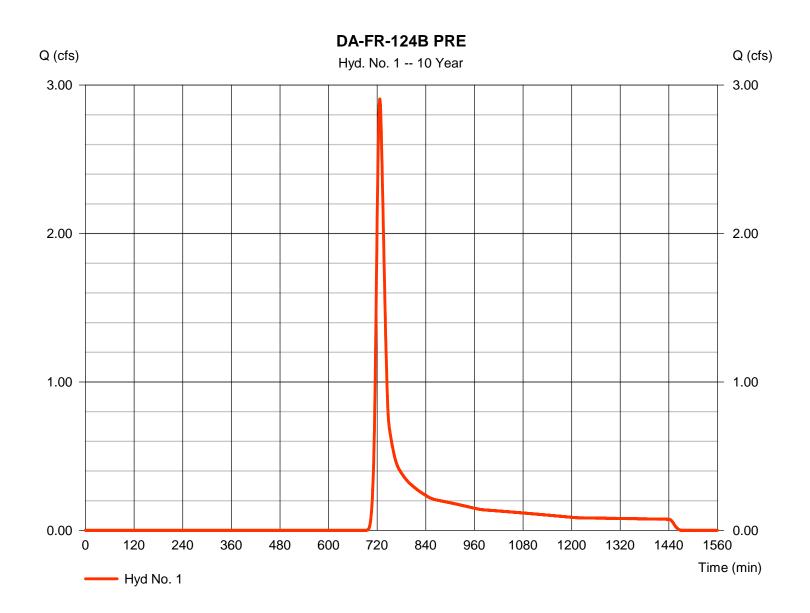
Thursday, 08 / 31 / 2017

Hyd. No. 1

DA-FR-124B PRE

Hydrograph type = SCS Runoff Peak discharge = 2.907 cfsStorm frequency = 10 yrsTime to peak = 726 min Time interval = 1 minHyd. volume = 10.378 cuftCurve number Drainage area = 2.120 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) = 18.60 min Tc method = TR55 Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs = 484Shape factor

^{*} Composite (Area/CN) = $[(0.003 \times 82) + (0.109 \times 58) + (0.003 \times 100) + (2.003 \times 55)] / 2.120$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

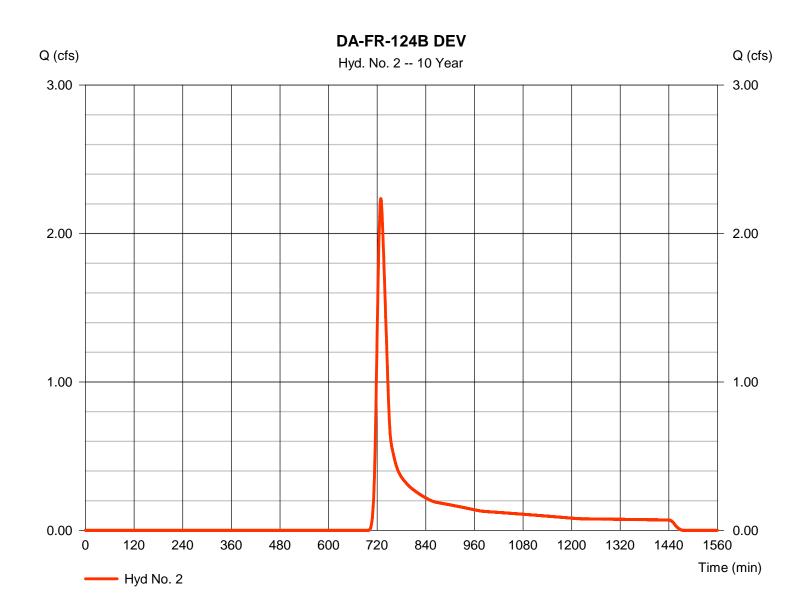
Thursday, 08 / 31 / 2017

Hyd. No. 2

DA-FR-124B DEV

Hydrograph type = SCS Runoff Peak discharge = 2.235 cfsStorm frequency = 10 yrsTime to peak = 729 min Time interval = 1 minHyd. volume = 9.190 cuftDrainage area = 2.120 acCurve number = 53*Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) = 22.40 min Tc method = TR55 Total precip. Distribution = Type II = 5.70 inStorm duration = 24 hrs = 484Shape factor

^{*} Composite $(Area/CN) = [(1.137 \times 48) + (0.003 \times 82) + (0.948 \times 58) + (0.003 \times 100) + (0.028 \times 55)] / 2.120$



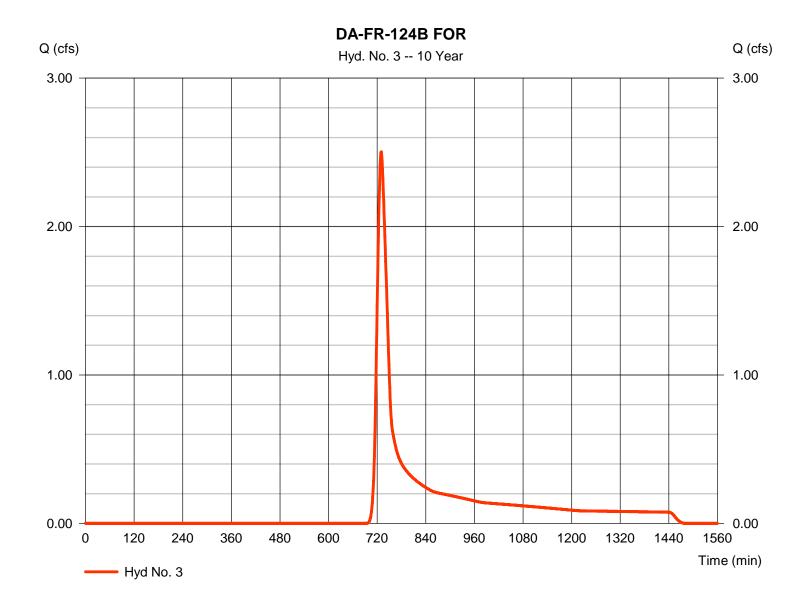
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Thursday, 08 / 31 / 2017

Hyd. No. 3

DA-FR-124B FOR

Hydrograph type = SCS Runoff Peak discharge = 2.503 cfsStorm frequency = 10 yrsTime to peak = 730 min Time interval = 1 minHyd. volume = 10,378 cuftDrainage area Curve number = 2.120 ac= 55Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 23.80 \, \text{min}$ = TR55 Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Thursday, 08 / 31 / 2017

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)								
(Yrs)	В	D	E	(N/A)					
1	0.0000	0.0000	0.0000						
2	69.8703	13.1000	0.8658						
3	0.0000	0.0000	0.0000						
5	79.2597	14.6000	0.8369						
10	88.2351	15.5000	0.8279						
25	102.6072	16.5000	0.8217						
50	114.8193	17.2000	0.8199						
100	127.1596	17.8000	0.8186						

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return					Intens	sity Values	(in/hr)					
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

ling\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.p

Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

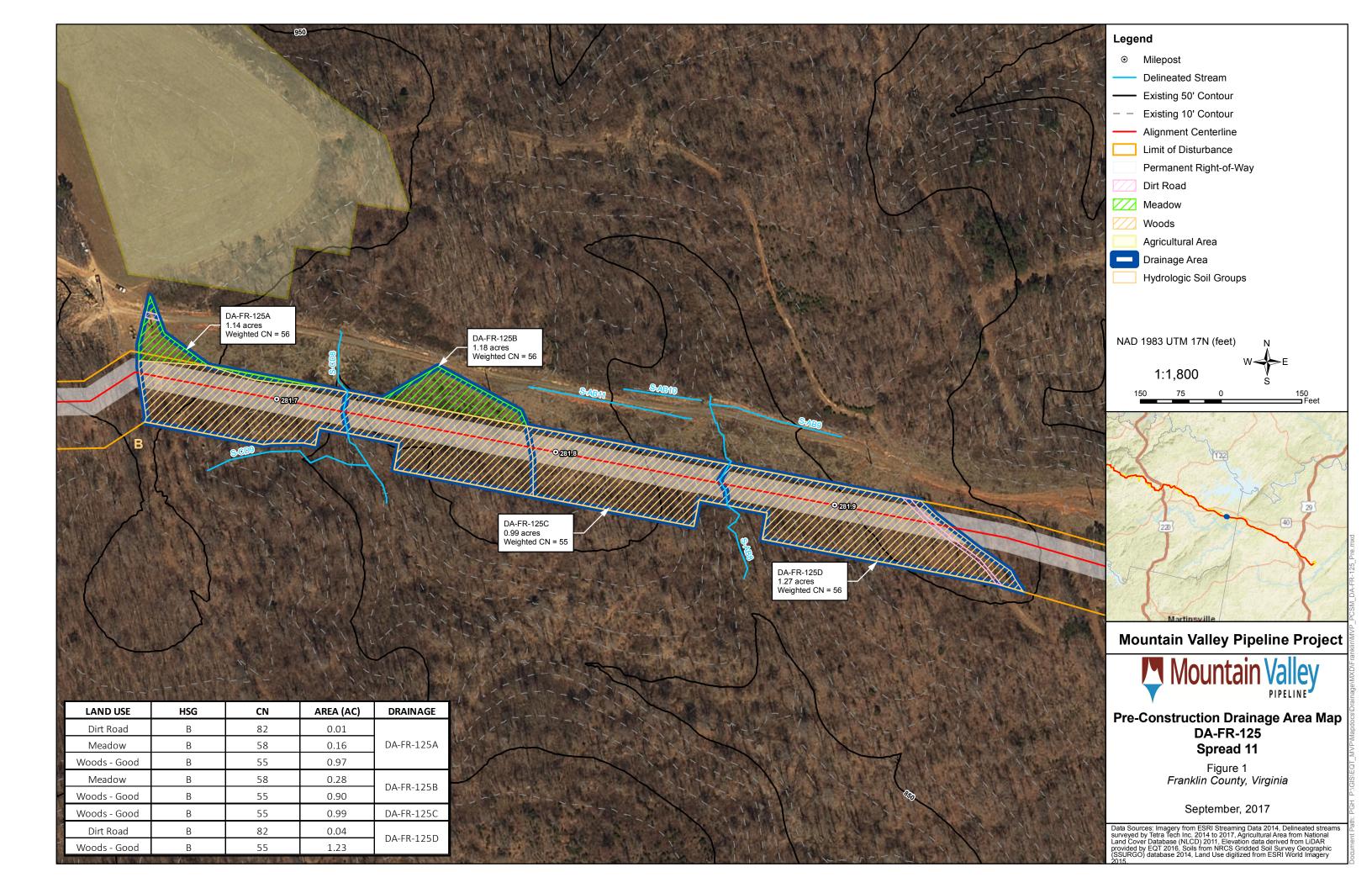
DA-FR-125

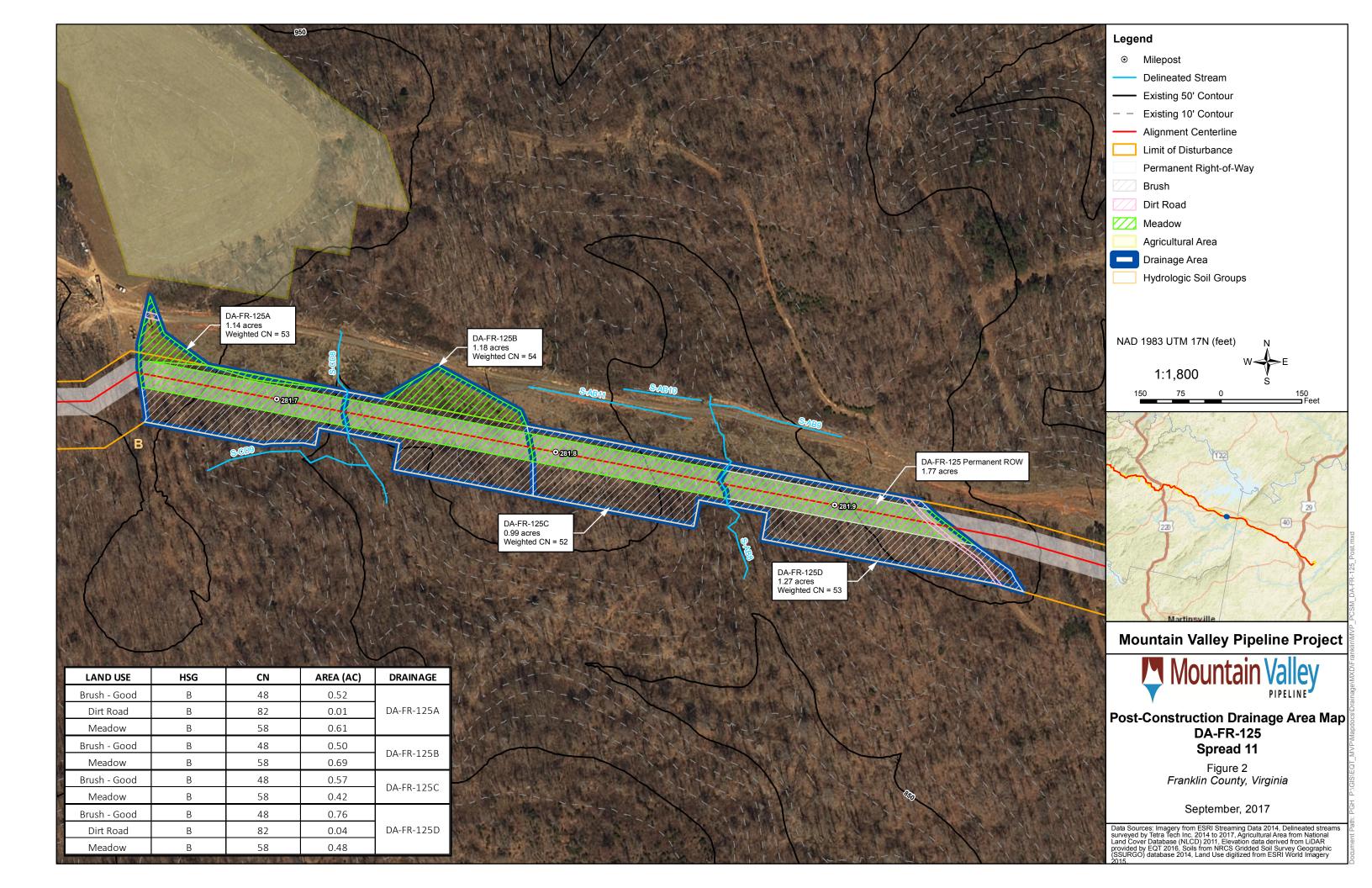
DA-FR-125 is located in a meadow and forested areas with hilly slopes and contains an existing gravel road. No new impervious area is proposed within DA-FR-125. The total phosphorus load reduction required for DA-FR-125 is -0.56 lb/yr. Multiple points of analysis were evaluated within DA-FR-125 to evaluate the effects on each receiving stream/channel following construction. Specifically, DA-FR-125 was sub-divided into four sub-drainage areas (sub-areas A through D).

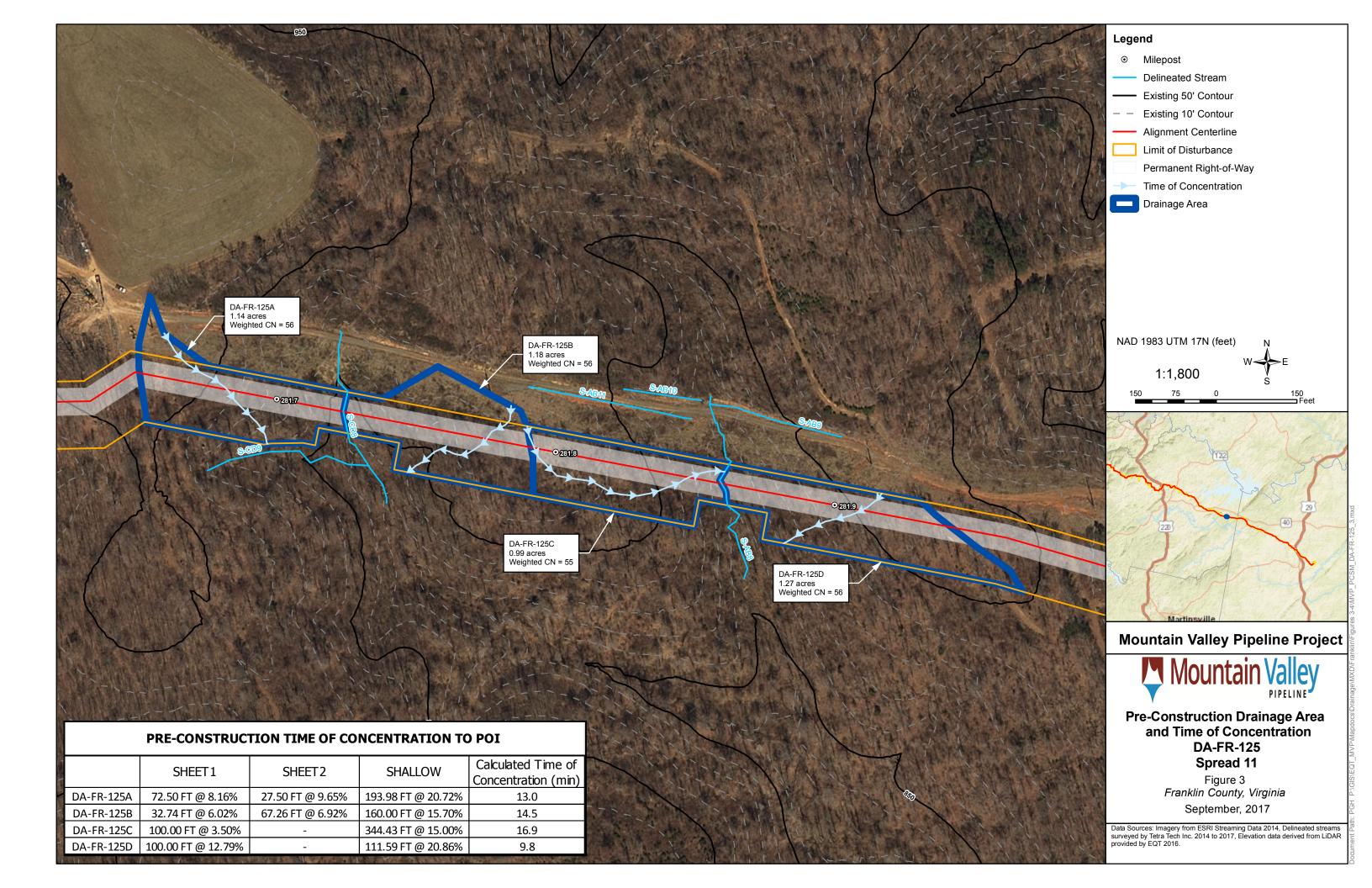
Stormwater quantity is met via the energy balance method for each of the four subareas DA-FR-125A, DA-FR-125B, DA-FR-125C and DA-FR-125D. In addition, the Hydraflow Hydrograph's 10-year 24-hour peak discharge results indicate a reduction in flows ranging from 0.27 to 0.68 cfs for all drainage areas (as seen in table below).

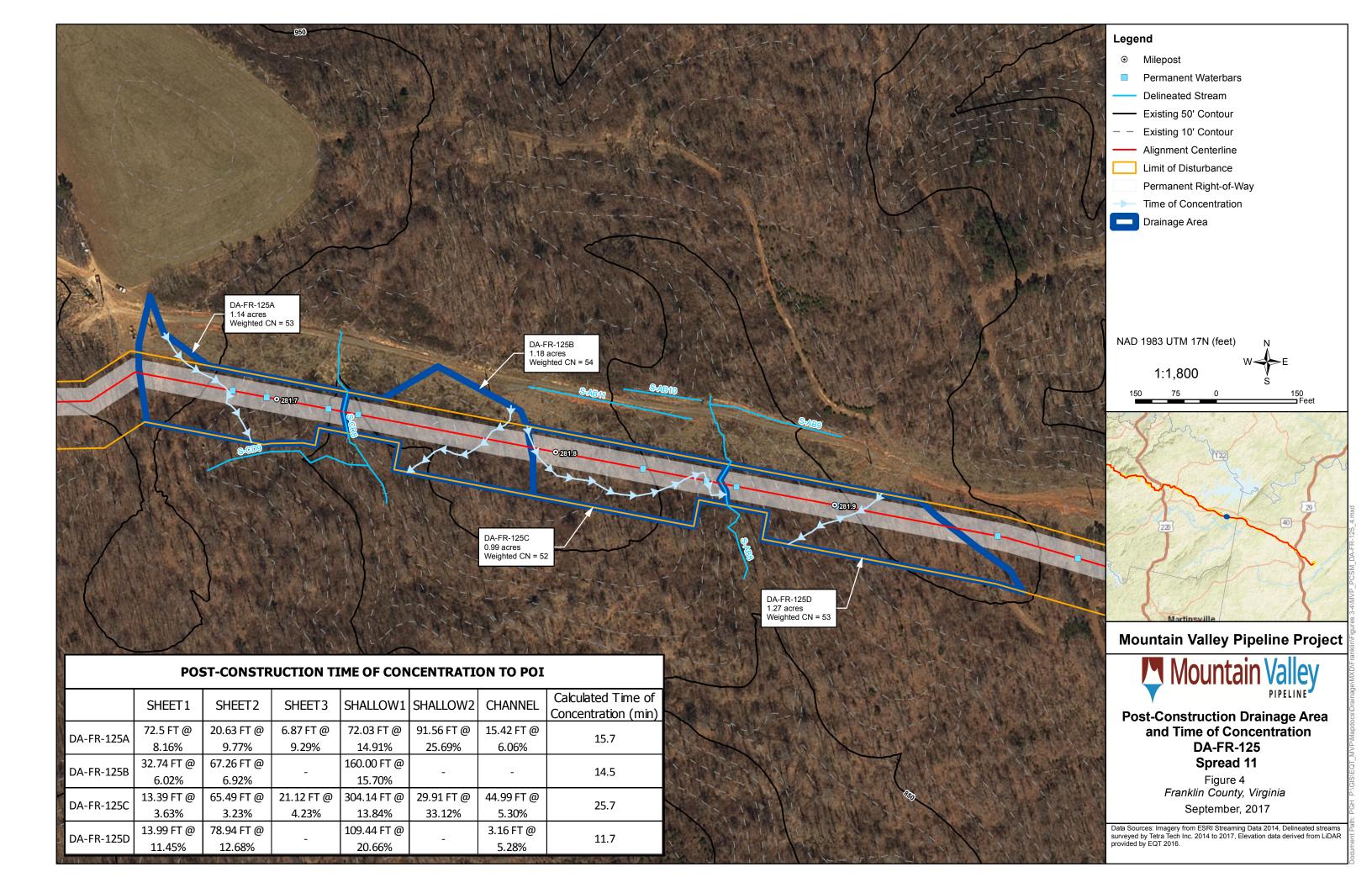
Sub Area	Pre Peak Flow,	Post Peak Flow, Q	Flow differential
	10-yr Q (cfs)	10-yr (cfs)	
DA-FR-125A	2.10	1.51	-0.59
DA-FR-125B	2.07	1.80	-0.27
DA-FR-125C	1.43	0.87	-0.56
DA-FR-125D	2.76	2.08	-0.68

Figures and calculations for each of the sub-areas for DA-FR-125 follow. See Appendix D of the Annual Standards and Specifications for further detail on stormwater methodology.









DEQ Virginia Runoff Reduction Method Re-Development Compliance Spreadsheet - Version 3.0

BMP Design Specifications List: 2013 Draft Stds & Specs

Site Summary - Linear Development Project***

Total Rainfall (in):	43
Total Disturbed Acreage:	1.77

Site Land Cover Summary

Pre-ReDevelopment Land Cover (acres)

in the end of the formation of the forma									
*************************************	A soils	B Soils	C Soils	D Soils	Totals	% of Total			
Forest/Open (acres)	0.00	1.75	0.00	0.00	1.75	99			
Managed Turf (acres)	0.00	0.00	0.00	0.00	0.00	0			
Impervious Cover (acres)	0.00	0.02	0.00	0.00	0.02	1			
					1.77	100			

Post-ReDevelopment Land Cover (acres)

~	A soils	B Soils	C Soils	D Soils	Totals	% of Total
Forest/Open (acres)	0.00	1.75	0.00	0.00	1.75	99
Managed Turf (acres)	0.00	0.00	0.00	0.00	0.00	0
Impervious Cover (acres)	0.00	0.02	0.00	0.00	0.02	1
* Forest/Open Space areas must be protected in	1.77	100				

Site Tv and Land Cover Nutrient Loads

	Final Post-Development (Post-ReDevelopment & New Impervious)	Post- ReDevelopment	Post- Development (New Impervious)	Adjusted Pre- ReDevelopment
Site Rv	0.04	0.04		0.04
Treatment Volume (ft ³)	260	260		260
TP Load (lb/yr)	0.16	0.16		0.16
	Deseller TD Lead (III /).	0.7257*	*Paduction balow now	dayalanment load limita

ReDevelopment TP Load per acre (lb/acre/yr)	Final Post-Development TP Load per acre (lb/acre/yr)	Post-ReDevelopment TP Load per acre (lb/acre/yr)
0.09	0.09	0.09

Baseline TP Load (lb/yr):

0.7257*

*Reduction below new development load limitation not required

Total TP Load Reduction Required (lb/yr)	-0.56	N/A***	N/A***
--	-------	--------	--------

***This is a linear development project

	Final Post-Development Load (Post-ReDevelopment & New Impervious)	Pre- ReDevelopment
TN Load (lb/yr)	1.17	1.17

Site Compliance Summary - ***Linear Development Project

Maximum % Reduction Required Below	20%	* Note: % Reduction will reduce post-development TP load to less than or equal to baseline load of 0.73 lb/yr (0.41 lb/ac/yr)
Pre-ReDevelopment Load	20%	[Post-Dev Reduction Requirement = Post-Dev TP load - baseline load of 0.73 lb/yr], baseline load = site area x 0.41 lb/ac/yr

Total Runoff Volume Reduction (ft ³)

a.	
Total TP Load Reduction Achieved (lb/yr)	0.00
Total TN Load Reduction Achieved (lb/yr)	0.00
Remaining Post Development TP Load (lb/yr)	0.16
Remaining TP Load Reduction (lb/yr) Required	0.00

** TARGET TP REDUCTION EXCEEDED BY 0.56 LB/YEAR **

.....

^{*}Reduction below new development load limitation not required



ENERGY BALANCE METHOD

In		

	1-Yr Event				
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)			
Pre-Developed Condition	0.251	1258			
Developed Condition	0.101	939			
Pre-Developed (Forest) Condition	0.202	1145			

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.8

Calculations:	Check #1:	$Q_{developed} \le IF \ x \left[\left(Q_{pre-developed} \ x \ RV_{pre-developed} \right) \ / \ RV_{developed} \right]>$	0.101	≤ OK	0.269
	Check #2:	$Q_{developed} \le Q_{pre-developed}$ >	0.101	≤ OK	0.251
	Check #3:	$Q_{developed} \frac{shall\ not}{shall\ not}$ be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed}$ >	0.101	<u>shall not</u> be required to be ≤	0.246

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n					
Grass:						
Average Grass Cover	0.40					
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40					
Light Turf	0.20					
Dense Turf	0.17 – 0.80					
Dense Grass	0.17 – 0.30					
Bermuda Grass	0.30 – 0.48					
Dense Shrubbery and Forest Litter	0.40					
Natural:						
Short Grass Prairie	0.10 – 0.20					
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40					
Sparse Vegetation	0.05 – 0.13					
Oak Grasslands, Open Grasslands	0.60					
Dense Cover of Trees and Bushes	0.80					
Rangeland:						
Typical	0.13					
No Debris Cover	0.09 – 0.34					
20% Debris Cover	0.05 – 0.25					
Woods:						
Light Underbrush	0.40					
Dense Underbrush	0.80					
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40					

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-125A PRE	
TR-55 Tc Worksheet	
Hydrograph No. 2, SCS Runoff, DA-FR-125A DEV	
TR-55 Tc Worksheet	
Hydrograph No. 3, SCS Runoff, DA-FR-125A FOR	
TR-55 Tc Worksheet	9
2 - Year	
Summary Report	10
Hydrograph Reports	11
Hydrograph No. 1, SCS Runoff, DA-FR-125A PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-125A DEV	12
Hydrograph No. 3, SCS Runoff, DA-FR-125A FOR	13
10 - Year	
	4.4
Summary ReportHydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-125A PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-125A FRE	
Hydrograph No. 3, SCS Runoff, DA-FR-125A FOR	
Trydrograph No. 3, 300 Narion, DA-1 N-123A 1 ON	17
IDE Papart	10

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5 <u>Legend</u> Hyd. Origin **Description** SCS Runoff DA-FR-125A PRE 2 SCS Runoff DA-FR-125A DEV SCS Runoff DA-FR-125A FOR Monday, 08 / 21 / 2017 Project: DA-FR-125A_Hydraflow.gpw

Hydrograph Return Period Recap

	Hydrograph	Inflow				Peak Out	flow (cfs)				Hydrograph
No. type hyd(s (origin)		hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		0.251	0.469			2.101				DA-FR-125A PRE
2	SCS Runoff		0.101	0.238			1.510				DA-FR-125A DEV
3	SCS Runoff		0.202	0.403			1.982				DA-FR-125A FOR

Proj. file: DA-FR-125A_Hydraflow.gpw

Monday, 08 / 21 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description			
1	SCS Runoff	0.251	1	725	1,258				DA-FR-125A PRE			
2	SCS Runoff	0.101	1	729	939				DA-FR-125A DEV			
2 3	SCS Runoff SCS Runoff	0.101	1 1	729	939 1,145				DA-FR-125A FOR			
DA	DA-FR-125A_Hydraflow.gpw					Return Period: 1 Year			Monday, 08 / 21 / 2017			

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

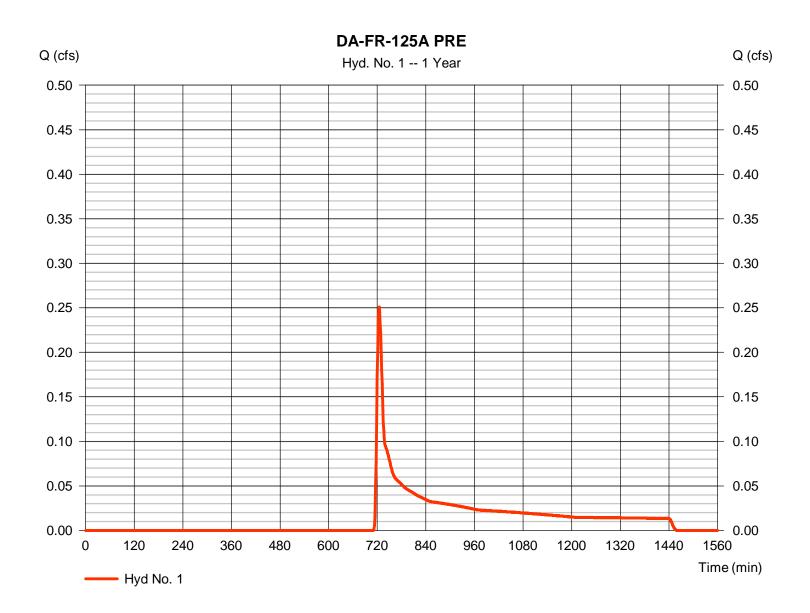
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-125A PRE

Hydrograph type = SCS Runoff Peak discharge = 0.251 cfsStorm frequency Time to peak = 725 min = 1 yrsTime interval = 1 min Hyd. volume = 1.258 cuftDrainage area Curve number = 1.130 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 13.00 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.005 \times 82) + (0.161 \times 58) + (0.968 \times 55)] / 1.130$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1

DA-FR-125A PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 72.5 = 3.70 = 8.16 = 8.80	+	0.400 27.5 3.70 9.65 3.79	+	0.011 0.0 0.00 0.00 0.00	=	12.59
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 193.98 = 20.72 = Unpaved =7.34	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.44	+	0.00	+	0.00	=	0.44
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							13.00 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

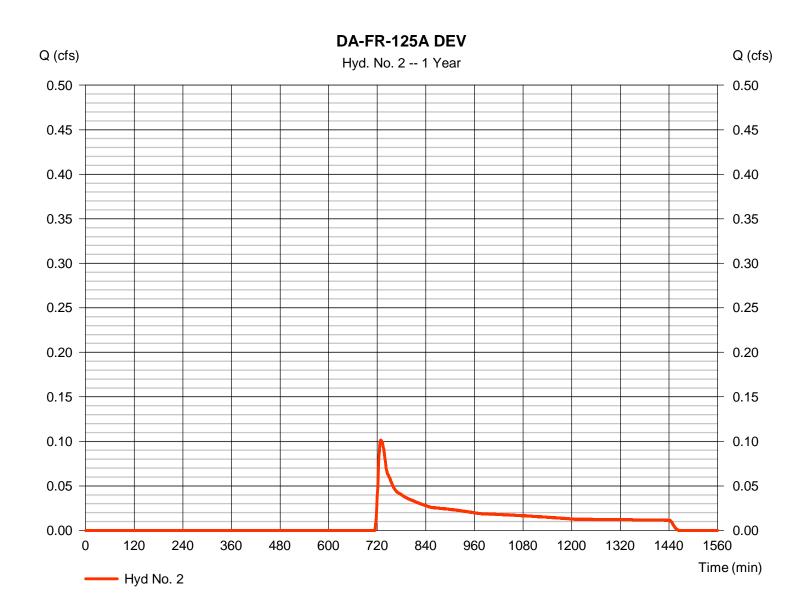
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-125A DEV

Hydrograph type = SCS Runoff Peak discharge = 0.101 cfsStorm frequency Time to peak = 729 min = 1 yrsTime interval = 1 min Hyd. volume = 939 cuft Drainage area Curve number = 1.140 ac= 53*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.70 min Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.525 \times 48) + (0.005 \times 82) + (0.606 \times 58)] / 1.140$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2

DA-FR-125A DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 72.5 = 3.70 = 8.16 = 8.80	+	0.800 20.6 3.70 9.77 5.22	+	0.400 6.9 3.70 9.29	=	15.28
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 72.03 = 14.91 = Unpave =6.23	d	91.56 25.69 Unpave 8.18	d	0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.19	+	0.19	+	0.00	=	0.38
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})15.4		0.0		0.0		
Travel Time (min)	= 0.05	+	0.00	+	0.00	=	0.05
Total Travel Time, Tc							15.70 min

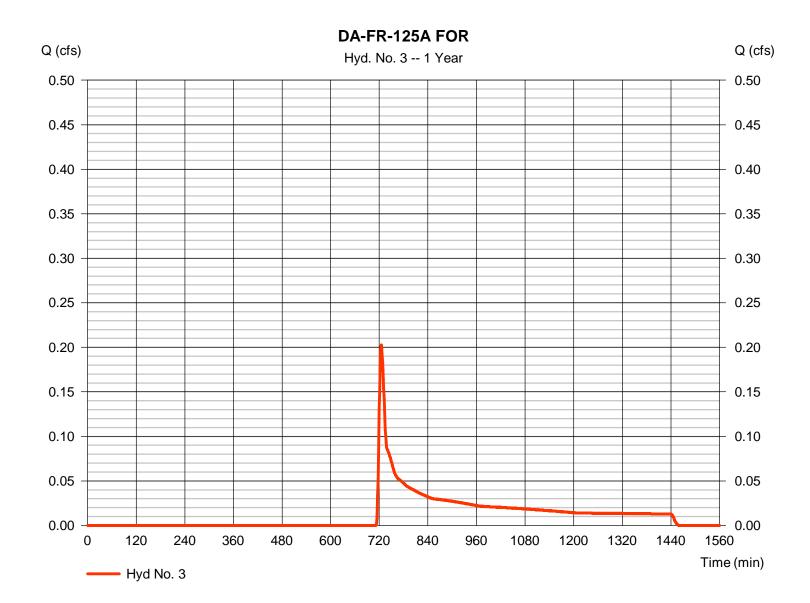
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-125A FOR

Hydrograph type = SCS Runoff Peak discharge = 0.202 cfsStorm frequency Time to peak = 725 min = 1 yrsTime interval = 1 min Hyd. volume = 1,145 cuftDrainage area Curve number = 1.140 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 13.00 \, \text{min}$ Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3

DA-FR-125A FOR

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 72.5 = 3.70 = 8.16 = 8.80	+	0.400 27.5 3.70 9.65 3.79	+	0.011 0.0 0.00 0.00 0.00	=	12.59
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 193.98 = 20.72 = Unpaved =7.34	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.44	+	0.00	+	0.00	=	0.44
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							13.00 min

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

lyd. Io.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description		
1	SCS Runoff	0.469	1	724	1,832				DA-FR-125A PRE		
2	SCS Runoff	0.238	1	727	1,440				DA-FR-125A DEV		
1 2 3											
DA-FR-125A_Hydraflow.gpw					Return P	eriod: 2 Ye	ear	Monday, 08 / 21 / 2017			

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

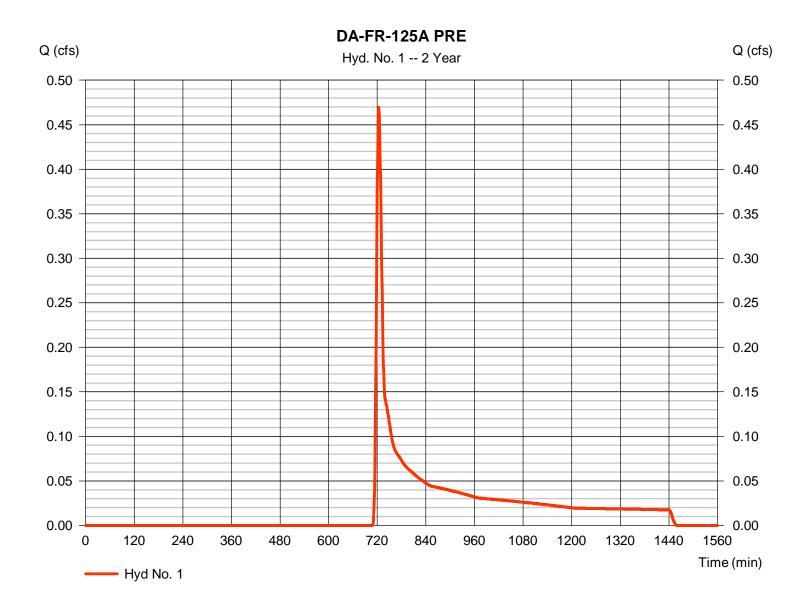
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-125A PRE

Hydrograph type = SCS Runoff Peak discharge = 0.469 cfsStorm frequency Time to peak = 724 min = 2 yrsTime interval = 1 minHyd. volume = 1,832 cuftDrainage area Curve number = 1.130 ac= 56* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 13.00 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.005 \times 82) + (0.161 \times 58) + (0.968 \times 55)] / 1.130$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

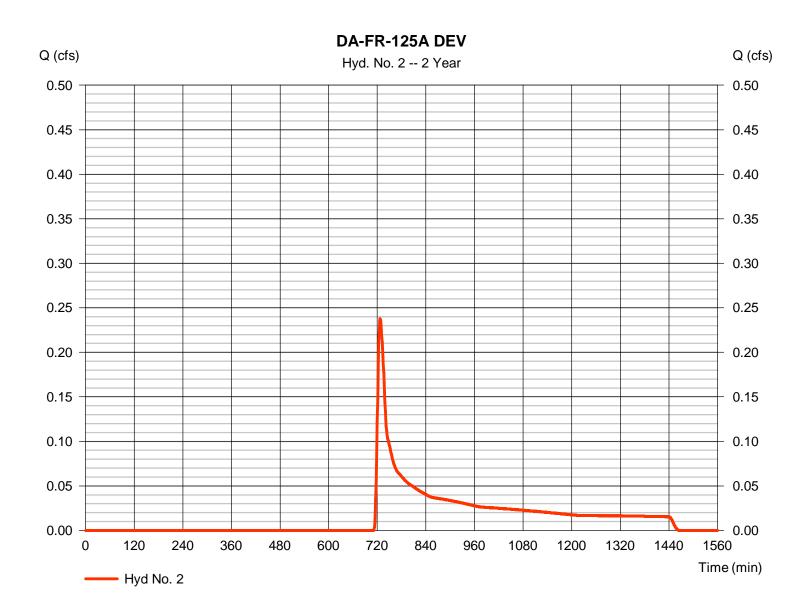
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-125A DEV

Hydrograph type = SCS Runoff Peak discharge = 0.238 cfsStorm frequency Time to peak = 727 min = 2 yrsTime interval = 1 min Hyd. volume = 1.440 cuftDrainage area Curve number = 1.140 ac= 53*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.70 min Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.525 \times 48) + (0.005 \times 82) + (0.606 \times 58)] / 1.140$



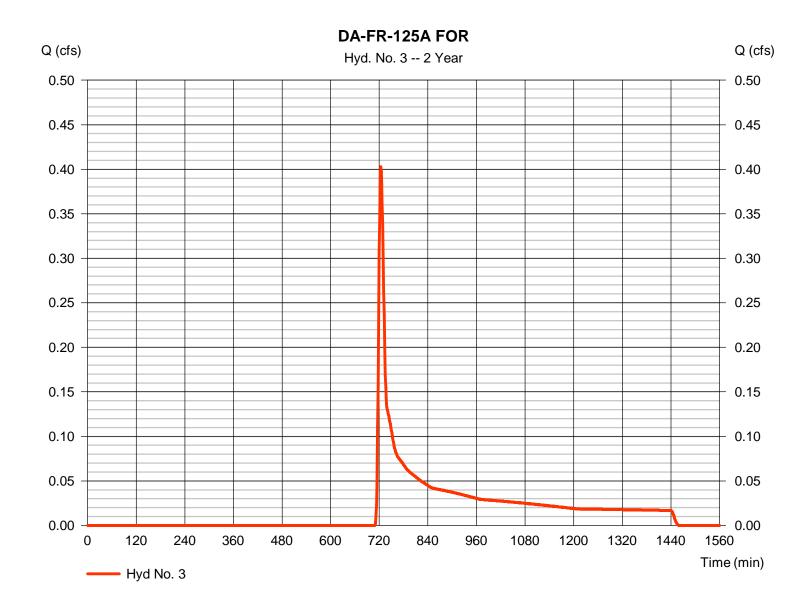
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-125A FOR

Hydrograph type = SCS Runoff Peak discharge = 0.403 cfsStorm frequency Time to peak = 724 min = 2 yrsTime interval = 1 min Hyd. volume = 1,693 cuftDrainage area Curve number = 1.140 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 13.00 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	2.101	1	722	5,742				DA-FR-125A PRE
2	SCS Runoff	1.510	1	724	5,049				DA-FR-125A DEV
DA	-FR-125A_H	ydraflow.g	gpw		Return F	Period: 10 \	/ear	Monday, 08	3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

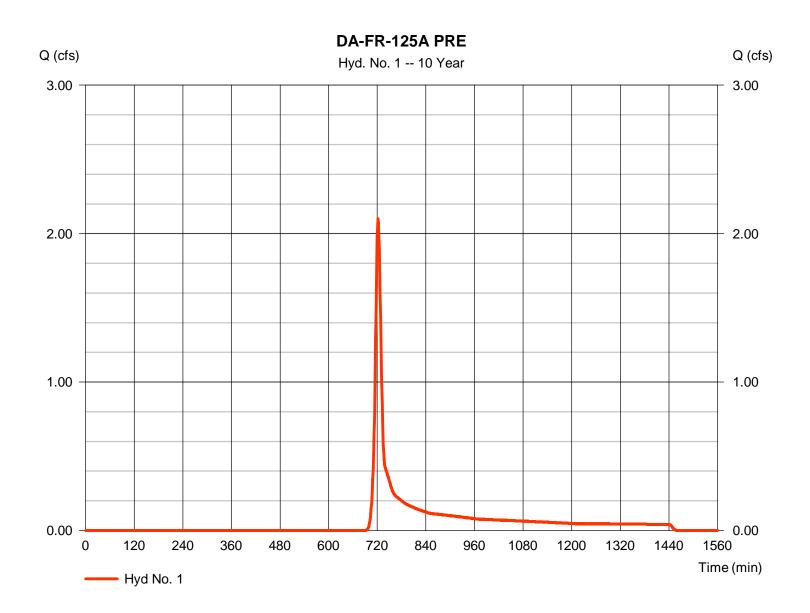
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-125A PRE

= SCS Runoff Hydrograph type Peak discharge = 2.101 cfsStorm frequency Time to peak = 722 min = 10 yrsTime interval = 1 min Hyd. volume = 5,742 cuftCurve number Drainage area = 1.130 ac= 56*Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 13.00 min Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.005 \times 82) + (0.161 \times 58) + (0.968 \times 55)] / 1.130$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

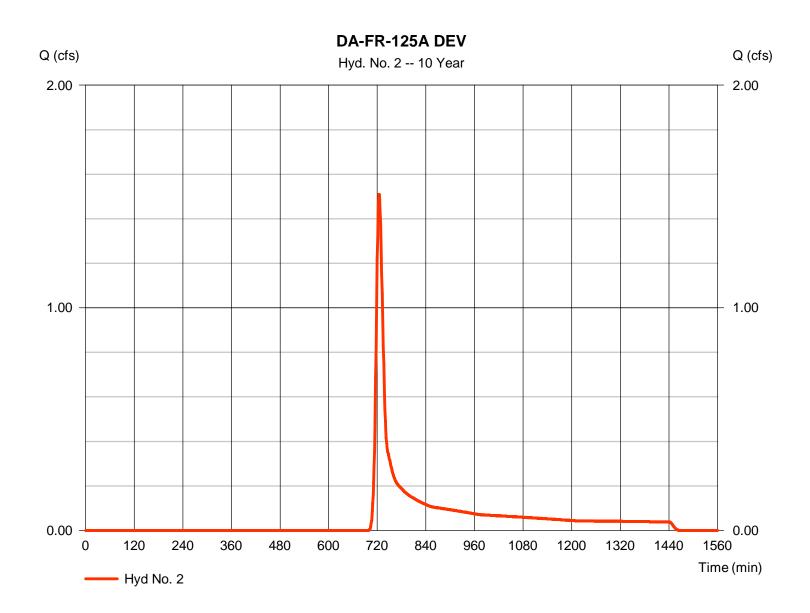
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-125A DEV

= SCS Runoff Hydrograph type Peak discharge = 1.510 cfsStorm frequency Time to peak = 724 min = 10 yrsTime interval = 1 min Hyd. volume = 5,049 cuftDrainage area Curve number = 1.140 ac= 53*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.70 min Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.525 \times 48) + (0.005 \times 82) + (0.606 \times 58)] / 1.140$



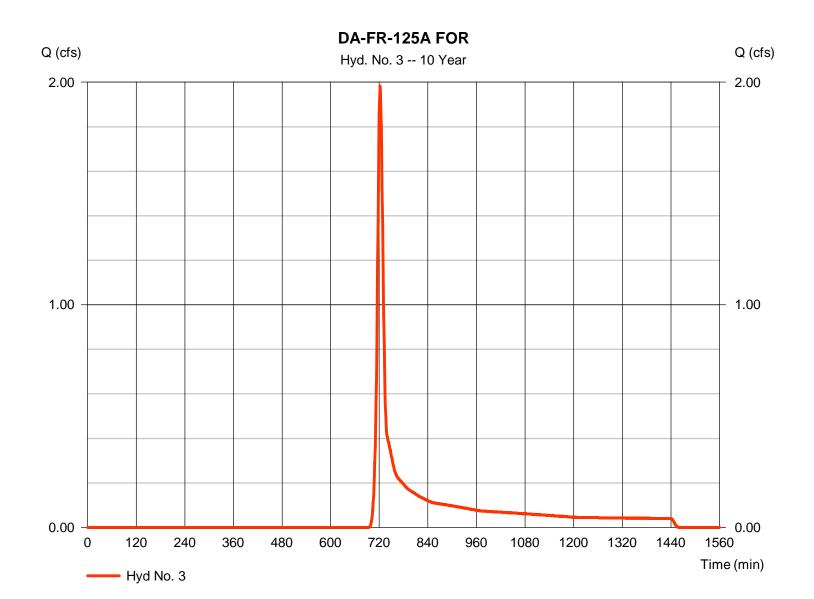
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-125A FOR

Hydrograph type = SCS Runoff Peak discharge = 1.982 cfsStorm frequency Time to peak = 722 min = 10 yrsTime interval = 1 min Hyd. volume = 5,493 cuftDrainage area Curve number = 1.140 ac= 55 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 13.00 min Total precip. Distribution = 5.70 in= Type II Storm duration = 484 = 24 hrs Shape factor



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Return Period	Intensity-Du	Intensity-Duration-Frequency Equation Coefficients (FHA)									
(Yrs)	В	D	E	(N/A)							
1	0.0000	0.0000	0.0000								
2	69.8703	13.1000	0.8658								
3	0.0000	0.0000	0.0000								
5	79.2597	14.6000	0.8369								
10	88.2351	15.5000	0.8279								
25	102.6072	16.5000	0.8217								
50	114.8193	17.2000	0.8199								
100	127.1596	17.8000	0.8186								

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return Period (Yrs)		Intensity Values (in/hr)													
	5 min	10	15	20	25	30	35	40	45	50	55	60			
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70			
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15			
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46			
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91			
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25			
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60			

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc|

		Rainfall Precipitation Table (in)												
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr						
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00						
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00						
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00						
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00						
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00						
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00						
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00						
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00						



ENERGY BALANCE METHOD

		ts

	1-Yr Event					
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)				
Pre-Developed Condition	0.244	1335				
Developed Condition	0.150	1079				
Pre-Developed (Forest) Condition	0.195	1204				

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.8

<u>Calculations:</u>	Check #1:	$Q_{\text{developed}} \leq \text{IF X [(}Q_{\text{pre-developed}} \times \text{RV}_{\text{pre-developed}}) \text{ / RV}_{\text{developed}}] \qquad>$	0.150	≤ OK	0.242
	Check #2:	$Q_{developed} \le Q_{pre-developed}$ >>	0.150	≤ OK	0.244
	Check #3:	$Q_{developed}$ $Shall not$ be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed}$ >	0.150	shall not be required to be ≤	0.218

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	_
Summary Report	3
Hydrograph ReportsHydrograph No. 1, SCS Runoff, DA-FR-125B PRE	
TR-55 Tc Worksheet	
Hydrograph No. 2, SCS Runoff, DA-FR-125B DEV	
TR-55 Tc Worksheet	
Hydrograph No. 3, SCS Runoff, DA-FR-125B FOR	
TR-55 Tc Worksheet	
2 - Year	
Summary Report	10
Hydrograph Reports	11
Hydrograph No. 1, SCS Runoff, DA-FR-125B PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-125B DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-125B FOR	13
10 - Year	
	4.4
Summary Report	
Hydrograph ReportsHydrograph No. 1, SCS Runoff, DA-FR-125B PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-125B DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-125B DEV	
11yarograph 140. 3, 300 Kanon, DA-1 K-123D1 OK	17
IDE Donort	10

Project: DA-FR-125B_Hydraflow.gpw

Monday, 08 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5 <u>Legend</u> Hyd. Origin **Description** SCS Runoff DA-FR-125B PRE 2 SCS Runoff DA-FR-125B DEV SCS Runoff DA-FR-125B FOR

Hydrograph Return Period Recap

		Inflow				Hydrograph					
No.		hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		0.244	0.456			2.070				DA-FR-125B PRE
2	SCS Runoff		0.150	0.322			1.797				DA-FR-125B DEV
	SCS Runoff		0.195	0.387			1.933				DA-FR-125B FOR

Proj. file: DA-FR-125B_Hydraflow.gpw

Monday, 08 / 21 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

łyd. ło.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description			
1	SCS Runoff	0.244	1	726	1,335				DA-FR-125B PRE			
2	SCS Runoff	0.150	1	727	1,079				DA-FR-125B DEV			
1 2 3												
DA-FR-125B_Hydraflow.gpw					Return F	Period: 1 Ye	ear	Monday, 08	Monday, 08 / 21 / 2017			

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

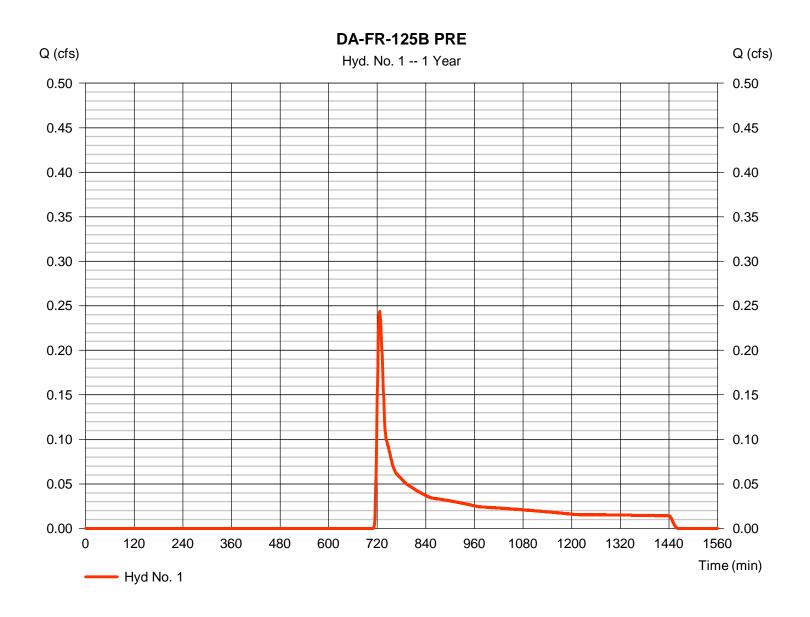
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-125B PRE

Hydrograph type = SCS Runoff Peak discharge = 0.244 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 min Hyd. volume = 1,335 cuftDrainage area Curve number = 1.180 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 14.50 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.283 x 58) + (0.899 x 55)] / 1.180



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1

DA-FR-125B PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>			
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 32.7 = 3.70 = 6.02 = 5.26 +		0.400 67.3 3.70 6.92 8.85 +		0.011 0.0 0.00 0.00 0.00 =		14.11			
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 160.00 = 15.70 = Unpaved =6.39	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00					
Travel Time (min)	= 0.42	+	0.00	+	0.00	=	0.42			
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015					
Flow length (ft)	({0})0.0		0.0		0.0					
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00			
Total Travel Time, Tc										

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

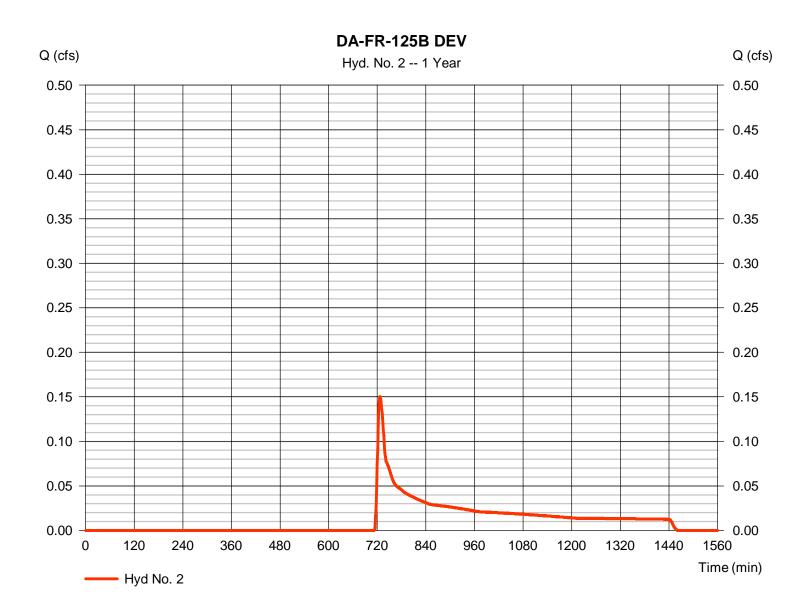
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-125B DEV

Hydrograph type = SCS Runoff Peak discharge = 0.150 cfsStorm frequency Time to peak = 727 min = 1 yrsTime interval = 1 min Hyd. volume = 1,079 cuftDrainage area Curve number = 1.180 ac= 54*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 14.50 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.495 x 48) + (0.687 x 58)] / 1.180



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2

DA-FR-125B DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 32.7 = 3.70 = 6.02 = 5.26	+	0.400 67.3 3.70 6.92 8.85	+	0.011 0.0 0.00 0.00 0.00	=	14.11
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 160.00 = 15.70 = Unpaved =6.39	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.42	+	0.00	+	0.00	=	0.42
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							14.50 min

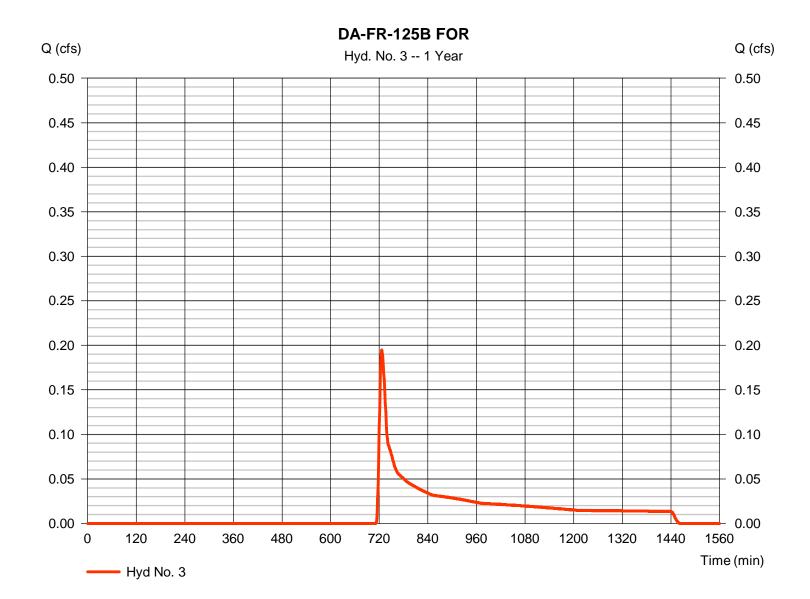
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-125B FOR

Hydrograph type = SCS Runoff Peak discharge = 0.195 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 min Hyd. volume = 1.204 cuftDrainage area Curve number = 1.180 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 14.50 \, \text{min}$ Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3

DA-FR-125B FOR

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 32.7 = 3.70 = 6.02		0.400 67.3 3.70 6.92		0.011 0.0 0.00 0.00		
Travel Time (min)	= 5.26	+	8.85	+	0.00	=	14.11
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 160.00 = 15.70 = Unpave =6.39	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.42	+	0.00	+	0.00	=	0.42
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							14.50 min

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

1 SCS Runolf 0.456 1 725 1,843 DA-FR-1258 PRE 2 SCS Runolf 0.322 1 725 1,623 DA-FR-1258 PCR DA-FR-12	Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
	1	SCS Runoff	0.456	1	725	1,943				DA-FR-125B PRE
3 SCS Rundf 0.387 1 725 1,780 DA-FR-125B FOR	2	SCS Runoff	0.322	1	725	1,623				DA-FR-125B DEV
	2	SCS Runoff	0.322	1	725	1,623				DA-FR-125B DEV
DA-FR-125B_Hydraflow.gpw Return Period: 2 Year Monday, 08 / 21 / 2017										

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

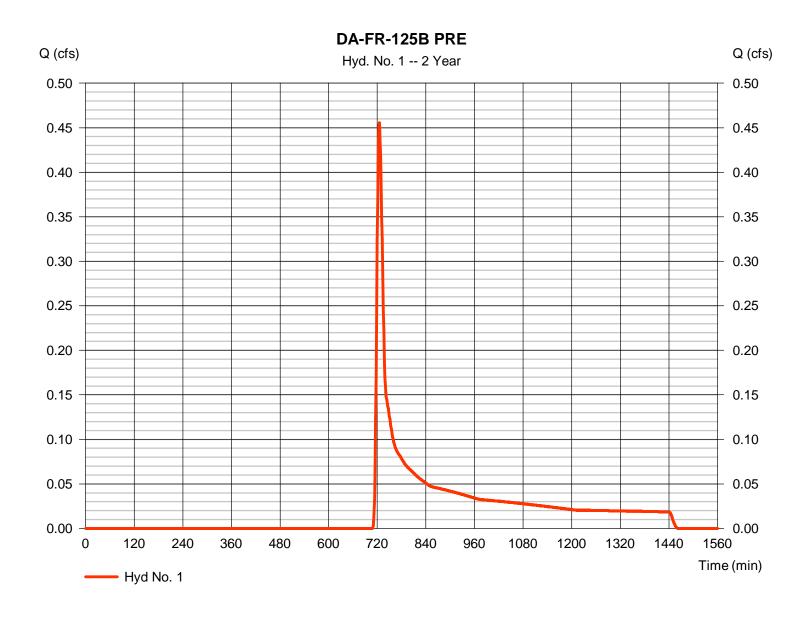
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-125B PRE

Hydrograph type = SCS Runoff Peak discharge = 0.456 cfsStorm frequency Time to peak = 725 min = 2 yrsTime interval = 1 min Hyd. volume = 1,943 cuftDrainage area Curve number = 1.180 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 14.50 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = [(0.283 x 58) + (0.899 x 55)] / 1.180



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

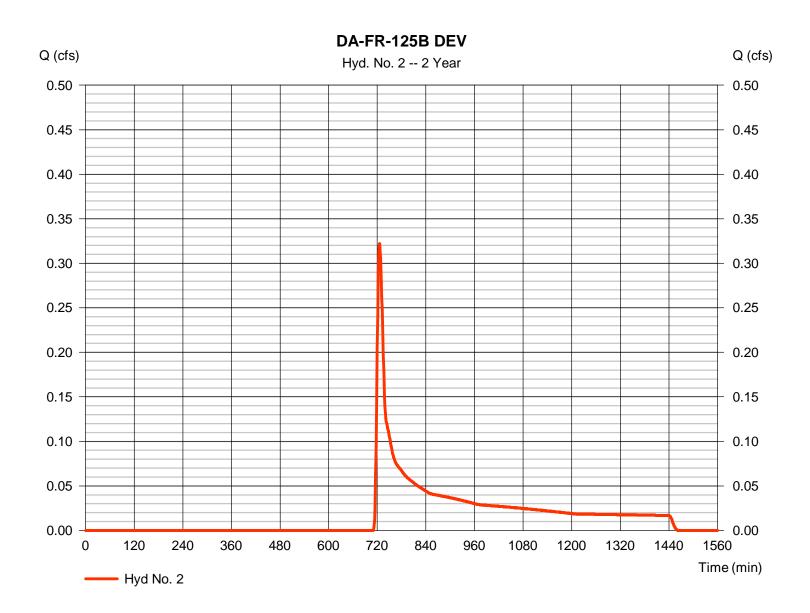
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-125B DEV

Hydrograph type = SCS Runoff Peak discharge = 0.322 cfsStorm frequency Time to peak = 725 min = 2 yrsTime interval = 1 min Hyd. volume = 1,623 cuftDrainage area Curve number = 1.180 ac= 54* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 14.50 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.495 x 48) + (0.687 x 58)] / 1.180



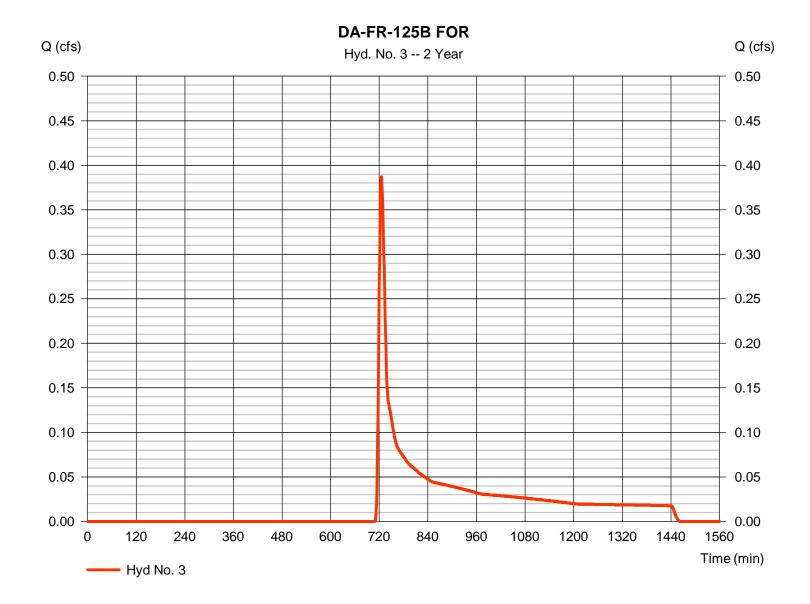
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-125B FOR

Hydrograph type = SCS Runoff Peak discharge = 0.387 cfsStorm frequency Time to peak = 725 min = 2 yrsTime interval = 1 min Hyd. volume = 1,780 cuftDrainage area Curve number = 1.180 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 14.50 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	2.070	1	723	6,091				DA-FR-125B PRE
2	SCS Runoff	1.797	1	723	5,466				DA-FR-125B DEV
DA	-FR-125B_H ₃	/draflow.g	jpw	l	Return F	Period: 10 Y	′ear	Monday, 08	3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

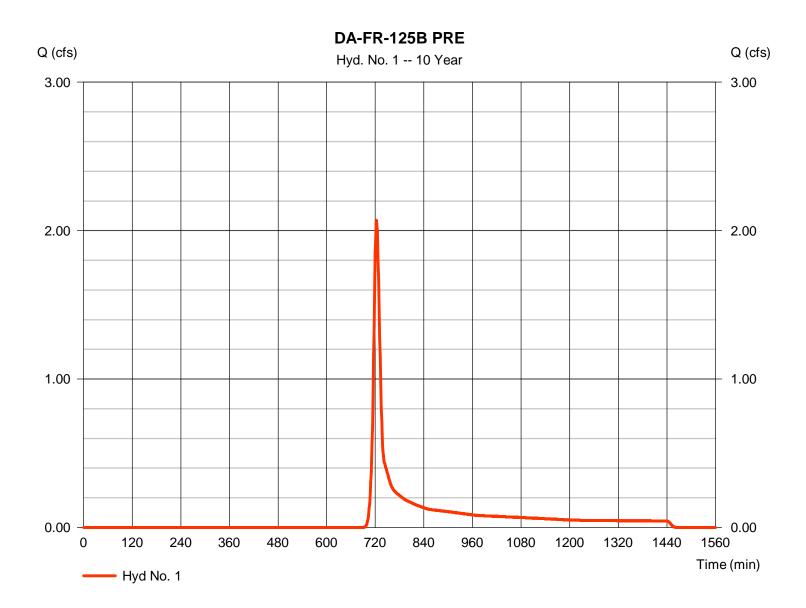
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-125B PRE

= SCS Runoff Hydrograph type Peak discharge = 2.070 cfsStorm frequency Time to peak = 723 min = 10 yrsTime interval = 1 min Hyd. volume = 6.091 cuftDrainage area Curve number = 1.180 ac= 56*Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 14.50 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.283 x 58) + (0.899 x 55)] / 1.180



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

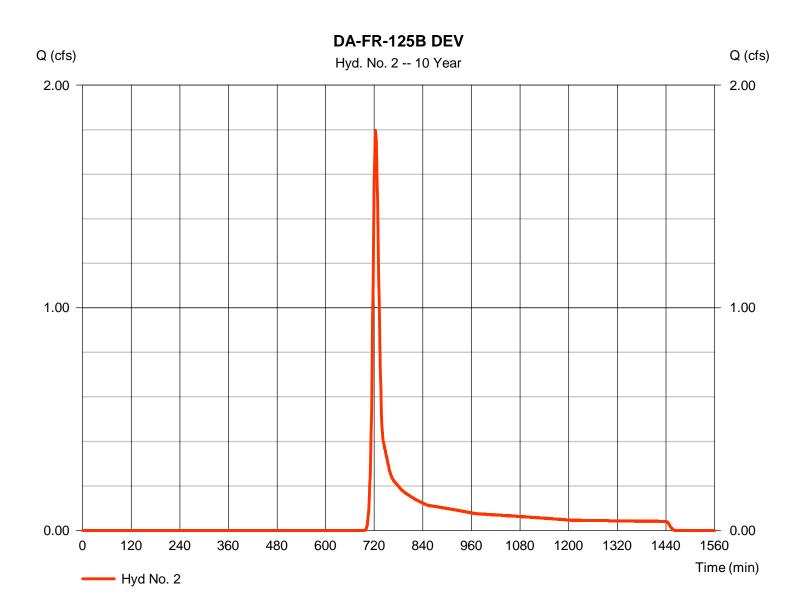
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-125B DEV

= SCS Runoff = 1.797 cfsHydrograph type Peak discharge Storm frequency Time to peak = 723 min = 10 yrsTime interval = 1 min Hyd. volume = 5,466 cuftDrainage area Curve number = 1.180 ac= 54*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 14.50 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.495 x 48) + (0.687 x 58)] / 1.180



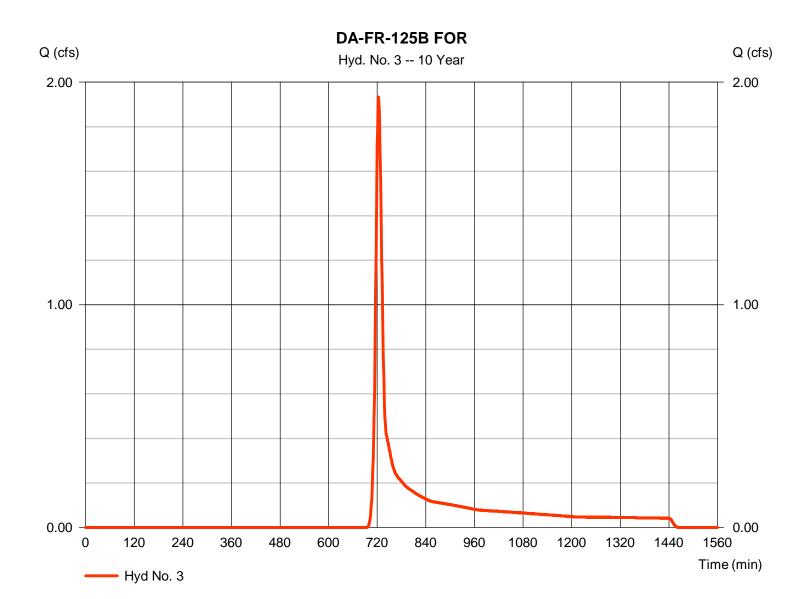
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-125B FOR

= SCS Runoff Hydrograph type Peak discharge = 1.933 cfsStorm frequency Time to peak = 723 min = 10 yrsTime interval = 1 min Hyd. volume = 5,776 cuftDrainage area Curve number = 1.180 ac= 55 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 14.50 \, \text{min}$ Total precip. Distribution = 5.70 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Return Period	Intensity-Du	ıration-Frequency Ed	quation Coefficients ((FHA)
(Yrs)	В	D	E	(N/A)
1	0.0000	0.0000	0.0000	
2	69.8703	13.1000	0.8658	
3	0.0000	0.0000	0.0000	
5	79.2597	14.6000	0.8369	
10	88.2351	15.5000	0.8279	
25	102.6072	16.5000	0.8217	
50	114.8193	17.2000	0.8199	
100	127.1596	17.8000	0.8186	

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return	Intensity Values (in/hr)											
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc|

		Rainfall Precipitation Table (in)										
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr				
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00				
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00				



ENERGY BALANCE METHOD

In		

	1-Yr Event				
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)			
Pre-Developed Condition	0.142	995			
Developed Condition	0.053	716			
Pre-Developed (Forest) Condition	0.142	999			

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.9

<u>Calculations:</u>	Check #1:	$Q_{\text{developed}} \leq \text{IF } x \left[\left(Q_{\text{gre-developed}} \times \text{RV}_{\text{gre-developed}} \right) / \text{RV}_{\text{developed}} \right]>$	0.053	≤ N/A - See Check #3	0.178
	Check #2:	$Q_{developed} \le Q_{pre-developed}$	0.053	≤ N/A - See Check #3	0.142
	Check #3:	$Q_{developed} \frac{shall\ not}{not}$ be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed}$ >	0.053	<u>shall not</u> be required to be ≤	0.198

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	·
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	<mark>0.40</mark>
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 – Manning's n Values for Open Channel Flow

nan	nel T	уре		Manning n	
			Min.	Normal	Max
1.	Exc	avated or Dredged Channels¹			
	a.	Earth, Straight, and Uniform:			
		Clean, recently completed	0.016	0.018	0.020
		Clean, after weathering	0.018	0.022	0.02
		Gravel, uniform section, clean	0.022	0.025	0.030
		With short grass, few weeds	0.022	0.027	0.03
	b.	Earth Winding and Sluggish:			
		No vegetation	0.023	0.025	0.030
		Grass, some weeds	0.025	0.030	0.03
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040
		Earth bottom and rubble sides	0.028	0.030	0.03
		Stony bottom and weedy banks	0.025	0.035	0.040
		Cobble bottom and clean sides	0.030	<mark>0.040</mark>	0.050
	c.	Dragline-Excavated or Dredged:		•	
		No vegetation	0.025	0.028	0.03
		Light brush on banks	0.035	0.050	0.06
	d.	Rock Cuts:			
		Smooth and uniform	0.025	0.035	0.040
		Jagged and irregular	0.035	0.040	0.050
	e.	Channels not Maintained, Weeds and Brush Uncut:			
		Dense weeds, high as flow depth	0.050	0.080	0.120
		Clean bottom, brush on sides	0.040	0.050	0.080
		Same as above, highest stage of flow	0.045	0.070	0.110
		Dense brush, high stage	0.080	0.100	0.140
2.	Mai	n Channels²			
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.03
	b.	Same as above, but more stones and weeds	0.030	0.035	0.04
	c.	Clean, winding, some pools and shoals	0.033	0.040	0.04
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.05
	f.	Same as (d) with more stones	0.045	0.050	0.060
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	4
Hydrograph No. 1, SCS Runoff, DA-FR-125C PRE	4
TR-55 Tc Worksheet	
Hydrograph No. 2, SCS Runoff, DA-FR-125C DEV	6
TR-55 Tc Worksheet	7
Hydrograph No. 3, SCS Runoff, DA-FR-125C FOR	
TR-55 Tc Worksheet	9
2 - Year	
Summary Report	10
Hydrograph Reports	11
Hydrograph No. 1, SCS Runoff, DA-FR-125C PRE	11
Hydrograph No. 2, SCS Runoff, DA-FR-125C DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-125C FOR	
40 Vaan	
10 - Year	
Summary Report	
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-125C PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-125C DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-125C FOR	17
IDE Papart	10

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5 <u>Legend</u> Hyd. Origin **Description** SCS Runoff DA-FR-125C PRE 2 SCS Runoff DA-FR-125C DEV SCS Runoff DA-FR-125C FOR Project: DA-FR-125C_Hydraflow.gpw Monday, 08 / 21 / 2017

Hydrograph Return Period Recap

		Inflow				Hydrograph					
	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		0.142	0.281			1.426				DA-FR-125C PRE
2	SCS Runoff		0.053	0.122			0.874				DA-FR-125C DEV
3	SCS Runoff		0.142	0.282			1.430				DA-FR-125C FOR

Proj. file: DA-FR-125C_Hydraflow.gpw

Monday, 08 / 21 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description	
1	SCS Runoff	0.142	1	729	995				DA-FR-125C PRE	
2	SCS Runoff	0.053	1	745	716				DA-FR-125C DEV	
DA-FR-125C_Hydraflow.gpw					Return F	eriod: 1 Ye	ear	Monday, 08 / 21 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

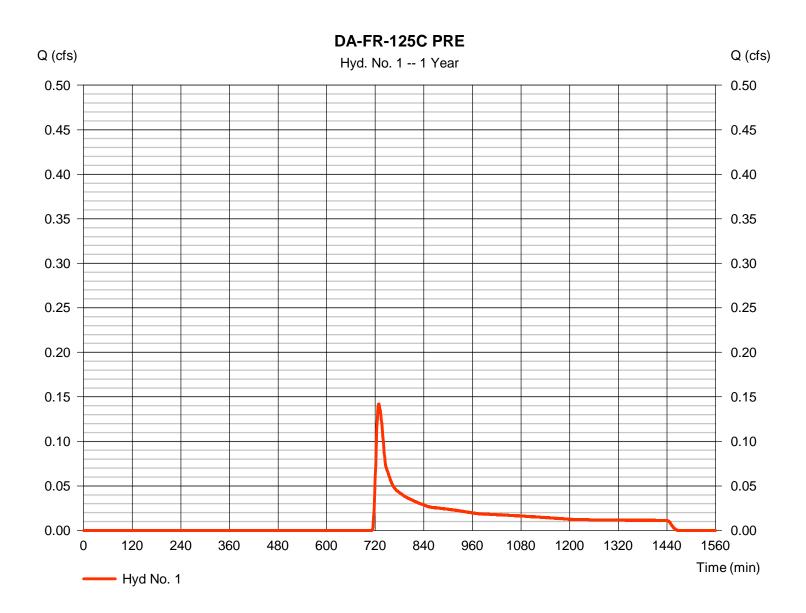
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-125C PRE

Hydrograph type = SCS Runoff Peak discharge = 0.142 cfsStorm frequency Time to peak = 729 min = 1 yrsTime interval = 1 minHyd. volume = 995 cuft Drainage area Curve number = 0.987 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 16.90 min Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.568 \times 48) + (0.419 \times 58)] / 0.987$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1

DA-FR-125C PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 3.50 = 15.97	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	=	15.97
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 344.43 = 15.00 = Unpave =6.25	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.92	+	0.00	+	0.00	=	0.92
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							16.90 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

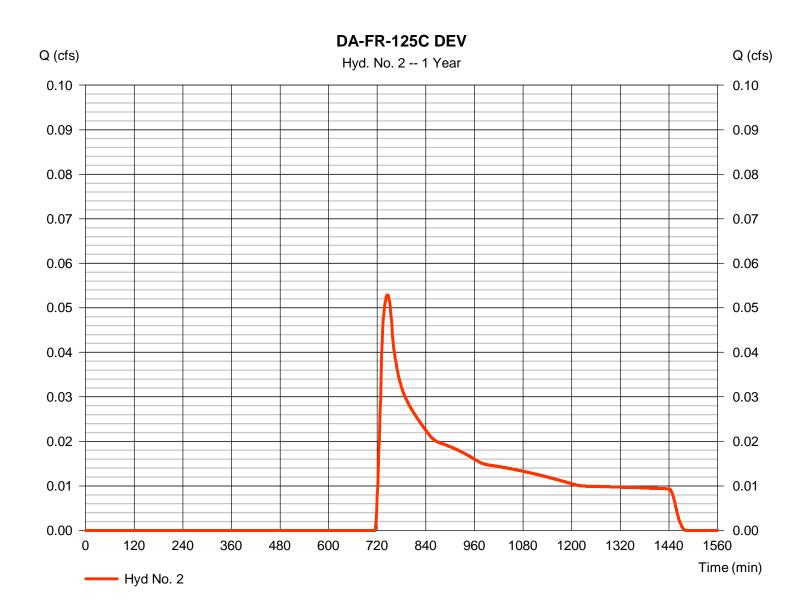
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-125C DEV

Hydrograph type = SCS Runoff Peak discharge = 0.053 cfsStorm frequency Time to peak = 745 min = 1 yrsTime interval = 1 min Hyd. volume = 716 cuft Drainage area Curve number = 0.990 ac= 52*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 25.70 min Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.568 \times 48) + (0.419 \times 58)] / 0.990$



 $\label{thm:condition} \mbox{Hydrographs Extension for AutoCAD} \mbox{\@none} \mbox{\$

Hyd. No. 2

DA-FR-125C DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.800 = 13.4 = 3.70 = 3.63		0.400 65.5 3.70 3.23		0.800 21.1 3.70 4.23			
Travel Time (min)	= 5.48	+	11.75	+	7.43	=	24.66	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 304.14 = 13.84 = Unpave =6.00	d	29.91 33.12 Unpave 9.29	d	0.00 0.00 Paved 0.00			
Travel Time (min)	= 0.84	+	0.05	+	0.00	=	0.90	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015			
Flow length (ft)	({0})45.0		0.0		0.0			
Travel Time (min)	= 0.15	+	0.00	+	0.00	=	0.15	
Total Travel Time, Tc								

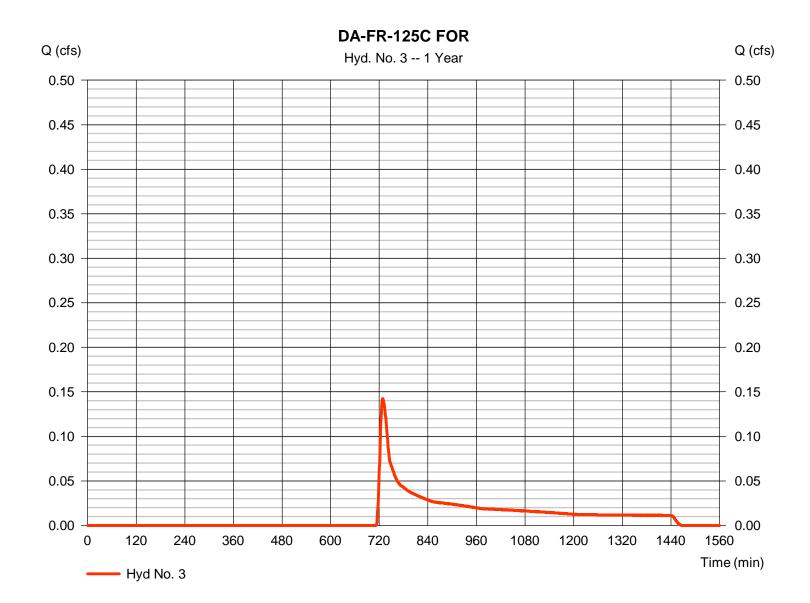
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-125C FOR

Hydrograph type = SCS Runoff Peak discharge = 0.142 cfsStorm frequency Time to peak = 729 min = 1 yrsTime interval = 1 min Hyd. volume = 999 cuft Drainage area Curve number = 0.990 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 16.90 min Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3

DA-FR-125C FOR

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 3.50 = 15.97	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	=	15.97
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 344.43 = 15.00 = Unpave =6.25	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.92	+	0.00	+	0.00	=	0.92
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							16.90 min

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.281	1	727	1,472				DA-FR-125C PRE
2	SCS Runoff	0.122	1	736	1,123				DA-FR-125C DEV
2									
DA:	-FR-125C_Hy	/draflow.g	jpw		Return F	Period: 2 Ye	ear	Monday, 08	3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

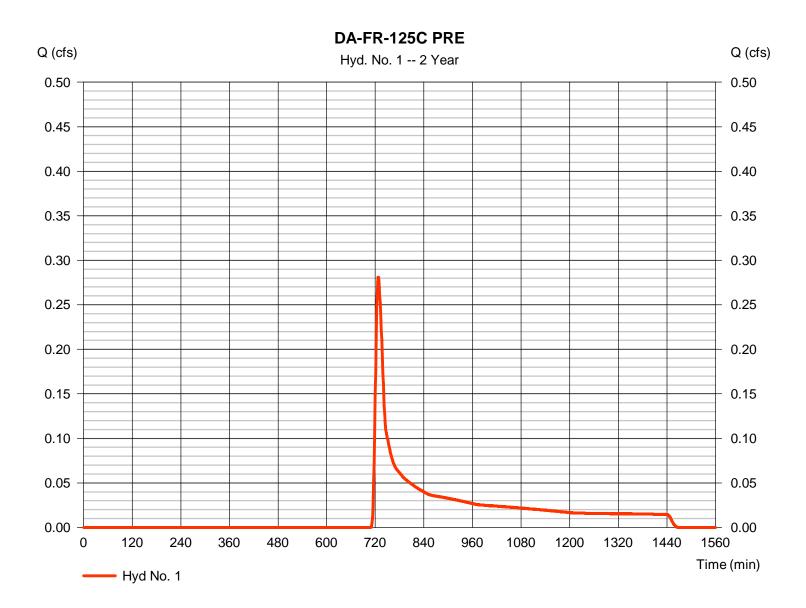
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-125C PRE

Hydrograph type = SCS Runoff Peak discharge = 0.281 cfsStorm frequency Time to peak = 727 min = 2 yrsTime interval = 1 minHyd. volume = 1,472 cuftDrainage area Curve number = 0.987 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 16.90 min Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = [(0.568 x 48) + (0.419 x 58)] / 0.987



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

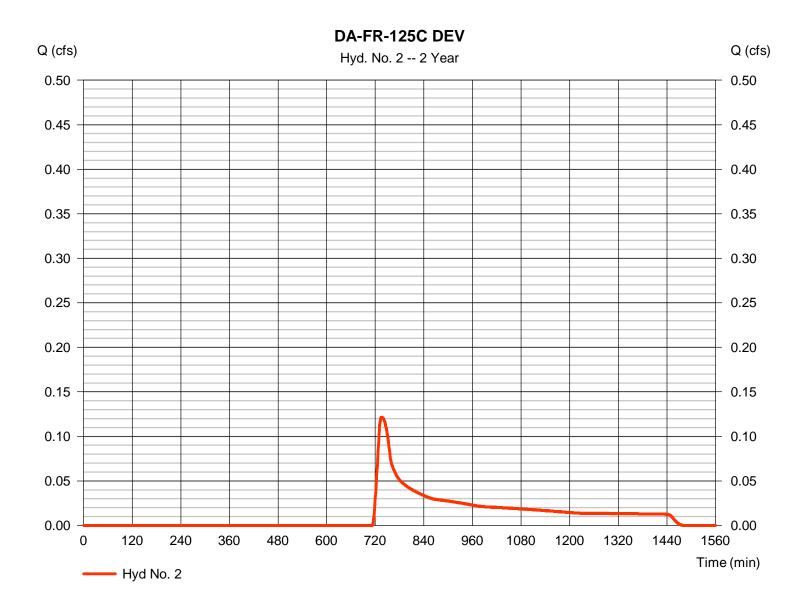
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-125C DEV

Hydrograph type = SCS Runoff Peak discharge = 0.122 cfsStorm frequency Time to peak = 736 min = 2 yrsTime interval = 1 min Hyd. volume = 1,123 cuftDrainage area Curve number = 0.990 ac= 52*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 25.70 min Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.568 \times 48) + (0.419 \times 58)] / 0.990$



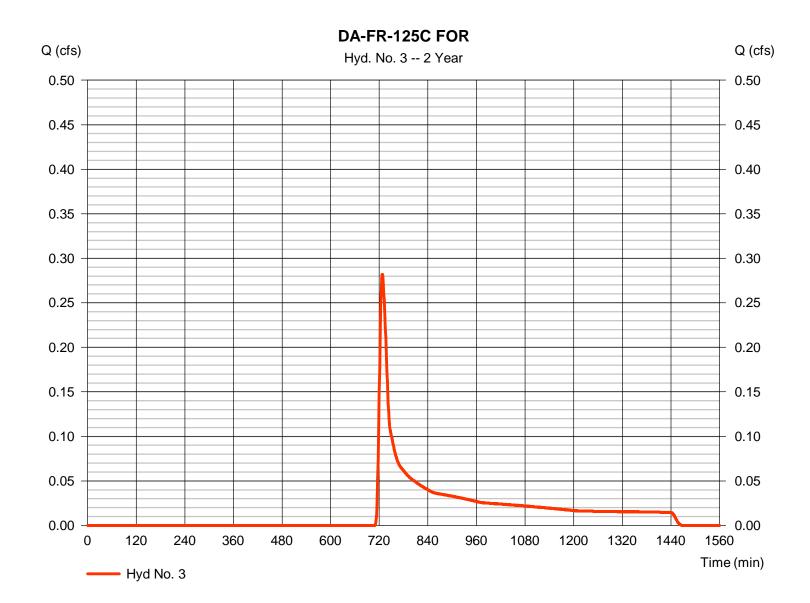
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-125C FOR

Hydrograph type = SCS Runoff Peak discharge = 0.282 cfsStorm frequency Time to peak = 727 min = 2 yrsTime interval = 1 min Hyd. volume = 1,477 cuftDrainage area Curve number = 0.990 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 16.90 min Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	1.426	1	725	4,775				DA-FR-125C PRE
2	SCS Runoff	0.874	1	731	4,111				DA-FR-125C DEV

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

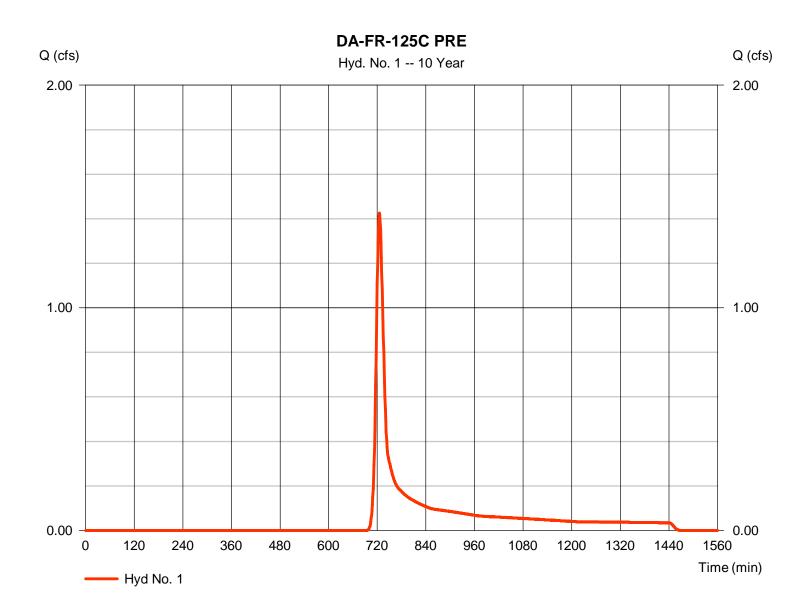
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-125C PRE

= SCS Runoff Hydrograph type Peak discharge = 1.426 cfsStorm frequency Time to peak = 725 min = 10 yrsTime interval = 1 min Hyd. volume = 4,775 cuftDrainage area Curve number = 0.987 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 16.90 min Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.568 x 48) + (0.419 x 58)] / 0.987



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

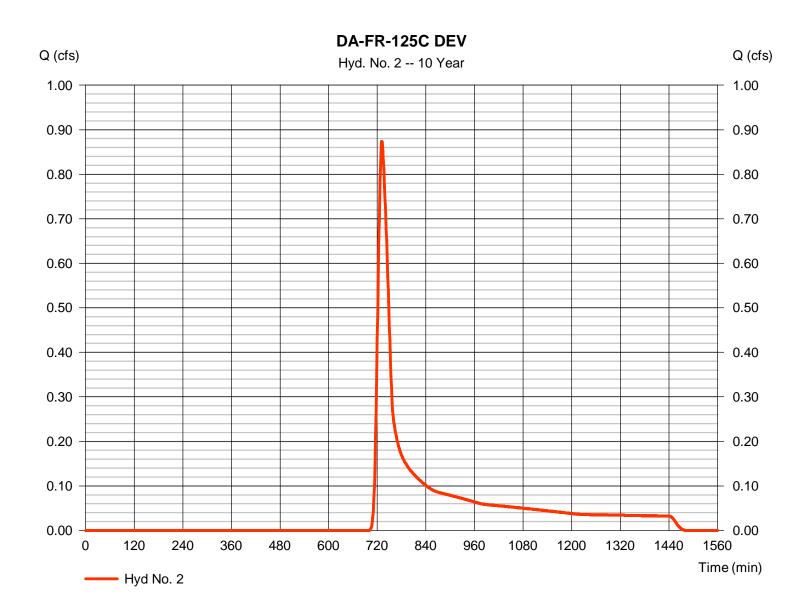
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-125C DEV

= SCS Runoff Hydrograph type Peak discharge = 0.874 cfsStorm frequency Time to peak = 731 min = 10 yrsTime interval = 1 min Hyd. volume = 4,111 cuftDrainage area Curve number = 0.990 ac= 52*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 25.70 min Total precip. = 5.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.568 \times 48) + (0.419 \times 58)] / 0.990$



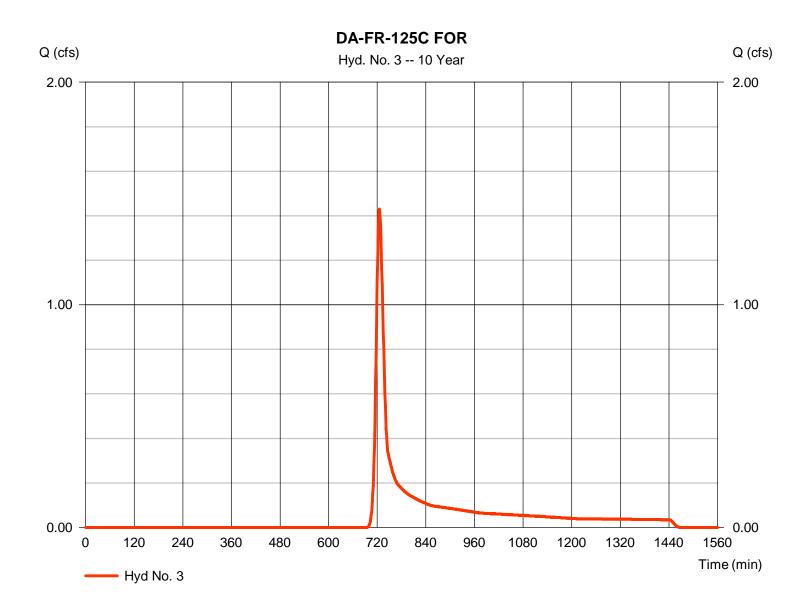
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-125C FOR

Hydrograph type = SCS Runoff Peak discharge = 1.430 cfsStorm frequency Time to peak = 725 min = 10 yrsTime interval = 1 min Hyd. volume = 4,791 cuftDrainage area Curve number = 0.990 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 16.90 min Total precip. Distribution = 5.70 in= Type II Storm duration = 484 = 24 hrs Shape factor



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Return Period								
(Yrs)	В	D	E	(N/A)				
1	0.0000	0.0000	0.0000					
2	69.8703	13.1000	0.8658					
3	0.0000	0.0000	0.0000					
5	79.2597	14.6000	0.8369					
10	88.2351	15.5000	0.8279					
25	102.6072	16.5000	0.8217					
50	114.8193	17.2000	0.8199					
100	127.1596	17.8000	0.8186					

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return		Intensity Values (in/hr)										
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc|

		Rainfall Precipitation Table (in)									
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr			
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00			
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			



ENERGY BALANCE METHOD

		ts

	1-Yr Event					
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)				
Pre-Developed Condition	0.344	1448				
Developed Condition	0.141	1052				
Pre-Developed (Forest) Condition	0.272	1296				

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.8

Calculations:	Check #1:	$Q_{developed} \leq IF \ x \ [(Q_{pre-developed} \ x \ RV_{pre-developed}) \ / \ RV_{developed}] \>$	0.141	≤ OK	0.379
	Check #2:	$Q_{developed} \le Q_{pre-developed}$	0.141	≤ OK	0.344
	Check #3:	$Q_{developed} shall not$ be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed}$ >	0.141	<u>shall not</u> be required to be ≤	0.335

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	·
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	<mark>0.40</mark>
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 – Manning's n Values for Open Channel Flow

nan	nel T	уре	Manning n					
			Min.	Normal	Max			
1.	Exc	avated or Dredged Channels¹						
	a.	Earth, Straight, and Uniform:						
		Clean, recently completed	0.016	0.018	0.020			
		Clean, after weathering	0.018	0.022	0.02			
		Gravel, uniform section, clean	0.022	0.025	0.030			
		With short grass, few weeds	0.022	0.027	0.03			
	b.	Earth Winding and Sluggish:						
		No vegetation	0.023	0.025	0.030			
		Grass, some weeds	0.025	0.030	0.03			
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040			
		Earth bottom and rubble sides	0.028	0.030	0.03			
		Stony bottom and weedy banks	0.025	0.035	0.040			
		Cobble bottom and clean sides	0.030	<mark>0.040</mark>	0.050			
	c.	Dragline-Excavated or Dredged:		•				
		No vegetation	0.025	0.028	0.03			
		Light brush on banks	0.035	0.050	0.06			
	d.	Rock Cuts:						
		Smooth and uniform	0.025	0.035	0.040			
		Jagged and irregular	0.035	0.040	0.050			
	e.	Channels not Maintained, Weeds and Brush Uncut:						
		Dense weeds, high as flow depth	0.050	0.080	0.120			
		Clean bottom, brush on sides	0.040	0.050	0.080			
		Same as above, highest stage of flow	0.045	0.070	0.110			
		Dense brush, high stage	0.080	0.100	0.140			
2.	Mai	n Channels²						
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	0.030	0.03			
	b.	Same as above, but more stones and weeds	0.030	0.035	0.04			
	c.	Clean, winding, some pools and shoals	0.033	0.040	0.04			
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050			
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.05			
	f.	Same as (d) with more stones	0.045	0.050	0.06			
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080			
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150			

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs. Sources:

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	. 2
1 - Year	
Summary Report	. 3
Hydrograph Reports	. 4
Hydrograph ReportsHydrograph No. 1, SCS Runoff, DA-FR-125D PRE	. 4
TR-55 Tc Worksheet	. 5
Hydrograph No. 2, SCS Runoff, DA-FR-125D DEV	. 6
TR-55 Tc Worksheet	. 7
Hydrograph No. 3, SCS Runoff, DA-FR-125D FOR	. 8
TR-55 Tc Worksheet	. 9
2 - Year	
Summary Report	10
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-125D PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-125D DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-125D FOR	
10 - Year	
Summary Report	
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-125D PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-125D DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-125D FOR	17
IDF Report	18

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5 <u>Legend</u> Hyd. Origin **Description** SCS Runoff DA-FR-125D PRE 2 SCS Runoff DA-FR-125D DEV SCS Runoff DA-FR-125D FOR Monday, 08 / 21 / 2017 Project: DA-FR-125D_Hydraflow.gpw

Hydrograph Return Period Recap

	Hydrograph	Inflow				Hydrograph						
No.	type (origin)		hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		0.344	0.637			2.760				DA-FR-125D PRE	
2	SCS Runoff		0.141	0.336			2.076				DA-FR-125D DEV	
3	SCS Runoff		0.272	0.540			2.565				DA-FR-125D FOR	

Proj. file: DA-FR-125D_Hydraflow.gpw

Monday, 08 / 21 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.344	1	722	1,448				DA-FR-125D PRE
2	SCS Runoff	0.141	1	725	1,052				DA-FR-125D DEV
	-FR-125D_H					Period: 1 Ye			3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

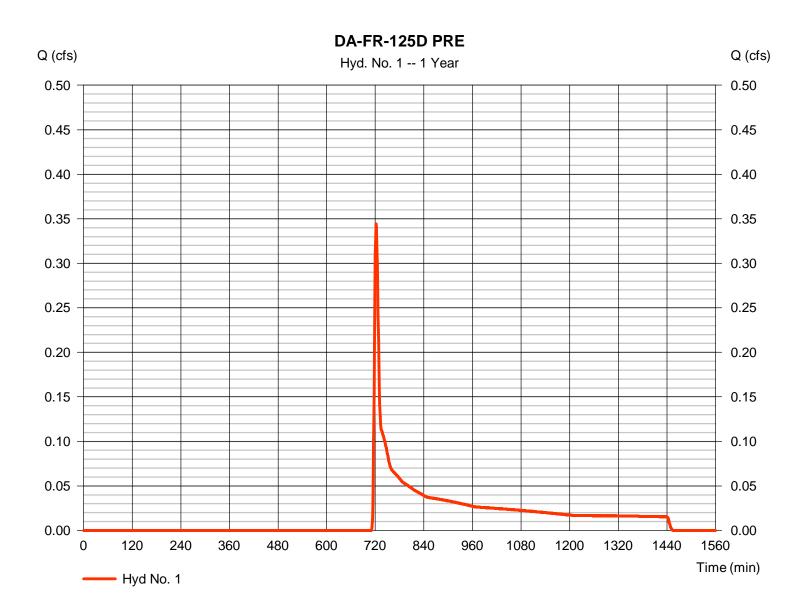
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-125D PRE

Hydrograph type = SCS Runoff Peak discharge = 0.344 cfsStorm frequency Time to peak = 722 min = 1 yrsTime interval = 1 min Hyd. volume = 1,448 cuftDrainage area Curve number = 1.280 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 9.80 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.038 x 82) + (1.239 x 55)] / 1.280



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1

DA-FR-125D PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 12.79 = 9.51	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	_	9.51
	= 9.51	Т	0.00	Т	0.00	-	3.31
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 111.59 = 20.86 = Unpave =7.37	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.25	+	0.00	+	0.00	=	0.25
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							9.80 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

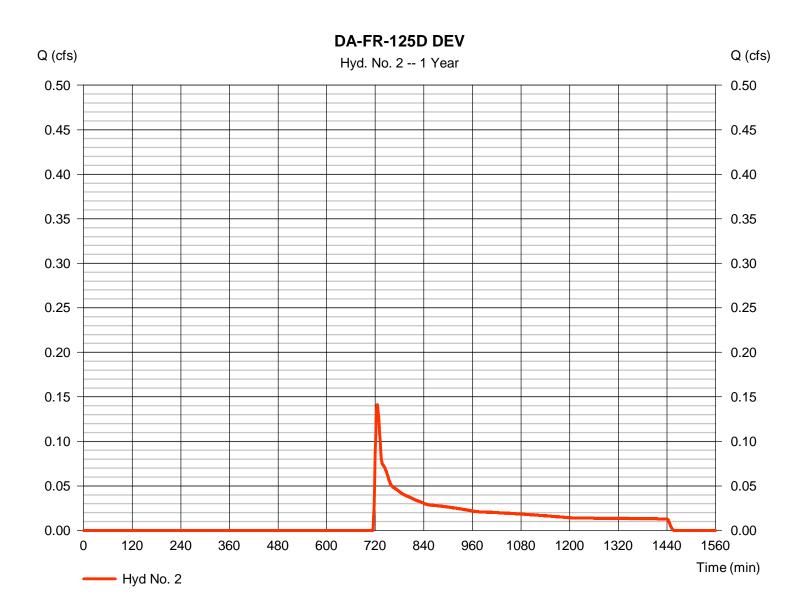
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-125D DEV

Hydrograph type = SCS Runoff Peak discharge = 0.141 cfsStorm frequency Time to peak = 725 min = 1 yrsTime interval = 1 min Hyd. volume = 1.052 cuftDrainage area Curve number = 1.270 ac= 53*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 11.70 min Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.756 \times 48) + (0.038 \times 82) + (0.478 \times 58)] / 1.270$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2

DA-FR-125D DEV

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.800 = 14.0 = 3.70 = 11.45		0.400 78.9 3.70 12.68		0.400 0.0 0.00 12.68		
Travel Time (min)	= 3.59	+	7.90	+	0.00	=	11.48
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 0.00 = 0.00 = Unpave =0.00	ed	109.44 20.66 Unpave 7.33	d	0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.00	+	0.25	+	0.00	=	0.25
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 2.00 = 4.47 = 5.00 = 0.040 =4.86		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})3.2		0.0		0.0		
Travel Time (min)	= 0.01	+	0.00	+	0.00	=	0.01
Total Travel Time, Tc							11.70 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

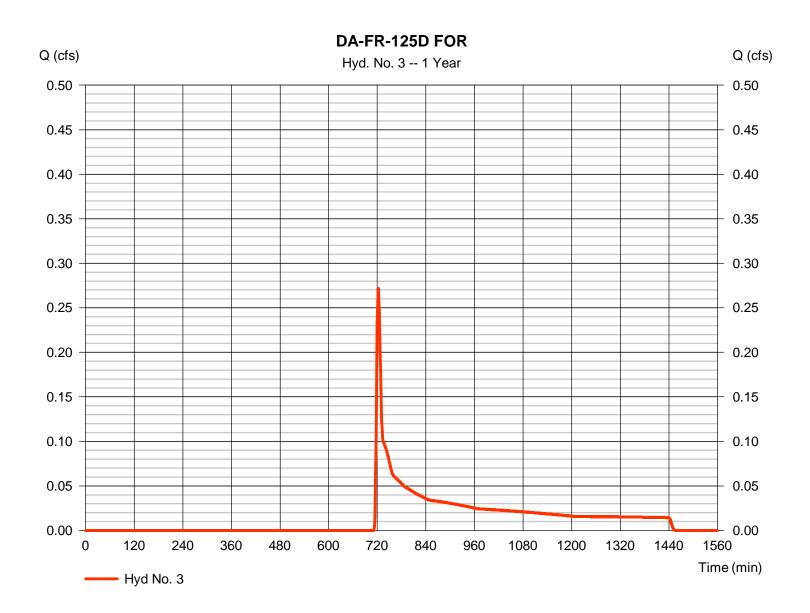
Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-125D FOR

Hydrograph type = SCS Runoff Peak discharge = 0.272 cfsStorm frequency Time to peak = 723 min = 1 yrsTime interval = 1 min Hyd. volume = 1,296 cuftDrainage area Curve number = 1.270 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 9.80 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.038 x 82) + (1.234 x 55)] / 1.270



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5 $\,$

Hyd. No. 3

DA-FR-125D FOR

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 12.79		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 9.51	+	0.00	+	0.00	=	9.51
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 111.59 = 20.86 = Unpave =7.37	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.25	+	0.00	+	0.00	=	0.25
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							9.80 min

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.637	1	721	2,108				DA-FR-125D PRE
2	SCS Runoff	0.336	1	724	1,612				DA-FR-125D DEV
	-FR-125D_H					Period: 2 Ye			3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

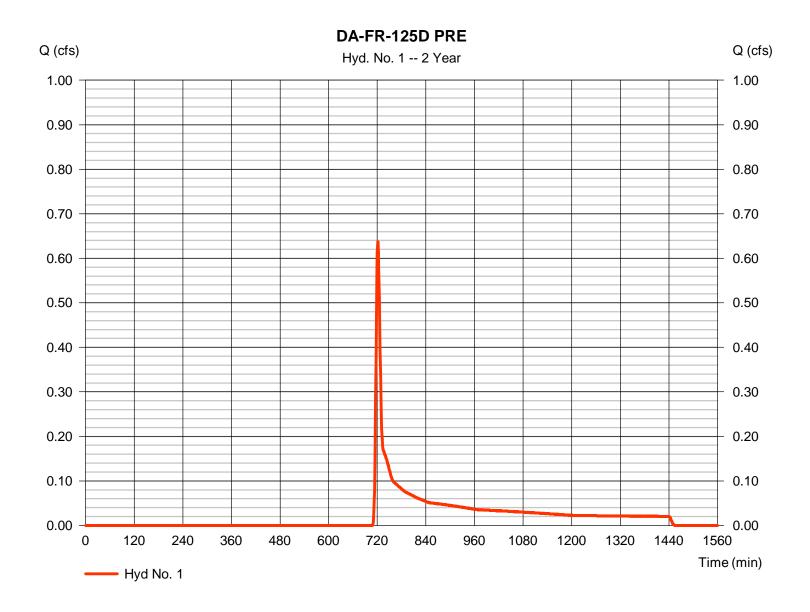
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-125D PRE

Hydrograph type = SCS Runoff Peak discharge = 0.637 cfsStorm frequency Time to peak = 721 min = 2 yrsTime interval = 1 min Hyd. volume = 2,108 cuftDrainage area Curve number = 1.280 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 9.80 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.038 x 82) + (1.239 x 55)] / 1.280



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

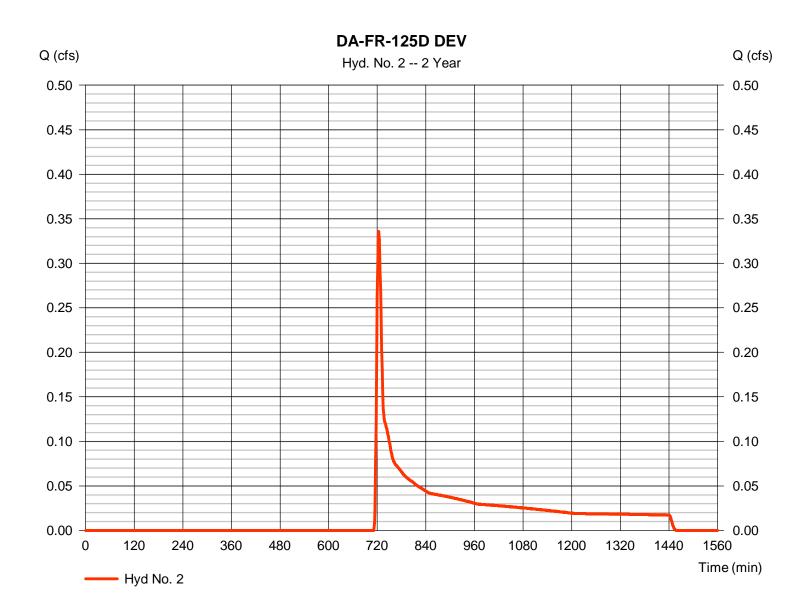
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-125D DEV

Hydrograph type = SCS Runoff Peak discharge = 0.336 cfsStorm frequency Time to peak = 724 min = 2 yrsTime interval = 1 min Hyd. volume = 1.612 cuftDrainage area Curve number = 1.270 ac= 53*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 11.70 min Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = $[(0.756 \times 48) + (0.038 \times 82) + (0.478 \times 58)] / 1.270$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

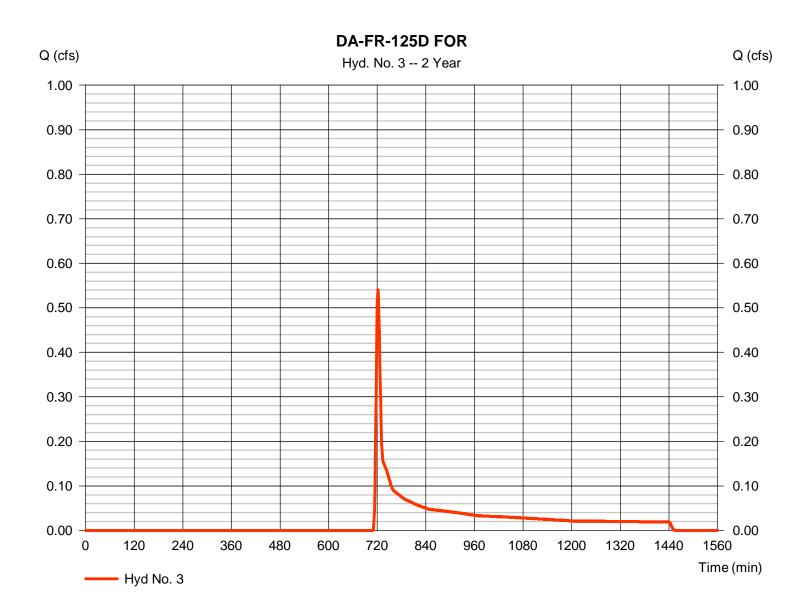
Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-125D FOR

Hydrograph type = SCS Runoff Peak discharge = 0.540 cfsStorm frequency Time to peak = 722 min = 2 yrsTime interval = 1 min Hyd. volume = 1,916 cuftDrainage area Curve number = 1.270 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 9.80 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 484 $= 24 \, hrs$

^{*} Composite (Area/CN) = [(0.038 x 82) + (1.234 x 55)] / 1.270



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	2.760	1	720	6,608				DA-FR-125D PRE
2	SCS Runoff	2.076	1	721	5,654				DA-FR-125D DEV
	FR-125D_H					Period: 10 \			3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

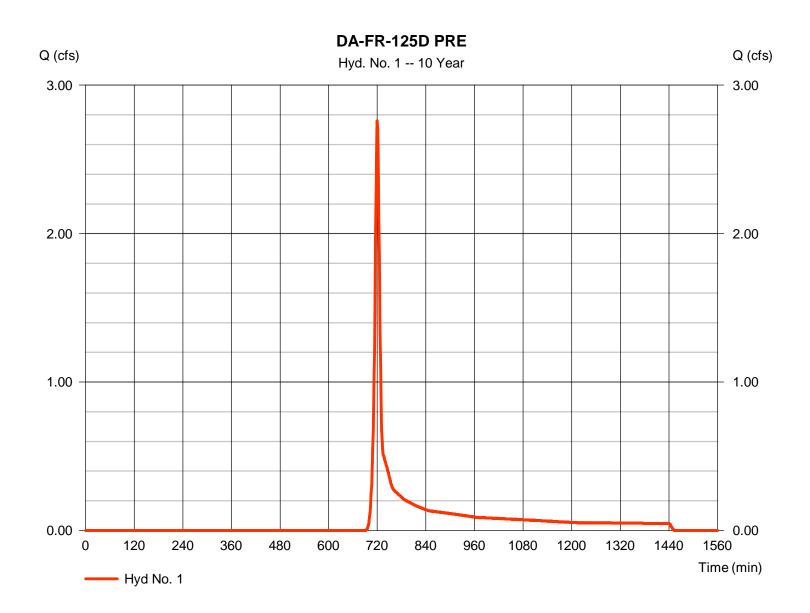
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-125D PRE

= SCS Runoff Hydrograph type Peak discharge = 2.760 cfsStorm frequency Time to peak = 720 min = 10 yrsTime interval = 1 min Hyd. volume = 6,608 cuftDrainage area Curve number = 1.280 ac= 56*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 9.80 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.038 x 82) + (1.239 x 55)] / 1.280



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

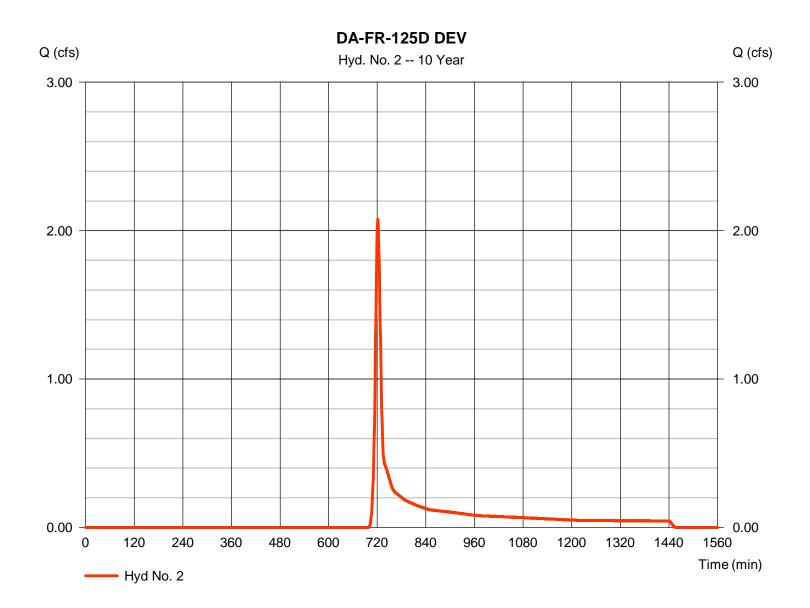
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-125D DEV

= SCS Runoff Hydrograph type Peak discharge = 2.076 cfsStorm frequency Time to peak = 721 min = 10 yrsTime interval = 1 min Hyd. volume = 5.654 cuftCurve number Drainage area = 1.270 ac= 53*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 11.70 min Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.756 \times 48) + (0.038 \times 82) + (0.478 \times 58)] / 1.270$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

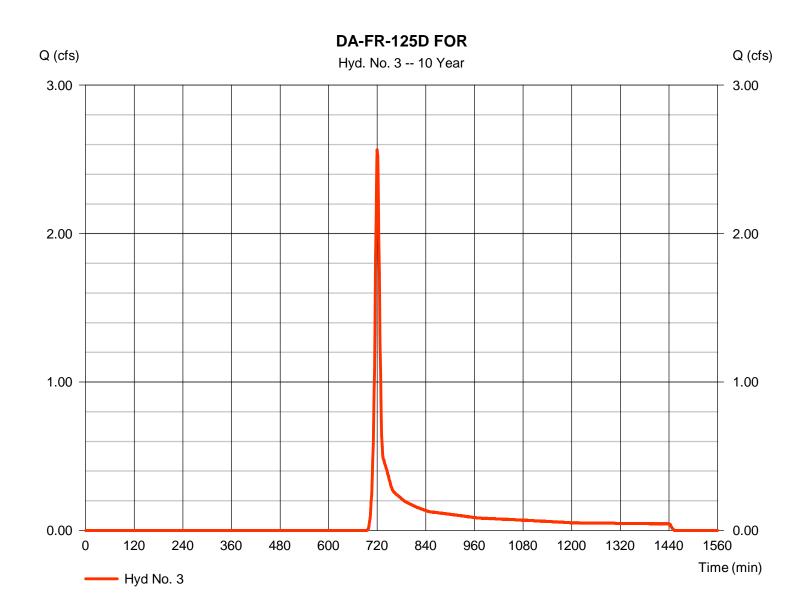
Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-125D FOR

= SCS Runoff Hydrograph type Peak discharge = 2.565 cfsStorm frequency Time to peak = 720 min = 10 yrsTime interval = 1 min Hyd. volume = 6.217 cuftDrainage area Curve number = 1.270 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 9.80 \, \text{min}$ Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.038 x 82) + (1.234 x 55)] / 1.270



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)						
(Yrs)	В	D	E	(N/A)			
1	0.0000	0.0000	0.0000				
2	69.8703	13.1000	0.8658				
3	0.0000	0.0000	0.0000				
5	79.2597	14.6000	0.8369				
10	88.2351	15.5000	0.8279				
25	102.6072	16.5000	0.8217				
50	114.8193	17.2000	0.8199				
100	127.1596	17.8000	0.8186				

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return	Intensity Values (in/hr)											
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc

		Rainfall Precipitation Table (in)								
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr		
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00		
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		

DA-FR-126

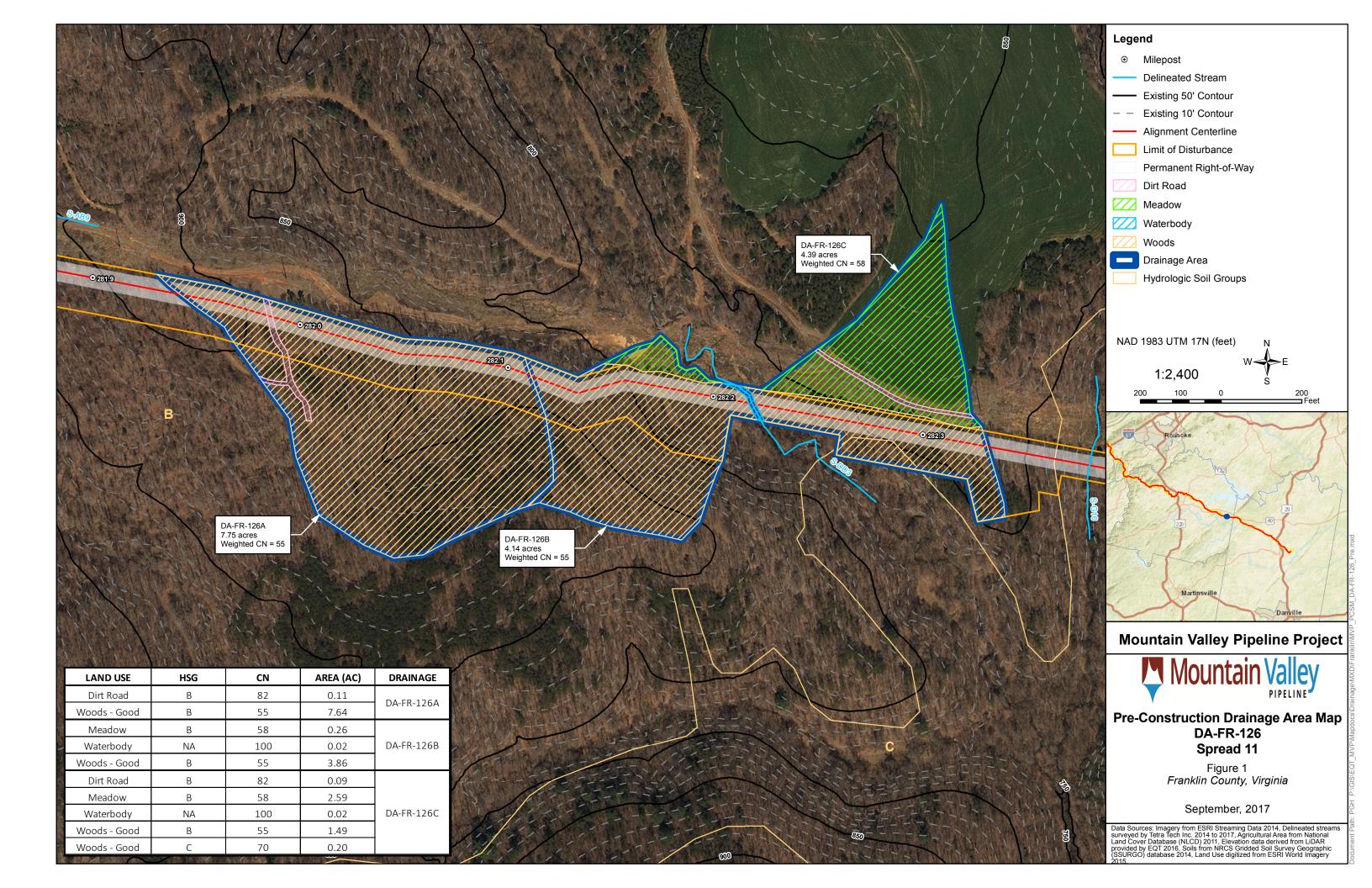
DA-FR-126 is located in a meadow and forested areas with hilly slopes and contains an existing gravel road. No new impervious area is proposed within DA-FR-126. The total phosphorus load reduction required for DA-FR-126 is -0.76 lb/yr. Multiple points of analysis were evaluated within DA-FR-126 to evaluate the effects on each receiving stream/channel following construction. Specifically, DA-FR-126 was sub-divided into three sub-drainage areas (sub areas A through C).

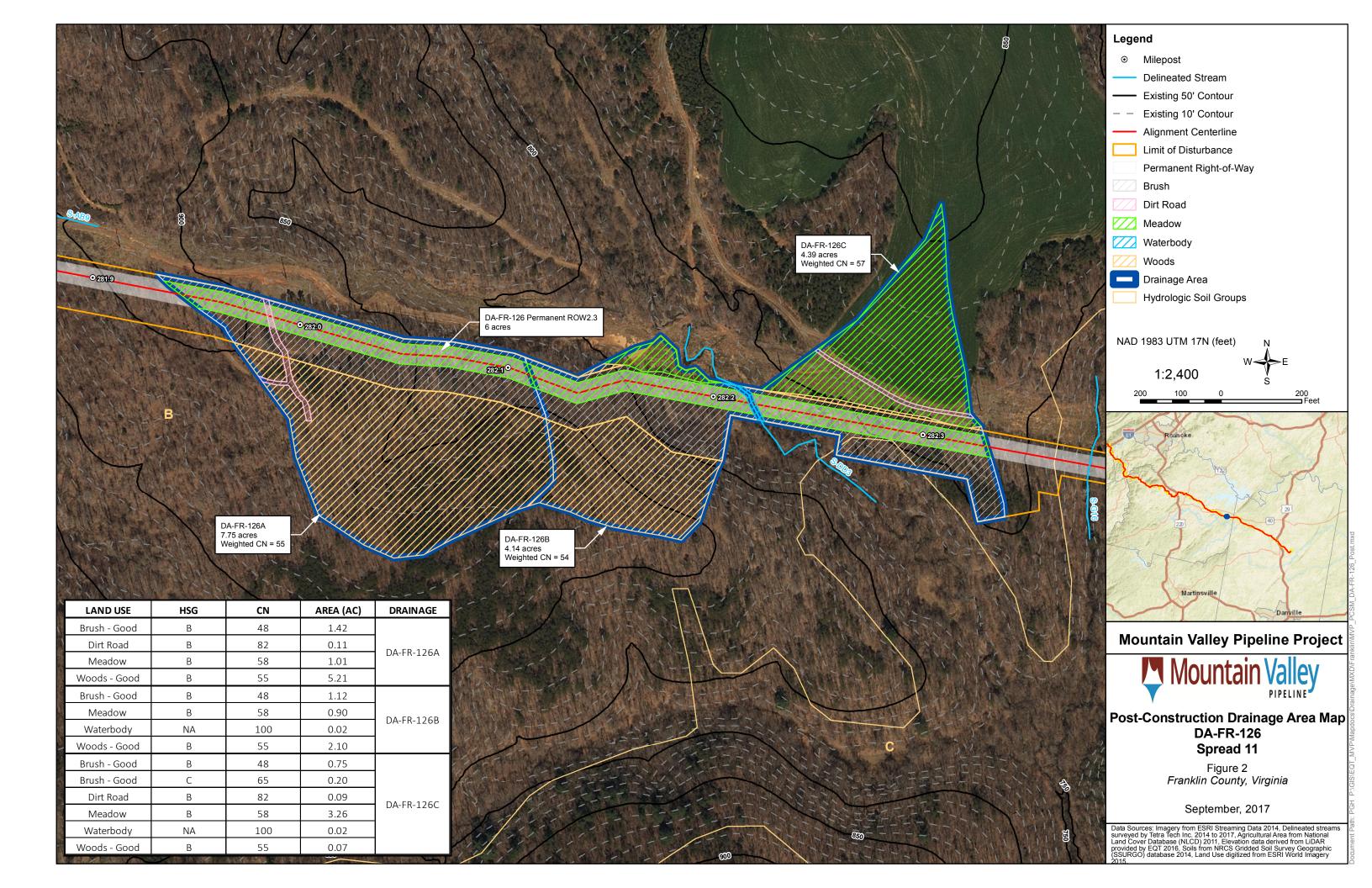
Stormwater quantity is met via the energy balance method for each of the three subareas DA-FR-126A, DA-FR-126B and DA-FR-126C. BMPs utilized in sub-area 126A included six water bars and associated soil amendments.

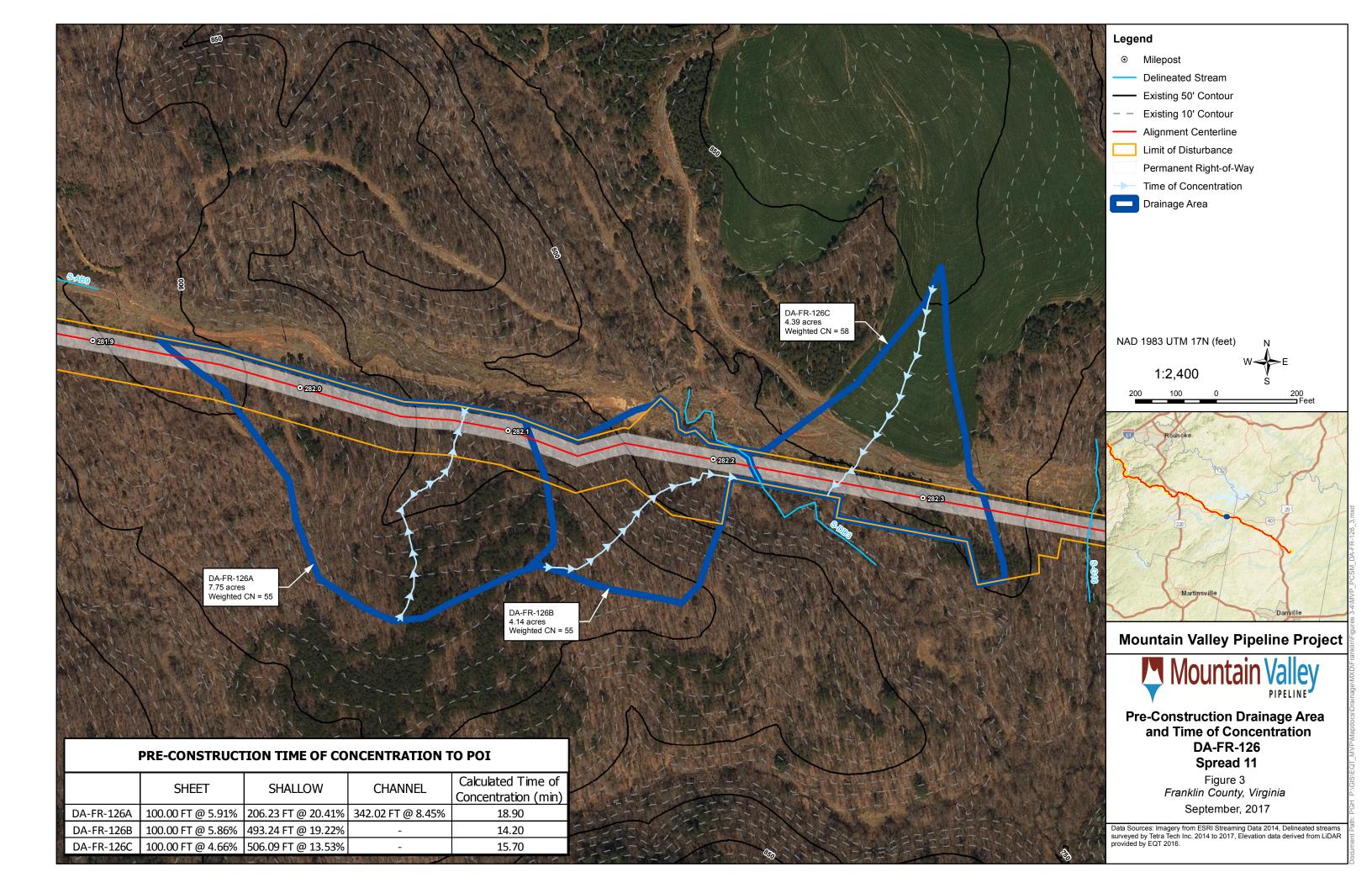
In addition, the Hydraflow Hydrograph's 10-year 24-hour peak discharge results indicate a reduction in flows ranging from 0.05 to 0.50 cfs for all drainage areas (as seen in table below).

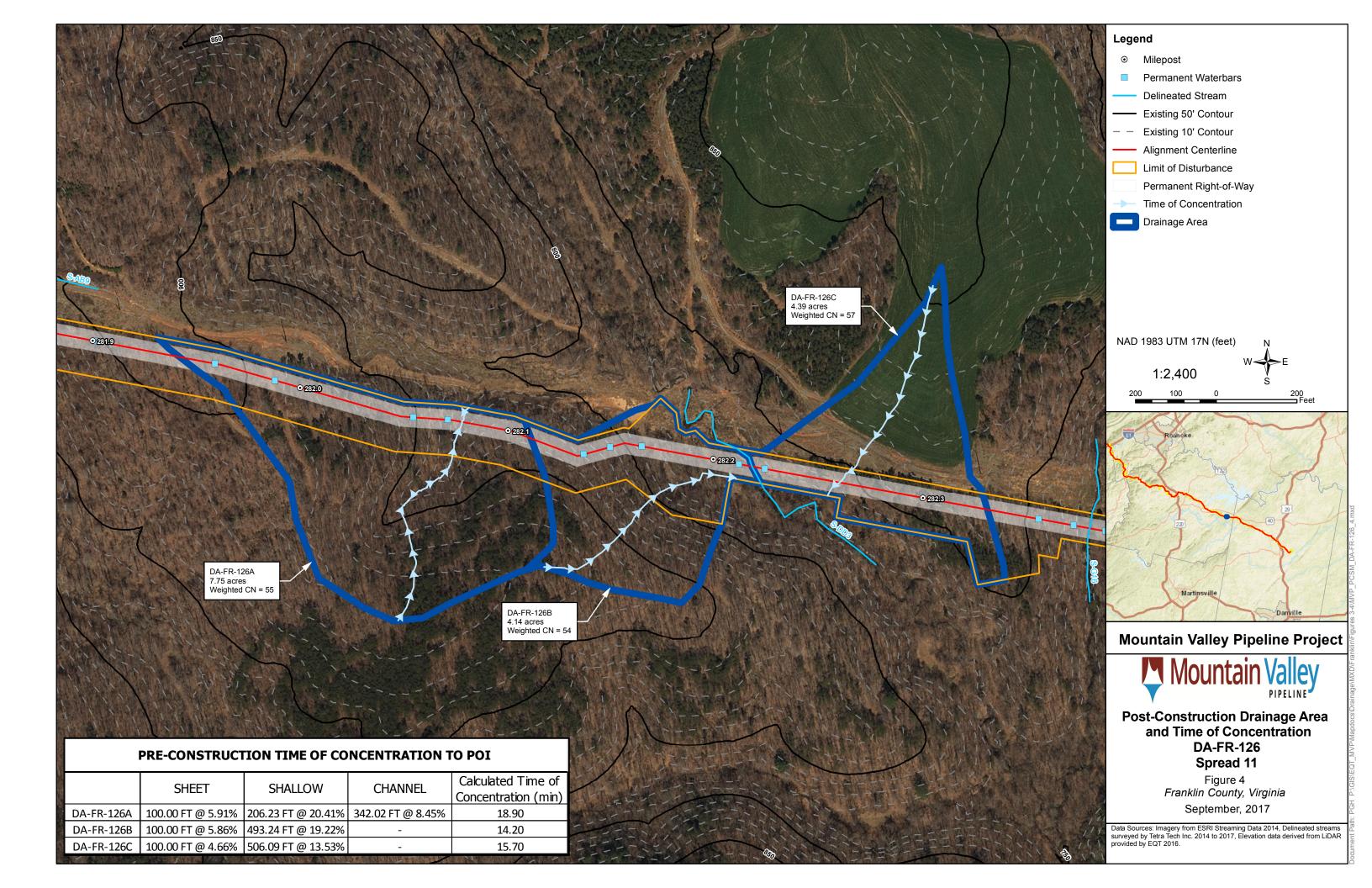
Sub Area	Pre Peak Flow,	Post Peak Flow, Q	Flow differential
	10-yr Q (cfs)	10-yr (cfs)	
DA-FR-126A	10.63	10.58	-0.05
DA-FR-126B	6.78	6.30	-0.48
DA-FR-126C	8.27	7.77	-0.50

Figures and calculations for each of the sub-areas for DA-FR-126 follow. See Appendix D of the Annual Standards and Specifications for further detail on stormwater methodology.









DEQ Virginia Runoff Reduction Method Re-Development Compliance Spreadsheet - Version 3.0

BMP Design Specifications List: 2013 Draft Stds & Specs

Site Summary - Linear Development Project***

Total Rainfall (in):	43
Total Disturbed Acreage:	2.36

Site Land Cover Summary

Pre-ReDevelopment Land Cover (acres)

000000000000000000000000000000000000000	A soils	B Soils	C Soils	D Soils	Totals	% of Total
Forest/Open (acres)	0.00	2.34	0.00	0.00	2.34	99
Managed Turf (acres)	0.00	0.00	0.00	0.00	0.00	0
Impervious Cover (acres)	0.00	0.02	0.00	0.00	0.02	1
					2.36	100

Post-ReDevelopment Land Cover (acres)

~	A soils	B Soils	C Soils	D Soils	Totals	% of Total
Forest/Open (acres)	0.00	2.34	0.00	0.00	2.34	99
Managed Turf (acres)	0.00	0.00	0.00	0.00	0.00	0
Impervious Cover (acres)	0.00	0.02	0.00	0.00	0.02	1
* Forest/Open Space areas must be protected in	2.36	100				

Site Tv and Land Cover Nutrient Loads

	Final Post-Development (Post-ReDevelopment & New Impervious)	Post- ReDevelopment	Post- Development (New Impervious)	Adjusted Pre- ReDevelopment
Site Rv	0.04	0.04		0.04
Treatment Volume (ft ³)	324	324		324
TP Load (lb/yr)	0.20	0.20		0.20
	Baseline TP Load (lb/yr):	0.9676*	*Reduction below new	development load limitat

Pre- ReDevelopment TP Load per acre (lb/acre/yr)	Final Post-Development TP Load per acre (lb/acre/yr)	Post-ReDevelopment TP Load per acre (lb/acre/yr)
0.09	0.09	0.09

Total TP Load Reduction Required (lb/yr)	-0.76	N/A***	N/A***
--	-------	--------	--------

***This is a linear development project

	Final Post-Development Load (Post-ReDevelopment & New Impervious)	Pre- ReDevelopment
TN Load (lb/yr)	1.46	1.46

Site Compliance Summary - ***Linear Development Project

_			_
	Maximum % Reduction Required Below	200/	* Note: % Reduction will reduce post-development TP load to less than or equal to baseline load of 0.97 lb/yr (0.41 lb/ac/yr)
	Pre-ReDevelopment Load	20%	[Post-Dev Reduction Requirement = Post-Dev TP load - baseline load of 0.97 lb/yr], baseline load = site area x 0.41 lb/ac/yr

Total Runoff Volume Reduction (ft ³) 0

Total TP Load Reduction Achieved (lb/yr)	0.00
Total TN Load Reduction Achieved (lb/yr)	0.00
Remaining Post Development TP Load (lb/yr)	0.20
Remaining TP Load Reduction (lb/yr) Required	0.00

** TARGET TP REDUCTION EXCEEDED BY 0.76 LB/YEAR **

^{*}Reduction below new development load limitation not required



STORAGE VOLUME OF WATERBAR WITH SOIL COMPOST AMENDMENT AREA

Equations Used:

¹Vgravel storage = L*W*D_{gravel}*(40/100)

 2 Vsoil storage = L*W*D_{soil}*(20/100)

 3 Vsurface storage = [W*S*D^2]+[L*S*D^2]+[W*L*D]+[((2*S*D)^2*D)/3]

³Equation #1 under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, but calculation also takes into account surface side slopes.

Inputs:	Depth	of Gravel Layer, D _{gravel} (ft) =	0	
	Depth of Soil A	Amendment Area, D _{soil} (ft) =	1	Refer to Table 4.3 in VA DEQ Stormwater Design Specification No. 4; Note that compost amendment may not be necessary for HSG A/B soils
	Length of Waterbar So	il Amendment Area, L (ft) =	50	Assume max. length of 50' for waterbar soil amendment areas (i.e., limited to permanent ROW)
	Width of Waterbar Soil	Amendment Area, W (ft) =	3	
	Inside Embani	ment Side Slopes, S (H:V) =	2	_Assume 2H:1V surface side slopes for waterbars
	Number of Perm. Wate	erbars in Drainage Area, n =	6	
	Design I	nfiltration Rate, IR (in/hr) =	0.2	Min. rate of 0.30 in/hr for HSG A soils and 0.15-0.30 in/hr for HSG B soils (see Chap. 4, p. 4-30 in VA Stormwater Management Handbook Volume II (First Edition, 1999)
	Sur	face Ponding Depth, D (ft) =	0.5	Assume 0.5' CFS height at the end of waterbars
Calculations:		orage Depth per BMP (ft) =	1.5	
		rage Volume per BMP (cf) =	102	
	Subsurface Sto	rage Volume per BMP (cf) =	30	
	Total Sto	rage Volume per BMP (cf) =	132	
	Total BMP Storage Vol	ume in Drainage Area (cf) =	793	
	Calculated Infiltr	ation Period per BMP (hr) =	53	
		Depth-Storage D	ata	
Depth (ft)	Width (ft)	Length (ft)	Storage Volume per BMP (cf)	Storage Volume in Drainage Area (cf)
0	3	50	0	0
0.5	3	50	15	90
1	3	50	30	180
1.5	5	52	132	793
2	7	54	291	1748

ENERGY BALANCE METHOD

Inputs:

Calculations:

	1	-Yr Event
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)
Pre-Developed Condition	1.056	7908
Developed Condition	0.837	7114
Pre-Developed (Forest) Condition	0.467	7908

0.8

Improvement Factor, IF =

Check #1: $Q_{\text{developed}} \leq \mathsf{IF} \; x \; [(Q_{\text{pre-developed}} \, x \; \mathsf{RV}_{\text{pre-developed}}) \, / \; \mathsf{RV}_{\text{developed}}] \quad ------>$ 0.837 0.939 ≤ ОК Check #2: Q_{developed} ≤ Q_{pre-developed} ------0.837 1.056 ОК $Q_{developed} \underline{shall \ not}$ be required to be $\leq (Q_{forest} \times RV_{forest}) / RV_{developed}$ ---> Check #3: 0.837 <u>shall not</u> be required to be ≤ 0.519

¹Equation #2b under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, assuming that gravel is made up of 40% voids.

²Equation #2b under "Volume Reduction Calculations" in Section 6.4.5 of PA BMP Manual, assuming that soil compost amendment is made up of 20% voids.

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	·
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Table 2 - Manning's n Values for Open Channel Flow

nannel Type		уре	Manning n			
			Min.	Normal	Max.	
1.	Exc	cavated or Dredged Channels ¹				
	a.	Earth, Straight, and Uniform:				
		Clean, recently completed	0.016	0.018	0.020	
		Clean, after weathering	0.018	0.022	0.025	
		Gravel, uniform section, clean	0.022	0.025	0.030	
		With short grass, few weeds	0.022	0.027	0.033	
	b.	Earth Winding and Sluggish:				
		No vegetation	0.023	0.025	0.030	
		Grass, some weeds	0.025	0.030	0.033	
		Dense weeds or aquatic plants in deep channels	0.030	0.035	0.040	
		Earth bottom and rubble sides	0.028	0.030	0.035	
		Stony bottom and weedy banks	0.025	0.035	0.040	
		Cobble bottom and clean sides	0.030	0.040	0.050	
	c.	Dragline-Excavated or Dredged:		•		
		No vegetation	0.025	0.028	0.033	
		Light brush on banks	0.035	0.050	0.060	
	d.	Rock Cuts:		•		
		Smooth and uniform	0.025	0.035	0.040	
		Jagged and irregular	0.035	0.040	0.050	
	e.	Channels not Maintained, Weeds and Brush Uncut:		•		
		Dense weeds, high as flow depth	0.050	0.080	0.120	
		Clean bottom, brush on sides	0.040	0.050	0.080	
		Same as above, highest stage of flow	0.045	0.070	0.110	
		Dense brush, high stage	0.080	0.100	0.140	
2.	Mai	n Channels²		•		
	a.	Clean, straight, full stage, no rifts or deep pools	0.025	<mark>0.030</mark>	0.033	
	b.	Same as above, but more stones and weeds	0.030	0.035	0.040	
	c.	Clean, winding, some pools and shoals	0.033	0.040	0.045	
	d.	Same as above, but some weeds and stones	0.035	0.045	0.050	
	e.	Same as above, lower stages, more ineffective	0.040	0.048	0.055	
	f.	Same as (d) with more stones	0.045	0.050	0.060	
	g.	Sluggish reaches, weedy, deep pools	0.050	0.070	0.080	
	h.	Very weedy reaches, deep pools, or floodways with heavy stand of timber and underbrush	0.075	0.100	0.150	

Notes

²For the pre-developed condition (if applicable), a Manning's n value of 0.030 was used in Hydraflow Hydrographs.

Sources:

-ASCE, (1982), Gravity Sanitary Sewer Design and Construction, ASCE Manual of Practice No. 60, New York, NY

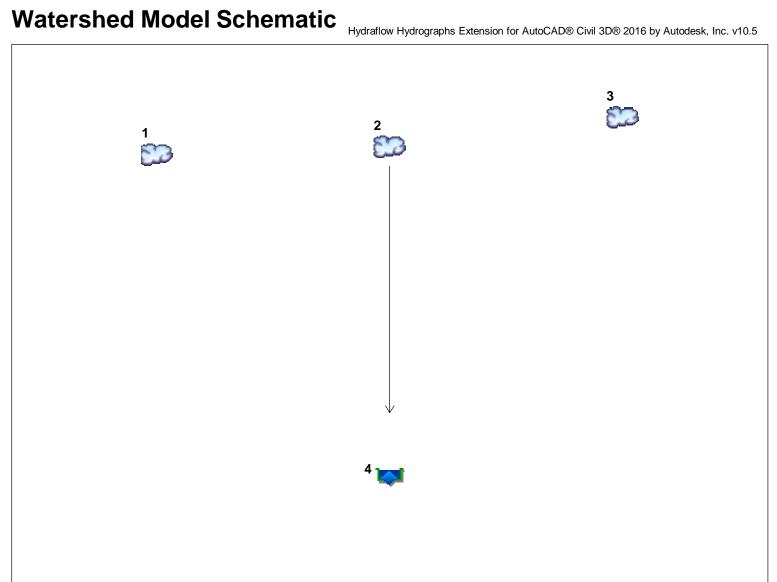
-Chow, V.T., (1959), Open Channel Hydraulics, McGraw-Hill, New York, NY

¹For the developed condition, a conservative Manning's n value of 0.040 was used in Hydraflow Hydrographs for open channel flow through the permanent waterbar or grass channel.

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	. 2
1 - Year	
Summary Report	. 3
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-126B PRE	. 4
TR-55 Tc Worksheet	. 5
Hydrograph No. 2, SCS Runoff, DA-FR-126B DEV	. 6
TR-55 Tc Worksheet	. 7
Hydrograph No. 3, SCS Runoff, DA-FR-126B FOR	
TR-55 Tc Worksheet	. 9
Hydrograph No. 4, Reservoir, WB Soil Amendments	10
Pond Report - Waterbar Soil Amendments	11
2 - Year Summary Report	14 14 15 16
10 - Year	
Summary Report	18
Hydrograph Reports	19
Hydrograph No. 1, SCS Runoff, DA-FR-126B PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-126B DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-126B FOR	
Hydrograph No. 4, Reservoir, WB Soil Amendments	22
IDE Report	23



<u>Legend</u>

<u>Hyd.</u>	<u>Origin</u>	<u>Description</u>
1	SCS Runoff	DA-FR-126B PRE
2	SCS Runoff	DA-FR-126B DEV
3	SCS Runoff	DA-FR-126B FOR
4	Reservoir	WB Soil Amendments

Project: DA-FR-126A_Hydraflow.gpw

Monday, 08 / 21 / 2017

Hydrograph Return Period Recap

	Hydrograph	Inflow				Peak Out	flow (cfs)				Hydrograph
о.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		1.056	2.084			10.63				DA-FR-126B PRE
2	SCS Runoff		1.056	2.084			10.63				DA-FR-126B DEV
3	SCS Runoff		0.467	0.808			3.717				DA-FR-126B FOR
4	Reservoir	2	0.837	1.946			10.58				WB Soil Amendments

Proj. file: DA-FR-126A_Hydraflow.gpw

Monday, 08 / 21 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	_			_		Trydranow Try	varographis Exteri	SIOIT IOI AUTOCAL	© CIVII 3D® 2016 by Autodesk, Inc. V10.5
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	1.056	1	730	7,908				DA-FR-126B PRE
2	SCS Runoff	1.056	1	730	7,908				DA-FR-126B DEV
3	SCS Runoff	0.467	1	793	7,908				DA-FR-126B FOR
4	Reservoir	0.837	1	740	7,114	2	101.57	934	WB Soil Amendments
DA-	-FR-126A_Hy	draflow.g	pw		Return P	eriod: 1 Ye	ear	Monday, 08	3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

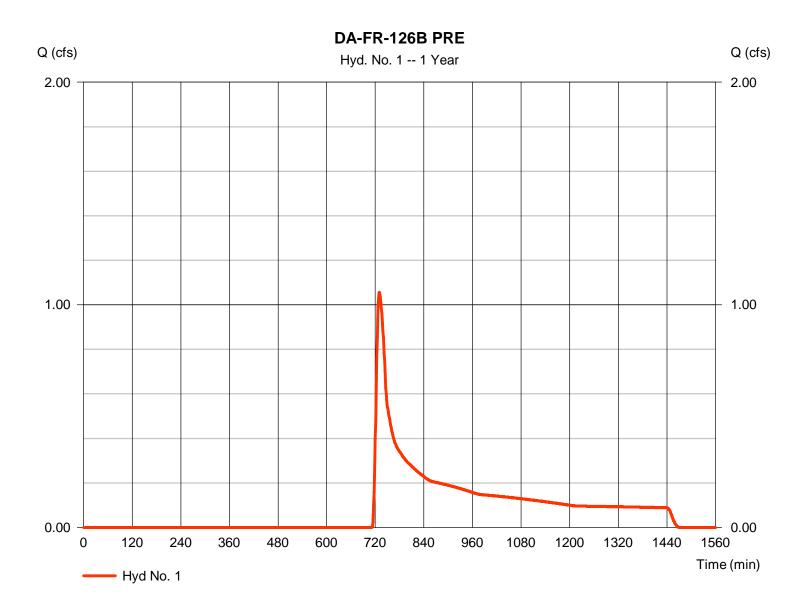
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-126B PRE

= SCS Runoff Hydrograph type Peak discharge = 1.056 cfsStorm frequency Time to peak = 730 min = 1 yrsTime interval = 1 min Hyd. volume = 7,908 cuftDrainage area Curve number = 7.750 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 18.90 min Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.112 x 82) + (7.642 x 55)] / 7.750



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1 DA-FR-126B PRE

<u>Description</u>	A	<u>B</u>	<u>C</u>	<u>Totals</u>
Sheet Flow				
Manning's n-value	= 0.400	0.011	0.011	
Flow length (ft)	= 100.0	0.0	0.0	
Two-year 24-hr precip. (in)	= 3.70	0.00	0.00	
Land slope (%)	= 5.91	0.00	0.00	
Travel Time (min)	= 12.95 +	0.00 +	0.00 =	12.95
Shallow Concentrated Flow				
Shallow Concentrated Flow Flow length (ft)	= 206.23	0.00	0.00	
	= 206.23 = 20.41	0.00 0.00	0.00 0.00	
Flow length (ft)				
Flow length (ft) Watercourse slope (%)	= 20.41	0.00	0.00	

Total Travel Time, Tc							18.90 min
Travel Time (min)	= 5.43	+	0.00	+	0.00	=	5.43
Flow length (ft)	({0})342.0		0.0		0.0		
			0.00		0.00		
Velocity (ft/s)	=1.05						
Manning's n-value	= 0.030		0.015		0.015		
Channel slope (%)	= 8.45		0.00		0.00		
Wetted perimeter (ft)	= 6.51		0.00		0.00		
Channel Flow X sectional flow area (sqft)	= 0.13		0.00		0.00		
Travel Time (min)	= 0.47	+	0.00	+	0.00	=	0.47
Surface description Average velocity (ft/s)	= Unpave =7.29	d	Paved 0.00		Paved 0.00		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

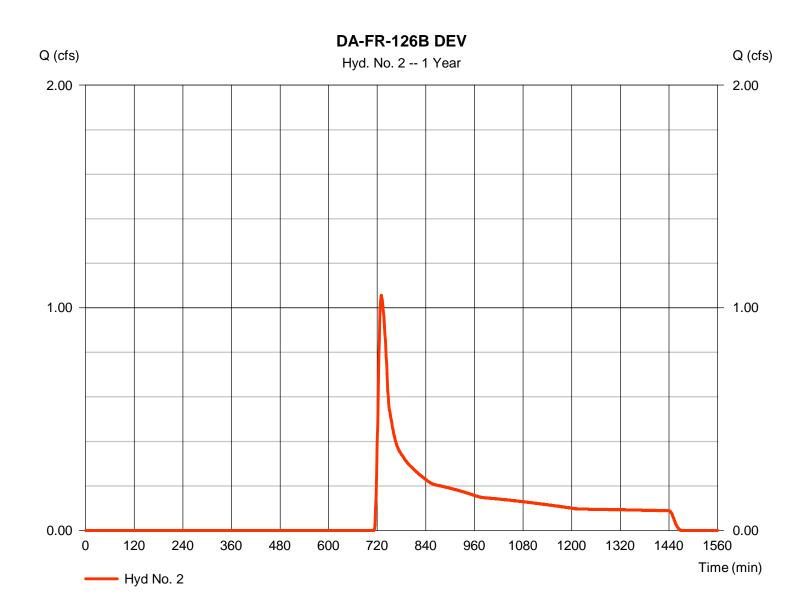
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-126B DEV

= SCS Runoff Hydrograph type Peak discharge = 1.056 cfsStorm frequency Time to peak = 730 min = 1 yrsTime interval = 1 minHyd. volume = 7,908 cuftDrainage area Curve number = 7.750 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 18.90 min Total precip. = 3.30 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(1.419 \times 48) + (0.112 \times 82) + (1.012 \times 58) + (5.210 \times 55)] / 7.750$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2

DA-FR-126B DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 5.91 = 12.95	+	0.011 0.0 0.00 0.00	+	0.011 0.0 0.00 0.00	_	12.95
, ,	- 12.00	•	0.00	•	0.00	_	12.50
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 206.23 = 20.41 = Unpaved =7.29		0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.47	+	0.00	+	0.00	=	0.47
Channel Flow X sectional flow area (sqft)	= 0.13		0.00		0.00		
Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 6.51 = 8.45 = 0.030 =1.05		0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Channel slope (%) Manning's n-value	= 8.45 = 0.030		0.00 0.00 0.015		0.00 0.00 0.015		
Channel slope (%) Manning's n-value Velocity (ft/s)	= 8.45 = 0.030 =1.05	+	0.00 0.00 0.015 0.00	+	0.00 0.00 0.015	=	5.43

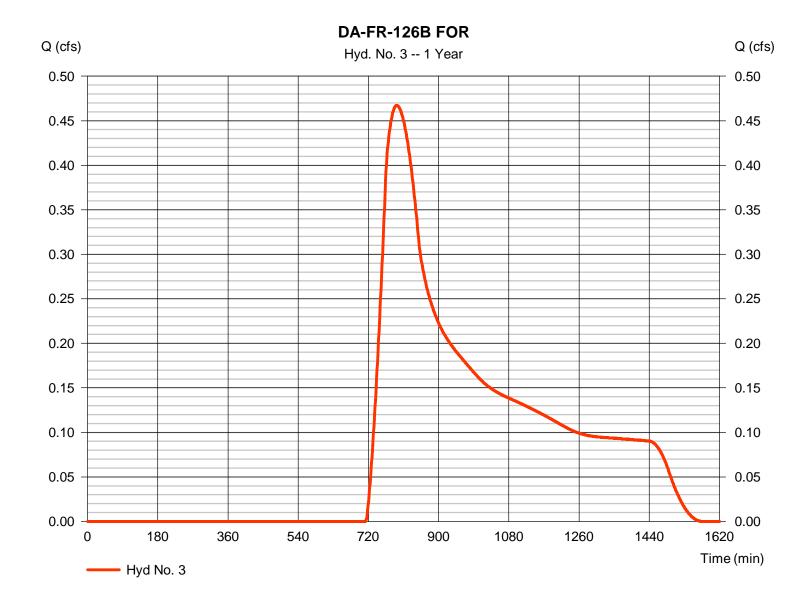
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-126B FOR

Hydrograph type = SCS Runoff Peak discharge = 0.467 cfsStorm frequency Time to peak = 793 min = 1 yrsTime interval = 1 min Hyd. volume = 7,908 cuftDrainage area Curve number = 7.750 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 85.90 min Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3

DA-FR-126B FOR

<u>Description</u>	A		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 5.91		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00		
Travel Time (min)	= 12.95	+	0.00	+	0.00	=	12.95
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 206.23 = 20.41 = Unpaved =7.29	t	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 0.47	+	0.00	+	0.00	=	0.47
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.13 = 6.51 = 8.45 = 0.400 =0.08		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})342.0		0.0		0.0		
Travel Time (min)	= 72.47	+	0.00	+	0.00	=	72.47
Total Travel Time, Tc							85.90 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

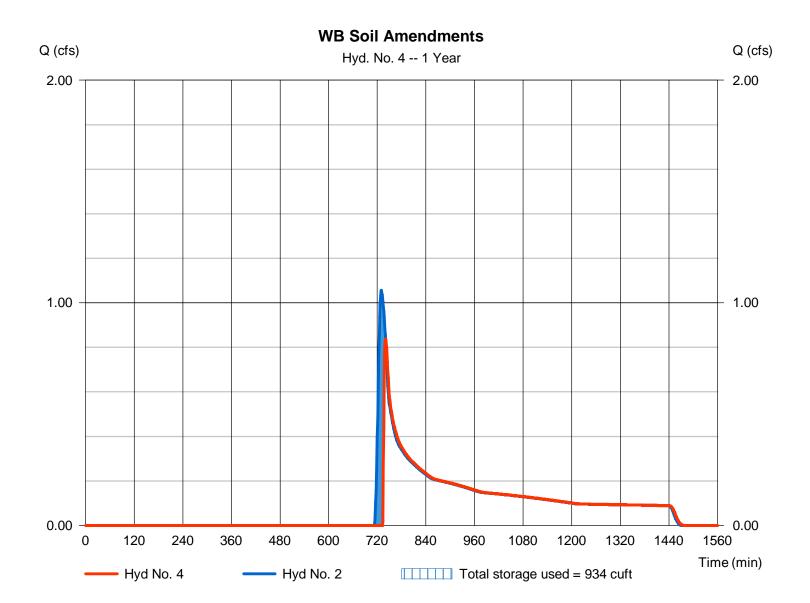
Monday, 08 / 21 / 2017

Hyd. No. 4

WB Soil Amendments

Hydrograph type = Reservoir Peak discharge = 0.837 cfsStorm frequency Time to peak = 740 min = 1 yrsTime interval = 1 minHyd. volume = 7,114 cuftInflow hyd. No. Max. Elevation = 2 - DA-FR-126B DEV = 101.57 ft= 934 cuft Reservoir name = Waterbar Soil Amendments Max. Storage

Storage Indication method used.



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Pond No. 1 - Waterbar Soil Amendments

Pond Data

Pond storage is based on user-defined values.

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	100.00	n/a	0	0
0.50	100.50	n/a	90	90
1.00	101.00	n/a	90	180
1.50	101.50	n/a	613	793
2.00	102.00	n/a	955	1,748

Culvert / Orifice Structures Weir Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 0.00	0.00	0.00	0.00	Crest Len (ft)	= 12.00	0.00	0.00	0.00
Span (in)	= 0.00	0.00	0.00	0.00	Crest El. (ft)	= 101.50	0.00	0.00	0.00
No. Barrels	= 0	0	0	0	Weir Coeff.	= 3.33	3.33	3.33	3.33
Invert El. (ft)	= 0.00	0.00	0.00	0.00	Weir Type	= Rect			
Length (ft)	= 0.00	0.00	0.00	0.00	Multi-Stage	= No	No	No	No
Slope (%)	= 0.00	0.00	0.00	n/a					
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by	Wet area)		
Multi-Stage	= n/a	No	No	No	TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Dis	scharge Labl	е
-----------------------	--------------	---

Stage ft	Storage cuft	Elevation ft	Clv A cfs	CIv B cfs	CIv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	100.00					0.00						0.000
0.05	9	100.05					0.00						0.000
0.10	18	100.10					0.00						0.000
0.15	27	100.15					0.00						0.000
0.20	36	100.20					0.00						0.000
0.25	45	100.25					0.00						0.000
0.30	54	100.30					0.00						0.000
0.35	63	100.35					0.00						0.000
0.40	72	100.40					0.00						0.000
0.45	81	100.45					0.00						0.000
0.50	90	100.50					0.00						0.000
0.55	99	100.55					0.00						0.000
0.60	108	100.60					0.00						0.000
0.65	117	100.65					0.00						0.000
0.70	126	100.70					0.00						0.000
0.75	135	100.75					0.00						0.000
0.80	144	100.80					0.00						0.000
0.85	153	100.85					0.00						0.000
0.90	162	100.90					0.00						0.000
0.95	171	100.95					0.00						0.000
1.00	180	101.00					0.00						0.000
1.05	241	101.05					0.00						0.000
1.10	303	101.10					0.00						0.000
1.15	364	101.15					0.00						0.000
1.20	425	101.20					0.00						0.000
1.25	486	101.25					0.00						0.000
1.30	548	101.30					0.00						0.000
1.35	609	101.35					0.00						0.000
1.40	670	101.40					0.00						0.000
1.45	732	101.45					0.00						0.000
1.50	793	101.50					0.00						0.000
1.55	889	101.55					0.45						0.447
1.60	984	101.60					1.26						1.264
1.65	1,080	101.65					2.32						2.322
1.70	1,175	101.70					3.57						3.574
1.75	1,271	101.75					5.00						4.995
1.80	1,366	101.80					6.57						6.567
1.85	1,462	101.85					8.27						8.275
1.90	1,557	101.90					10.11						10.11
											Continue	es on next	t page

Waterbar Soil Amendments Stage / Storage / Discharge Table

Stage	Storage	Elevation	CIv A	Clv B	CIv C	PrfRsr	Wr A	Wr B	Wr C	Wr D	Exfil	User	Total
ft	cuft	ft	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs
1.95 2.00	1,653 1,748	101.95 102.00					12.06 14.13						12.06 14.13

...End

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

lyd. lo.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	2.084	1	729	11,691				DA-FR-126B PRE
2	SCS Runoff	2.084	1	729	11,691				DA-FR-126B DEV
3	SCS Runoff	0.808	1	784	11,691				DA-FR-126B FOR
4	Reservoir	1.946	1	733	10,898	2	101.63	1,046	WB Soil Amendments
DA-FR-126A_Hydraflow.gpw						Period: 2 Ye	ear	Monday, 08	3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

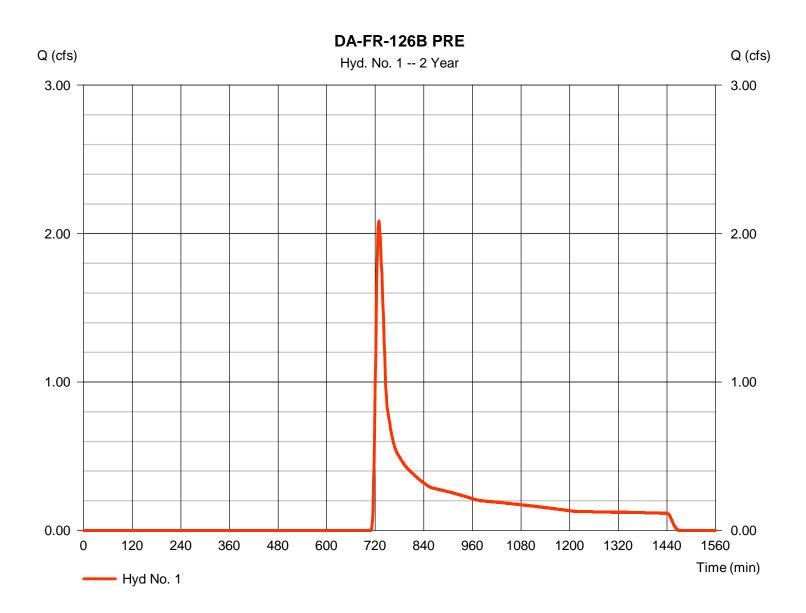
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-126B PRE

= SCS Runoff Hydrograph type Peak discharge = 2.084 cfsStorm frequency Time to peak = 729 min = 2 yrsTime interval = 1 min Hyd. volume = 11.691 cuft Curve number Drainage area = 7.750 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 18.90 min Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.112 x 82) + (7.642 x 55)] / 7.750



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

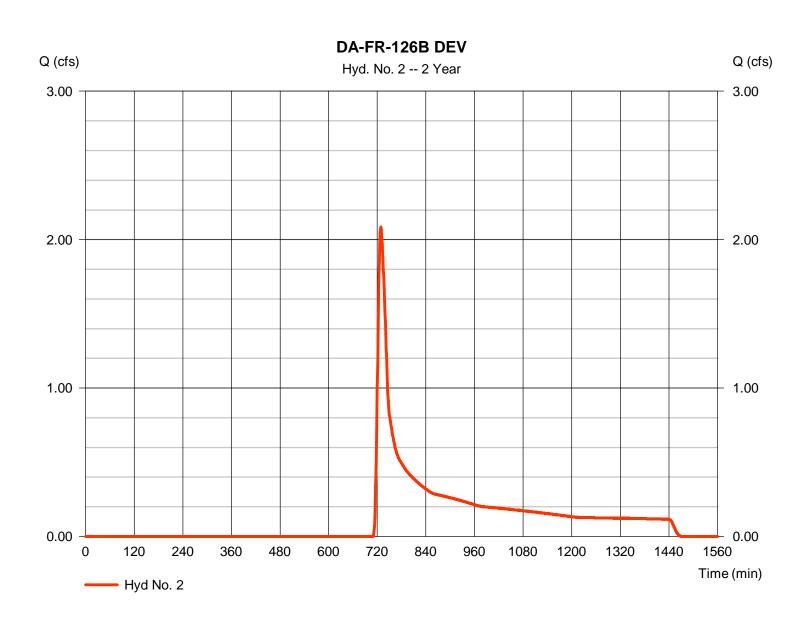
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-126B DEV

= SCS Runoff Hydrograph type Peak discharge = 2.084 cfsStorm frequency Time to peak = 729 min = 2 yrsTime interval = 1 min Hyd. volume = 11.691 cuftCurve number Drainage area = 7.750 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 18.90 min Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(1.419 \times 48) + (0.112 \times 82) + (1.012 \times 58) + (5.210 \times 55)] / 7.750$



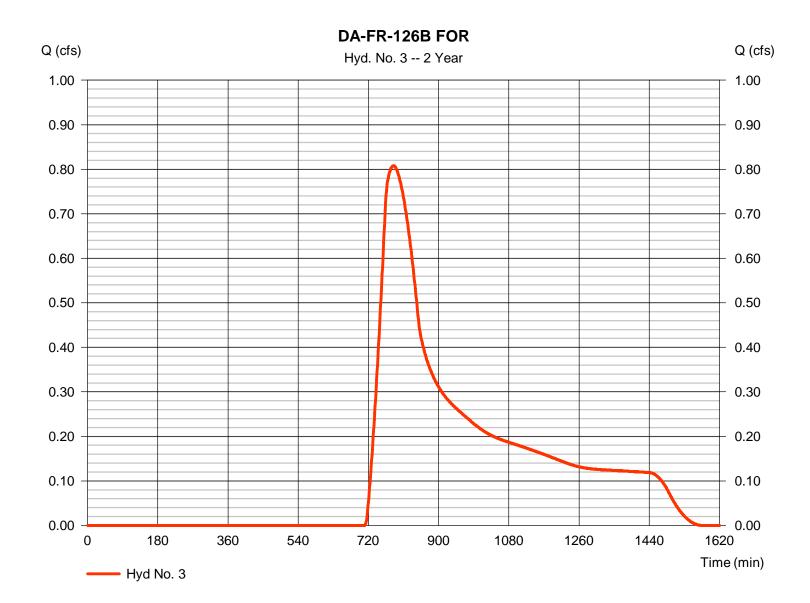
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-126B FOR

Hydrograph type = SCS Runoff Peak discharge = 0.808 cfsStorm frequency Time to peak = 784 min = 2 yrsTime interval = 1 min Hyd. volume = 11,691 cuftDrainage area Curve number = 7.750 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 85.90 min Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

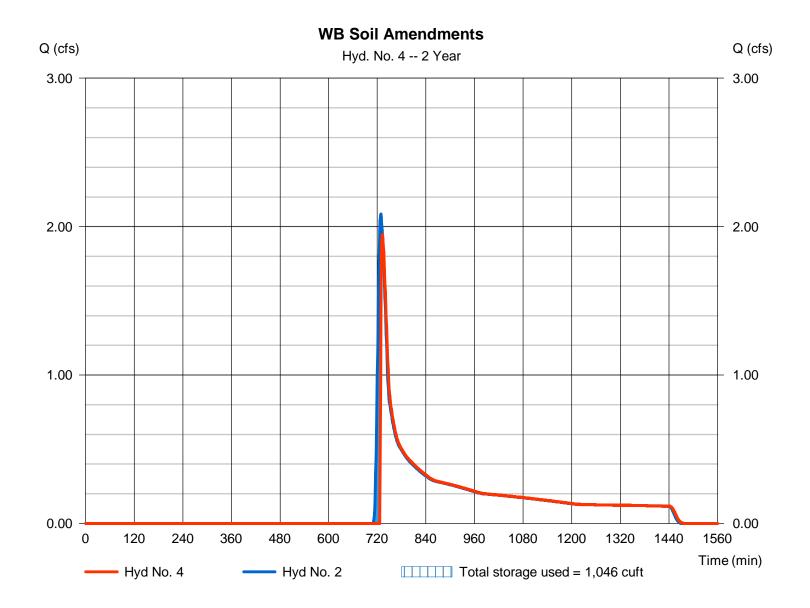
Monday, 08 / 21 / 2017

Hyd. No. 4

WB Soil Amendments

Hydrograph type = Reservoir Peak discharge = 1.946 cfsStorm frequency Time to peak = 733 min = 2 yrsTime interval = 1 min Hyd. volume = 10,898 cuftInflow hyd. No. Max. Elevation = 101.63 ft= 2 - DA-FR-126B DEV Reservoir name = Waterbar Soil Amendments Max. Storage = 1,046 cuft

Storage Indication method used.



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

		_			Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc						
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description		
1	SCS Runoff	10.63	1	726	37,937				DA-FR-126B PRE		
2	SCS Runoff	10.63	1	726	37,937				DA-FR-126B DEV		
3	SCS Runoff	3.717	1	769	37,937				DA-FR-126B FOR		
4	Reservoir	10.58	1	727	37,144	2	101.91	1,580	WB Soil Amendments		
DA-	DA-FR-126A_Hydraflow.gpw					Period: 10 Y	ear/	Monday, 08	3 / 21 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

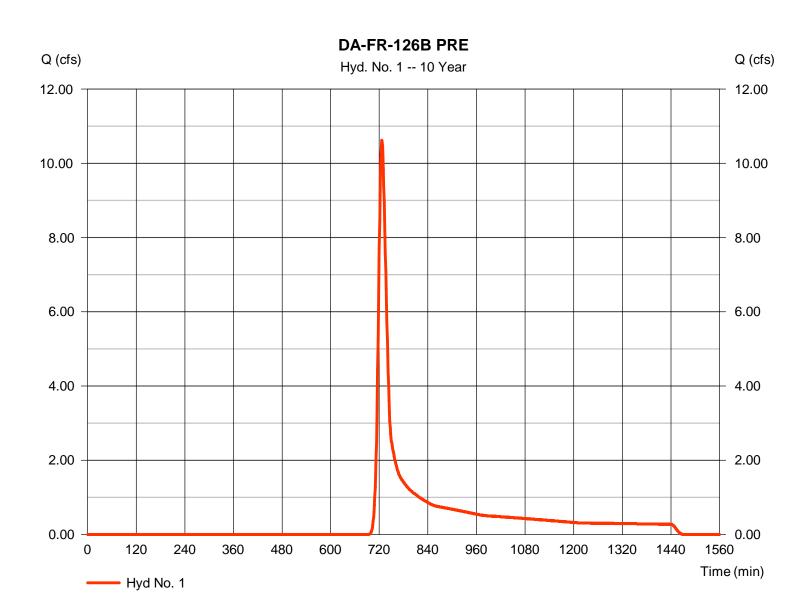
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-126B PRE

= SCS Runoff Hydrograph type Peak discharge = 10.63 cfsStorm frequency Time to peak = 726 min = 10 yrsTime interval = 1 min Hyd. volume = 37.937 cuft Drainage area Curve number = 7.750 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 18.90 min Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.112 x 82) + (7.642 x 55)] / 7.750



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

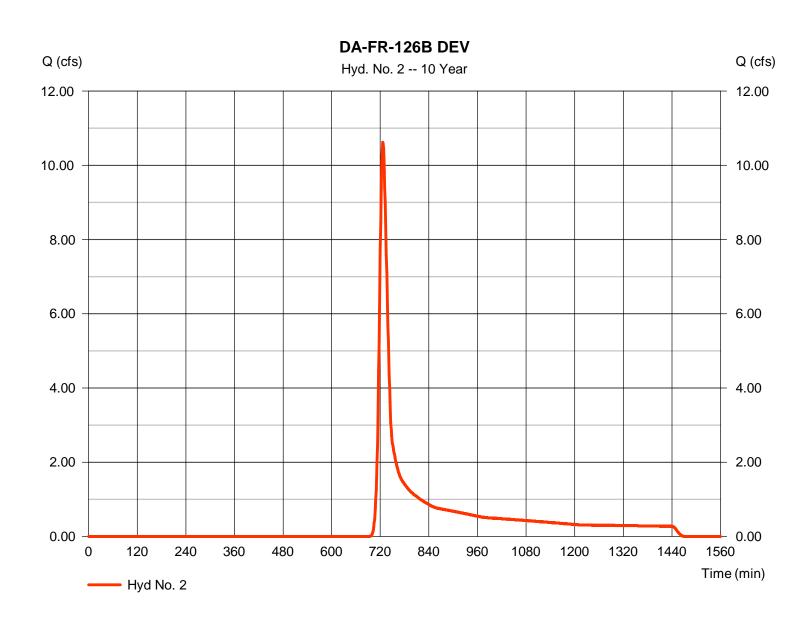
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-126B DEV

= SCS Runoff Hydrograph type Peak discharge = 10.63 cfsStorm frequency Time to peak = 726 min = 10 yrsTime interval = 1 min Hyd. volume = 37.937 cuft Drainage area Curve number = 7.750 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 18.90 min Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(1.419 \times 48) + (0.112 \times 82) + (1.012 \times 58) + (5.210 \times 55)] / 7.750$



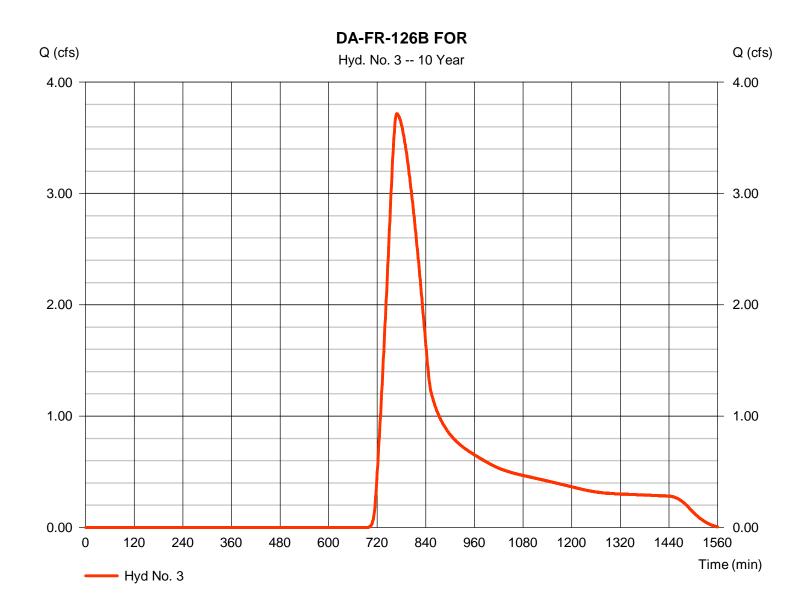
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-126B FOR

= SCS Runoff Hydrograph type Peak discharge = 3.717 cfsStorm frequency Time to peak = 769 min = 10 yrsTime interval = 1 min Hyd. volume = 37,937 cuftDrainage area Curve number = 7.750 ac= 55 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 85.90 min Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

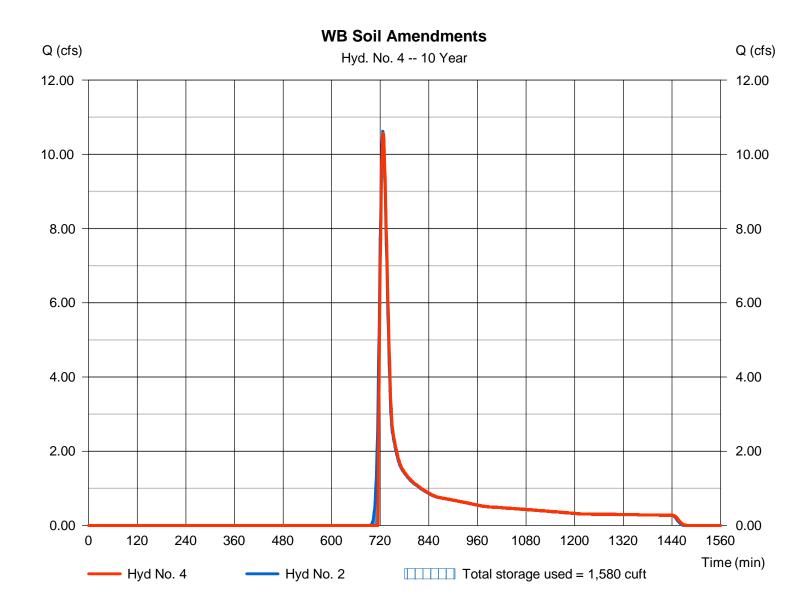
Monday, 08 / 21 / 2017

Hyd. No. 4

WB Soil Amendments

Hydrograph type = Reservoir Peak discharge = 10.58 cfsStorm frequency Time to peak = 727 min = 10 yrsTime interval = 1 minHyd. volume = 37,144 cuftInflow hyd. No. Max. Elevation = 2 - DA-FR-126B DEV = 101.91 ftReservoir name = Waterbar Soil Amendments Max. Storage = 1,580 cuft

Storage Indication method used.



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Return Period	Intensity-Du	ıration-Frequency Ed	quation Coefficients ((FHA)
(Yrs)	В	D	E	(N/A)
1	0.0000	0.0000	0.0000	
2	69.8703	13.1000	0.8658	
3	0.0000	0.0000	0.0000	
5	79.2597	14.6000	0.8369	
10	88.2351	15.5000	0.8279	
25	102.6072	16.5000	0.8217	
50	114.8193	17.2000	0.8199	
100	127.1596	17.8000	0.8186	

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return					Intens	ity Values	(in/hr)					
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc

	Rainfall Precipitation Table (in)							
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00



ENERGY BALANCE METHOD

		ts

	1-Yr Event					
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)				
Pre-Developed Condition	0.683	4224				
Developed Condition	0.527	3786				
Pre-Developed (Forest) Condition	0.683	4224				

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.8

<u>Calculations:</u>	Check #1:	$Q_{developed} \le IF \times [(Q_{pre-developed} \times RV_{pre-developed}) / RV_{developed}]$	0.527	≤ N/A - See Check #3	0.610
	Check #2:	$Q_{developed} \le Q_{pre-developed}$	0.527	≤ N/A - See Check #3	0.683
	Check #3:	$Q_{\text{developed }} \underline{shall \ not}$ be required to be $\leq (Q_{\text{forest}} \times RV_{\text{forest}}) / RV_{\text{developed}}$ >	0.527	shall not be required to be ≤	0.762

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	·
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

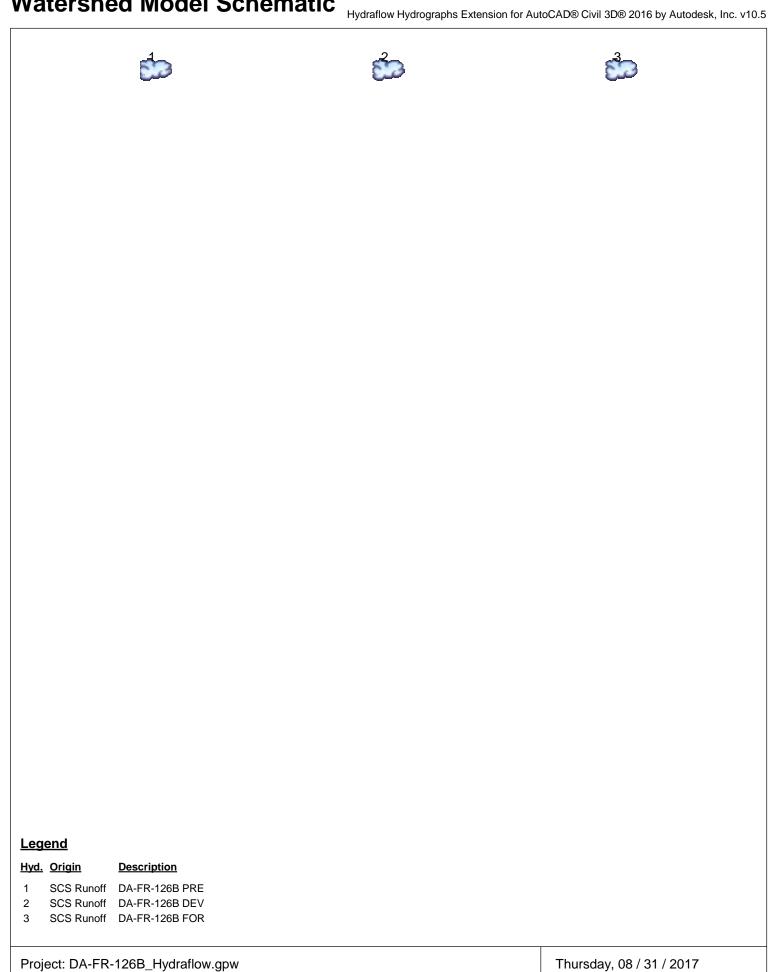
-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Thursday, 08 / 31 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	4
Hydrograph No. 1, SCS Runoff, DA-FR-126B PRE	4
TR-55 Tc Worksheet	5
Hydrograph No. 2, SCS Runoff, DA-FR-126B DEV	6
TR-55 Tc Worksheet	
Hydrograph No. 3, SCS Runoff, DA-FR-126B FOR	
TR-55 Tc Worksheet	9
2 - Year	
Summary Report	10
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-126B PRE	11
Hydrograph No. 2, SCS Runoff, DA-FR-126B DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-126B FOR	
10 - Year	
Summary Report	14
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-126B PRE	15
Hydrograph No. 2, SCS Runoff, DA-FR-126B DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-126B FOR	
IDF Report	. 18



Hydrograph Return Period Recap Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

	Hydrograph	Inflow	Peak Outflow (cfs)								Hydrograph
No.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		0.683	1.357			6.781				DA-FR-126B PRE
2	SCS Runoff		0.527	1.130			6.303				DA-FR-126B DEV
	SCS Runoff		0.683	1.357			6.781				DA-FR-126B FOR

Proj. file: DA-FR-126B_Hydraflow.gpw

Thursday, 08 / 31 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	0.683	1	726	4,224				DA-FR-126B PRE
2	SCS Runoff	0.527	1	727	3,786				DA-FR-126B DEV
3	SCS Runoff	0.683	1	726	4,224				DA-FR-126B FOR
	ED 10cD L	udraffo			Potrice 5	Deriod: 4 V		Thursday	09 / 24 / 2047
DA	-FR-126B_H	yaranow.g	рw		Return P	eriod: 1 Ye	еаг	i nursday, (08 / 31 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

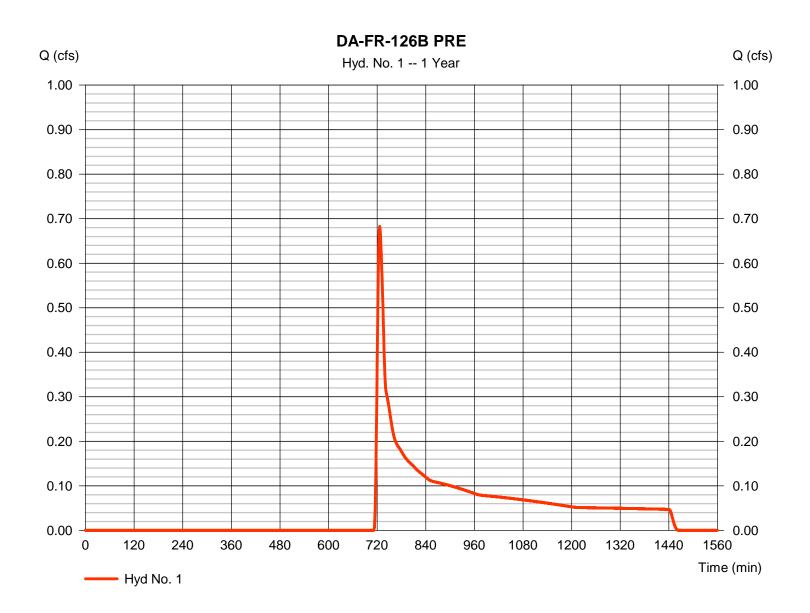
Thursday, 08 / 31 / 2017

Hyd. No. 1

DA-FR-126B PRE

Hydrograph type = SCS Runoff Peak discharge = 0.683 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 minHyd. volume = 4,224 cuftCurve number Drainage area = 4.140 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 14.20 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.258 x 58) + (0.021 x 100) + (3.858 x 55)] / 4.140



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1

DA-FR-126B PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 5.86		0.011 0.0 3.70 0.00		0.011 0.0 3.70 0.00		
Travel Time (min)	= 12.99	+	0.00	+	0.00	=	12.99
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 493.24 = 19.22 = Unpaved =7.07	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.16	+	0.00	+	0.00	=	1.16
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							14.20 min

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

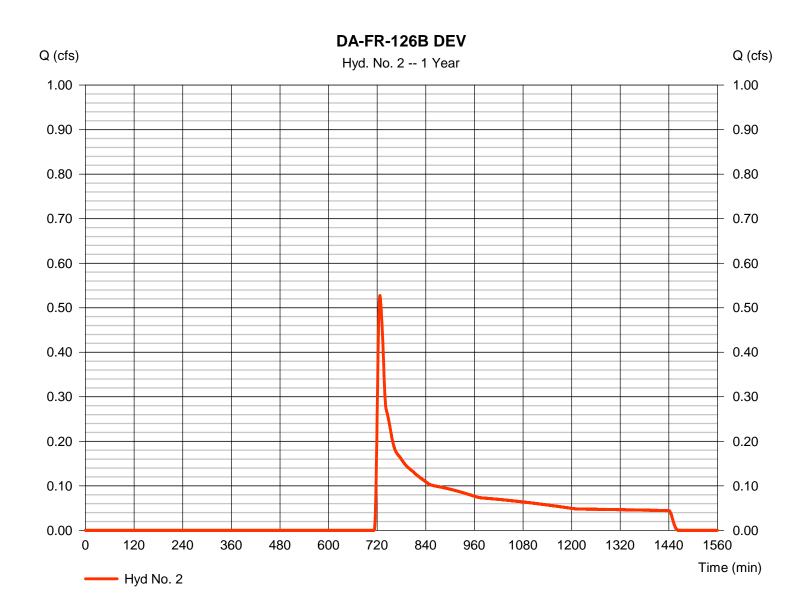
Thursday, 08 / 31 / 2017

Hyd. No. 2

DA-FR-126B DEV

Hydrograph type = SCS Runoff Peak discharge = 0.527 cfsStorm frequency Time to peak = 727 min = 1 yrsTime interval = 1 minHyd. volume = 3.786 cuftDrainage area Curve number = 4.140 ac= 54*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 14.20 \, \text{min}$ Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(1.120 \times 48) + (0.895 \times 58) + (0.021 \times 100) + (2.101 \times 55)] / 4.140$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2

DA-FR-126B DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 5.86 = 12.99	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	12.99
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 493.24 = 19.22 = Unpaved =7.07	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.16	+	0.00	+	0.00	=	1.16
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							14.20 min

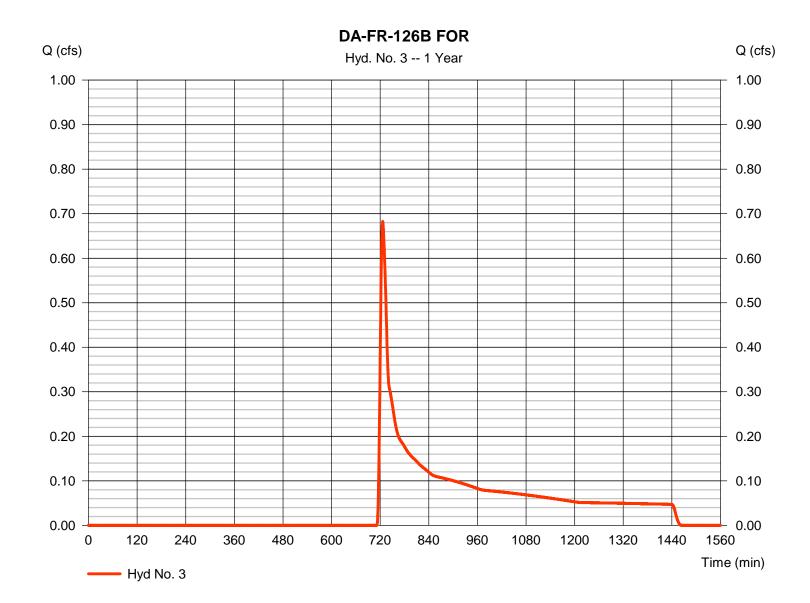
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Thursday, 08 / 31 / 2017

Hyd. No. 3

DA-FR-126B FOR

Hydrograph type = SCS Runoff Peak discharge = 0.683 cfsStorm frequency Time to peak = 726 min = 1 yrsTime interval = 1 min Hyd. volume = 4.224 cuft Drainage area Curve number = 4.140 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 14.20 min = TR55 Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 3

DA-FR-126B FOR

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 5.86 = 12.99	+	0.011 0.0 3.70 0.00	+	0.011 0.0 3.70 0.00	_	12.99
rraver rime (min)	- 12.99	т	0.00	Т	0.00	-	12.33
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 493.24 = 19.22 = Unpave =7.07	d	0.00 0.00 Paved 0.00		0.00 0.00 Paved 0.00		
Travel Time (min)	= 1.16	+	0.00	+	0.00	=	1.16
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015		
Flow length (ft)	({0})0.0		0.0		0.0		
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00
Total Travel Time, Tc							14.20 min

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	1.357	1	725	6,245				DA-FR-126B PRE
2	SCS Runoff	1.130	1	725	5,694				DA-FR-126B DEV
3	SCS Runoff	1.357	1	725	6,245				DA-FR-126B FOR
DA	-FR-126B_H	vdraflow o	NDW.		Raturn	Period: 2 Ye		Thursday	08 / 31 / 2017
"	. ι Ν-120 D_ Π	yuranow.(312 AA		Netuin F	GIIOU. Z 16	ui	i iiui suay, C	00 / 01 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

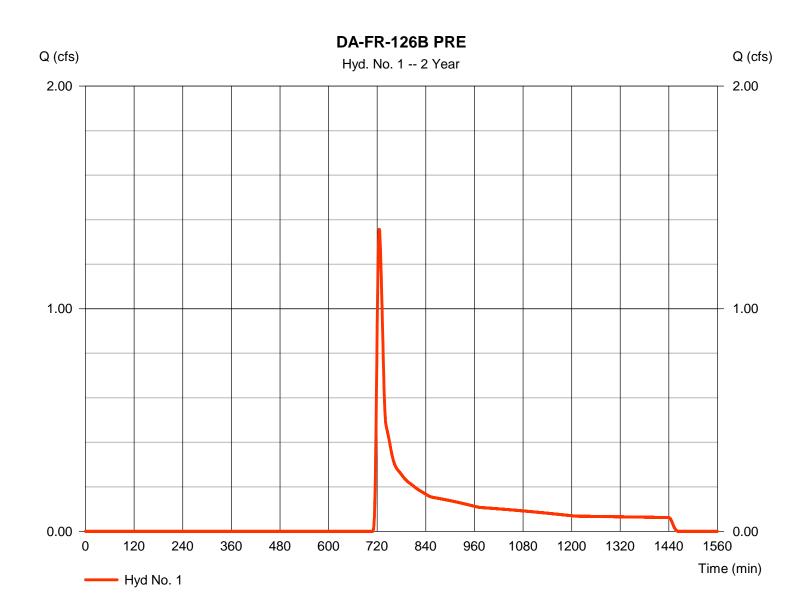
Thursday, 08 / 31 / 2017

Hyd. No. 1

DA-FR-126B PRE

Hydrograph type = SCS Runoff Peak discharge = 1.357 cfsStorm frequency = 2 yrsTime to peak = 725 min Time interval = 1 minHyd. volume = 6,245 cuftDrainage area Curve number = 4.140 ac= 55*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 14.20 min = TR55 Total precip. = 3.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.258 x 58) + (0.021 x 100) + (3.858 x 55)] / 4.140



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

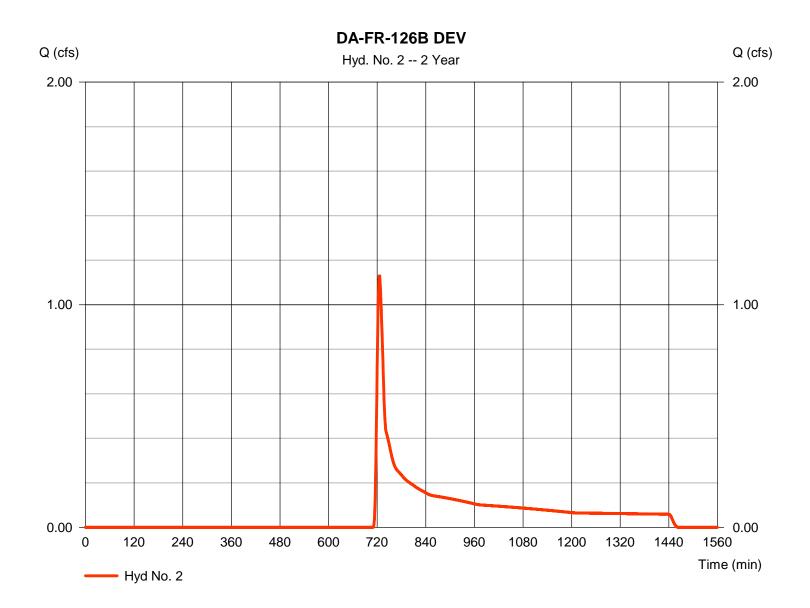
Thursday, 08 / 31 / 2017

Hyd. No. 2

DA-FR-126B DEV

Hydrograph type = SCS Runoff Peak discharge = 1.130 cfsStorm frequency = 2 yrsTime to peak = 725 min Time interval = 1 minHyd. volume = 5.694 cuftCurve number Drainage area = 4.140 ac= 54*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 14.20 \, \text{min}$ Total precip. = 3.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(1.120 x 48) + (0.895 x 58) + (0.021 x 100) + (2.101 x 55)] / 4.140



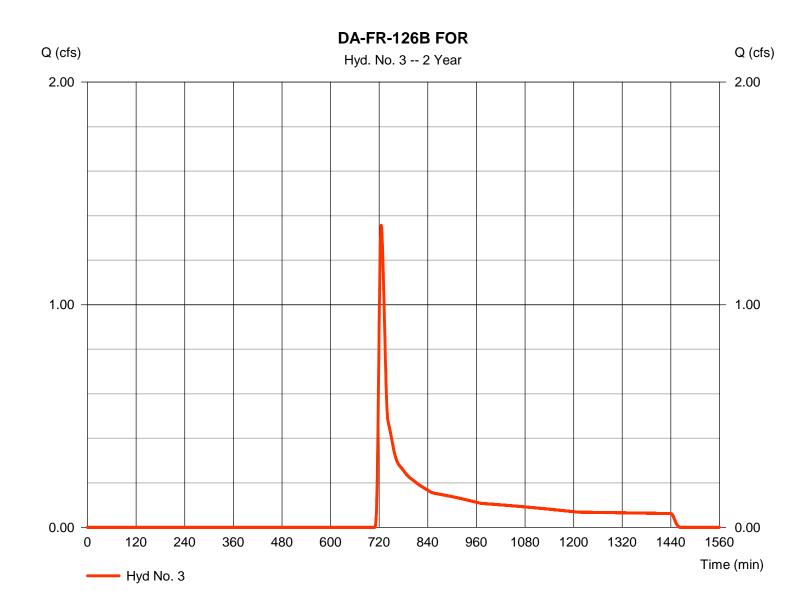
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Thursday, 08 / 31 / 2017

Hyd. No. 3

DA-FR-126B FOR

Hydrograph type = SCS Runoff Peak discharge = 1.357 cfsStorm frequency = 2 yrsTime to peak = 725 min Time interval = 1 min Hyd. volume = 6.245 cuftDrainage area Curve number = 4.140 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 14.20 min = TR55 Total precip. = 3.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	6.781	1	723	20,266				DA-FR-126B PRE
2	SCS Runoff	6.303	1	723	19,177				DA-FR-126B DEV
3	SCS Runoff	6.781	1	723	20,266				DA-FR-126B FOR
DA	DA-FR-126B_Hydraflow.gpw			Return P	eriod: 10 Y	l ′ear	Thursday, 08 / 31 / 2017		

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

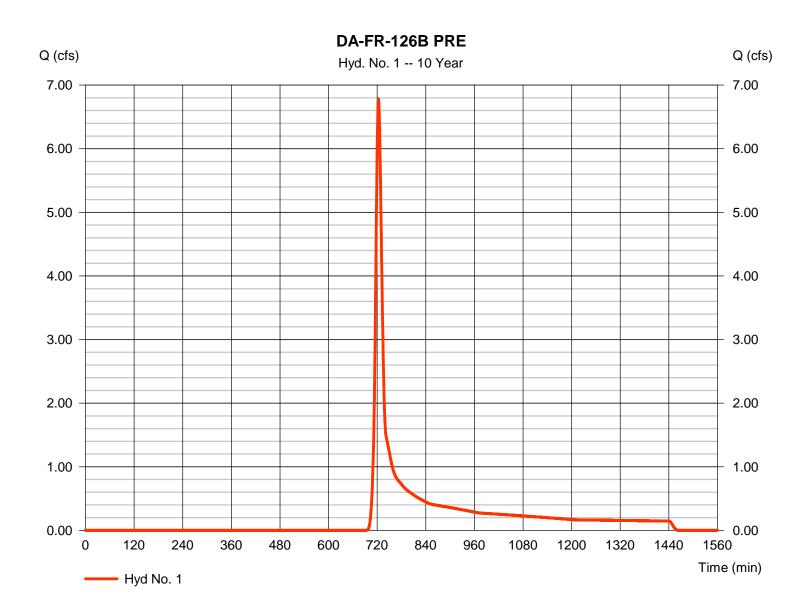
Thursday, 08 / 31 / 2017

Hyd. No. 1

DA-FR-126B PRE

Hydrograph type = SCS Runoff Peak discharge = 6.781 cfsStorm frequency = 10 yrsTime to peak = 723 min Time interval = 1 minHyd. volume = 20.266 cuftDrainage area Curve number = 4.140 ac= 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 14.20 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inShape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.258 x 58) + (0.021 x 100) + (3.858 x 55)] / 4.140



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

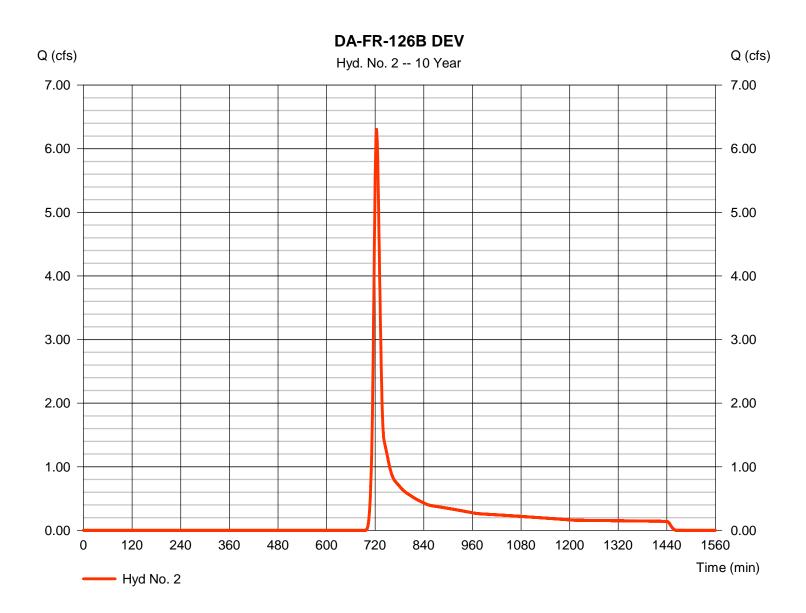
Thursday, 08 / 31 / 2017

Hyd. No. 2

DA-FR-126B DEV

Hydrograph type = SCS Runoff Peak discharge = 6.303 cfsStorm frequency = 10 yrsTime to peak = 723 min Time interval = 1 minHyd. volume = 19.177 cuft Curve number Drainage area = 4.140 ac= 54* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 14.20 \, \text{min}$ Total precip. Distribution = Type II = 5.70 inShape factor Storm duration = 484= 24 hrs

^{*} Composite (Area/CN) = [(1.120 x 48) + (0.895 x 58) + (0.021 x 100) + (2.101 x 55)] / 4.140



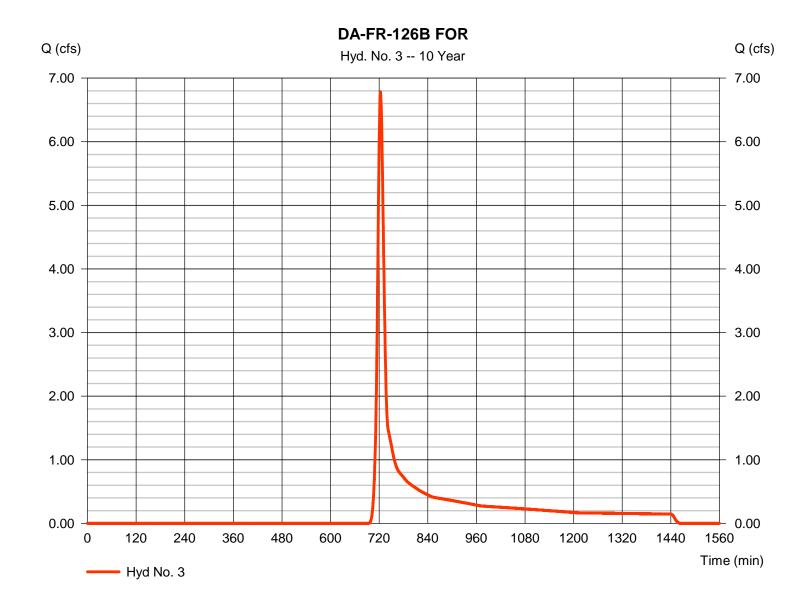
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Thursday, 08 / 31 / 2017

Hyd. No. 3

DA-FR-126B FOR

Hydrograph type = SCS Runoff Peak discharge = 6.781 cfsStorm frequency = 10 yrsTime to peak = 723 min Time interval = 1 minHyd. volume = 20.266 cuftDrainage area Curve number = 4.140 ac= 55Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 14.20 min = TR55 Total precip. = 5.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Thursday, 08 / 31 / 2017

Return Period	Intensity-Du	Intensity-Duration-Frequency Equation Coefficients (FHA)							
(Yrs)	В	D	E	(N/A)					
1	0.0000	0.0000	0.0000						
2	69.8703	13.1000	0.8658						
3	0.0000	0.0000	0.0000						
5	79.2597	14.6000	0.8369						
10	88.2351	15.5000	0.8279						
25	102.6072	16.5000	0.8217						
50	114.8193	17.2000	0.8199						
100	127.1596	17.8000	0.8186						

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return					Intens	sity Values	(in/hr)					
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60

Tc = time in minutes. Values may exceed 60.

 $Iing \\ \label{ling} Franklin \ County \\ \ DA \ Batch \ 4-046-058 \\ \ Precipitation \\ \ Hydraflow \\ \ Franklin \ Co.p \\ \ Precipitation \\ \ Hydraflow \\ \ Franklin \ Co.p \\ \ Precipitation \\ \ Hydraflow \\ \ Franklin \ Co.p \\ \ Precipitation \\ \ Hydraflow \\ \ Franklin \ Co.p \\ \ Precipitation \\ \ Hydraflow \\ \ Franklin \ Co.p \\ \ Precipitation \\ \ Hydraflow \\ \ Franklin \ Co.p \\ \ Precipitation \\ \ Hydraflow \\ \ Franklin \ Co.p \\ \ Precipitation \\ \ Hydraflow \\$

	Rainfall Precipitation Table (in)										
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr			
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00			
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			



ENERGY BALANCE METHOD

In		

	1-Yr Event					
	Peak Flow, Q (cfs)	Runoff Volume, RV (cf)				
Pre-Developed Condition	1.236	6083				
Developed Condition	1.033	5544				
Pre-Developed (Forest) Condition	0.676	4535				

^{*}Peak Flow and Runoff Volume inputs taken from Hydraflow Hydrographs model

Improvement Factor, IF = 0.8

<u>Calculations:</u>	Check #1:	$Q_{\text{developed}} \leq \text{IF x } \left[\left(Q_{\text{pre-developed}} \times \text{RV}_{\text{pre-developed}} \right) / \left(\text{RV}_{\text{developed}} \right) \right] - \cdots >$	1.033	≤ OK	1.085
	Check #2:	$Q_{developed} \le Q_{pre-developed}$	1.033	≤ OK	1.236
	Check #3:	$Q_{\text{developed } \underline{shall \ not}}$ be required to be $\leq (Q_{\text{forest}} \times RV_{\text{forest}}) / RV_{\text{developed}}>$	1.033	<u>shall not</u> be required to be ≤	0.553

STORMWATER QUANTITY REQUIREMENTS ARE SATISFIED

Table 1 – Manning's n Values for Sheet Flow

Land Surface Type	Manning n
Grass:	
Average Grass Cover	0.40
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Light Turf	0.20
Dense Turf	0.17 – 0.80
Dense Grass	0.17 – 0.30
Bermuda Grass	0.30 – 0.48
Dense Shrubbery and Forest Litter	0.40
Natural:	•
Short Grass Prairie	0.10 – 0.20
Poor Grass Cover, Moderately Rough Surface	0.30 – 0.40
Sparse Vegetation	0.05 – 0.13
Oak Grasslands, Open Grasslands	0.60
Dense Cover of Trees and Bushes	0.80
Rangeland:	
Typical	0.13
No Debris Cover	0.09 – 0.34
20% Debris Cover	0.05 – 0.25
Woods:	
Light Underbrush	0.40
Dense Underbrush	0.80
Rural Residential (1 – 10 acre lots, Maintenance or grazing assumed)	0.40

Note:

Manning's n values for sheet flow that are used in Hydraflow Hydrographs are highlighted.

For Paved Road land surface types a Manning's n value of 0.011 was used.

Sources:

-USACE, 1998, HEC-1 Flood Hydrograph Package User's Manual, Hydrologic Engineering Center, Davis, CA

-Soil Conservation Service, 1986, Urban Hydrology for Small Watersheds, Technical Release 55, U.S. Department of Agriculture, Washington, DC

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Watershed Model Schematic	1
Hydrograph Return Period Recap	2
1 - Year	
Summary Report	3
Hydrograph Reports	4
Hydrograph ReportsHydrograph No. 1, SCS Runoff, DA-FR-126C PRE	4
TR-55 Tc Worksheet	5
Hydrograph No. 2, SCS Runoff, DA-FR-126C DEV	6
TR-55 Tc Worksheet	
Hydrograph No. 3, SCS Runoff, DA-FR-126C for	8
TR-55 Tc Worksheet	9
2 - Year	
Summary Report	10
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, DA-FR-126C PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-126C DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-126C for	
10 - Year	
	4.4
Summary Report	
Hydrograph ReportsHydrograph No. 1, SCS Runoff, DA-FR-126C PRE	
Hydrograph No. 2, SCS Runoff, DA-FR-126C DEV	
Hydrograph No. 3, SCS Runoff, DA-FR-126C DEV	
11yalogiapii No. 3, 303 Kulioli, DA-FK-1200 IOI	17
IDF Report	18

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5 <u>Legend</u> Hyd. Origin **Description** SCS Runoff DA-FR-126C PRE 2 SCS Runoff DA-FR-126C DEV SCS Runoff DA-FR-126C for Project: DA-FR-126C_Hydraflow.gpw Monday, 08 / 21 / 2017

Hydrograph Return Period Recap

Hyd. No.	Hydrograph	Inflow	Peak Outflow (cfs)						Hydrograph		
lo.	type (origin)	hyd(s)	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	Description
1	SCS Runoff		1.236	2.103			8.265				DA-FR-126C PRE
2	SCS Runoff		1.033	1.836			7.768				DA-FR-126C DEV
3	SCS Runoff		0.676	1.341			6.784				DA-FR-126C for

Proj. file: DA-FR-126C_Hydraflow.gpw

Monday, 08 / 21 / 2017

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	1.236	1	726	6,083				DA-FR-126C PRE
2	SCS Runoff	1.033	1	727	5,544				DA-FR-126C DEV
DA	-FR-126C_H	ydraflow.g	gpw		Return F	Period: 1 Ye	ear	Monday, 08	3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

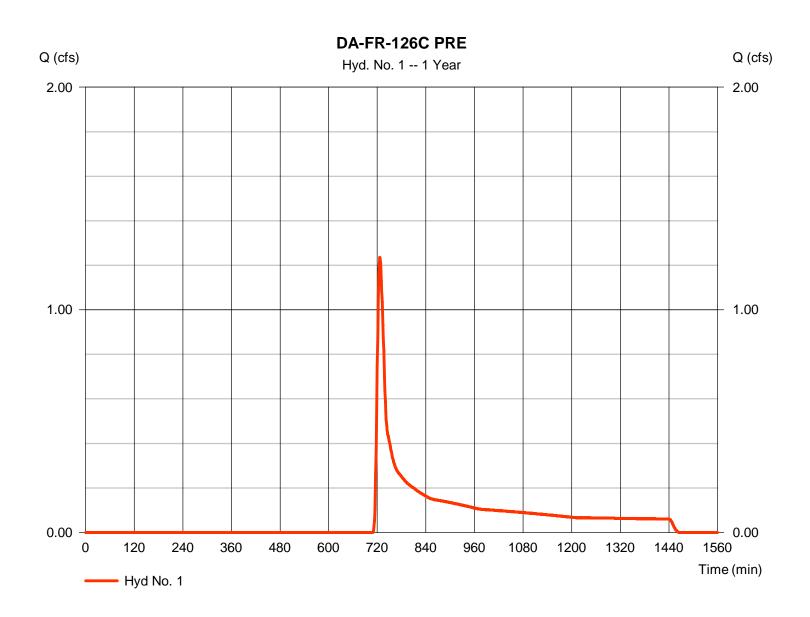
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-126C PRE

Hydrograph type = SCS Runoff Peak discharge = 1.236 cfs= 726 min Storm frequency Time to peak = 1 yrsTime interval = 1 minHyd. volume = 6,083 cuftDrainage area Curve number = 4.390 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.70 min Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.090 \times 82) + (2.590 \times 58) + (0.018 \times 100) + (1.488 \times 55) + (0.204 \times 70)] / 4.390$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 1

DA-FR-126C PRE

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%) Travel Time (min)	= 0.400 = 100.0 = 3.70 = 4.66 = 14.24	+	0.011 0.0 0.00 0.00 0.00	+	0.011 0.0 0.00 0.00 0.00	=	14.24	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 506.09 = 13.53 = Unpaved =5.93	d	0.00 0.00 Unpave 0.00	ed	0.00 0.00 Paved 0.00			
Travel Time (min)	= 1.42	+	0.00	+	0.00	=	1.42	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015			
Flow length (ft)	({0})0.0		0.0		0.0			
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00	
Total Travel Time, Tc								

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

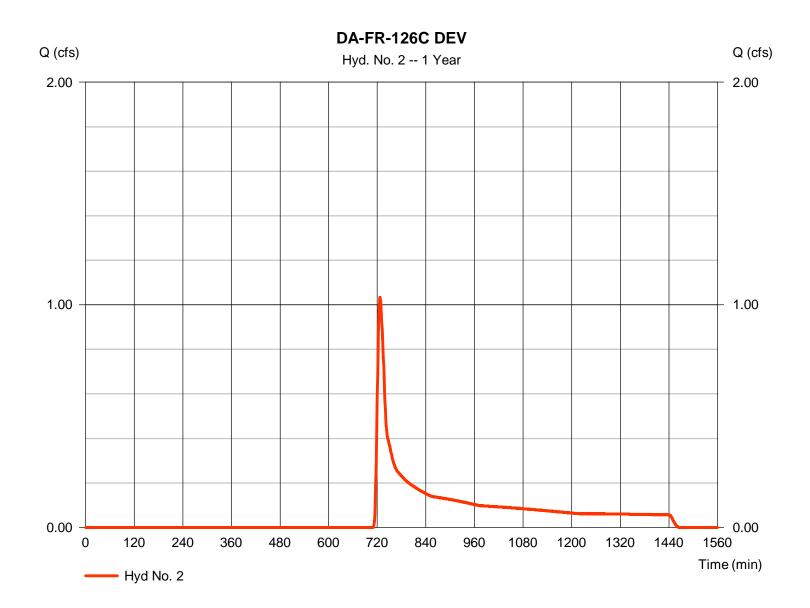
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-126C DEV

Hydrograph type = SCS Runoff Peak discharge = 1.033 cfsStorm frequency Time to peak = 727 min = 1 yrsTime interval = 1 minHyd. volume = 5.544 cuftDrainage area = 57* Curve number = 4.390 acBasin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.70 min Total precip. = 3.30 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.746 \times 48) + (0.204 \times 65) + (0.090 \times 82) + (3.259 \times 58) + (0.018 \times 100) + (0.073 \times 55)] / 4.390$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No. 2

DA-FR-126C DEV

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 4.66		0.800 0.0 0.00 0.00		0.800 0.0 0.00 0.00		
Travel Time (min)	= 14.24	+	0.00	+	0.00	=	14.24
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 506.09 = 13.53 = Unpaved =5.93	I	0.00 0.00 Unpave 0.00	d	0.00 0.00 Unpave 0.00	ed	
Travel Time (min)	= 1.42	+	0.00	+	0.00	=	1.42
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%)	= 0.00 = 0.00 = 0.00		0.00		0.00		
Manning's n-value Velocity (ft/s)	= 0.00 = 0.015 =0.00		0.00 0.015 0.00		0.00 0.015 0.00		
_	= 0.015		0.015		0.015		
Velocity (ft/s)	= 0.015 =0.00	+	0.015	+	0.015	=	0.00

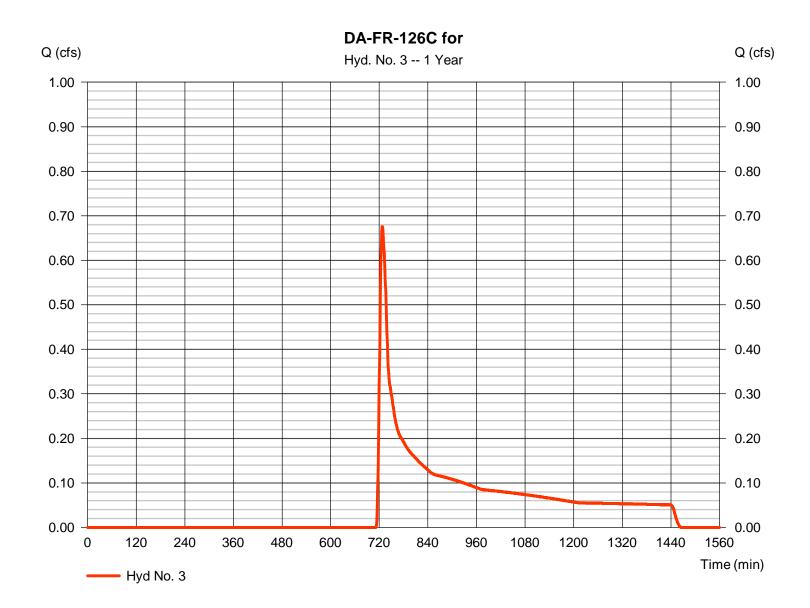
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-126C for

Hydrograph type = SCS Runoff Peak discharge = 0.676 cfsStorm frequency Time to peak = 727 min = 1 yrsTime interval = 1 min Hyd. volume = 4,535 cuftDrainage area Curve number = 4.390 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 15.70 min Total precip. Distribution = 3.30 in= Type II Storm duration = 24 hrs Shape factor = 484



 $\label{thm:condition} \mbox{Hydrographs Extension for AutoCAD} \mbox{\@none} \mbox{\$

Hyd. No. 3

DA-FR-126C for

<u>Description</u>	<u>A</u>		<u>B</u>		<u>C</u>		<u>Totals</u>	
Sheet Flow Manning's n-value Flow length (ft) Two-year 24-hr precip. (in) Land slope (%)	= 0.400 = 100.0 = 3.70 = 4.66		0.011 0.0 0.00 0.00		0.011 0.0 0.00 0.00			
Travel Time (min)	= 14.24	+	0.00	+	0.00	=	14.24	
Shallow Concentrated Flow Flow length (ft) Watercourse slope (%) Surface description Average velocity (ft/s)	= 506.09 = 13.53 = Unpave =5.93	d	0.00 0.00 Unpave 0.00	ed	0.00 0.00 Paved 0.00			
Travel Time (min)	= 1.42	+	0.00	+	0.00	=	1.42	
Channel Flow X sectional flow area (sqft) Wetted perimeter (ft) Channel slope (%) Manning's n-value Velocity (ft/s)	= 0.00 = 0.00 = 0.00 = 0.015 =0.00		0.00 0.00 0.00 0.015 0.00		0.00 0.00 0.00 0.015			
Flow length (ft)	({0})0.0		0.0		0.0			
Travel Time (min)	= 0.00	+	0.00	+	0.00	=	0.00	
Total Travel Time, Tc								

Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

⊣yd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	2.103	1	726	8,617				DA-FR-126C PRE
2	SCS Runoff	1.836	1	726	7,957				DA-FR-126C DEV
2 3	SCS Runoff SCS Runoff	1.836	1 1	726 726	7,957 6,705				DA-FR-126C DEV DA-FR-126C for
DA	-FR-126C_H	ydraflow.	gpw		Return F	Period: 2 Ye	ear	Monday, 08	3 / 21 / 2017

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

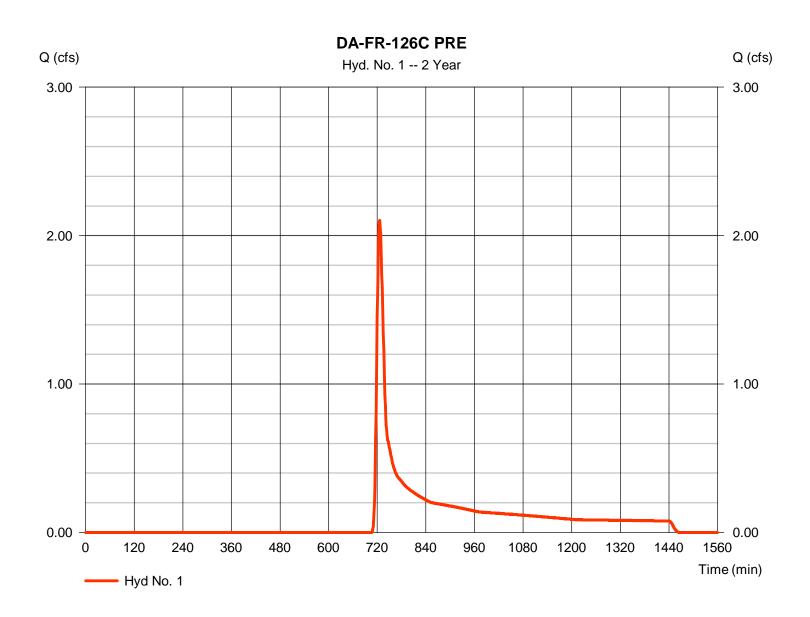
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-126C PRE

Hydrograph type = SCS Runoff Peak discharge = 2.103 cfs= 726 min Storm frequency Time to peak = 2 yrsTime interval = 1 min Hyd. volume = 8.617 cuftCurve number Drainage area = 4.390 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.70 min Total precip. = 3.70 inDistribution = Type II Storm duration Shape factor = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.090 \times 82) + (2.590 \times 58) + (0.018 \times 100) + (1.488 \times 55) + (0.204 \times 70)] / 4.390$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

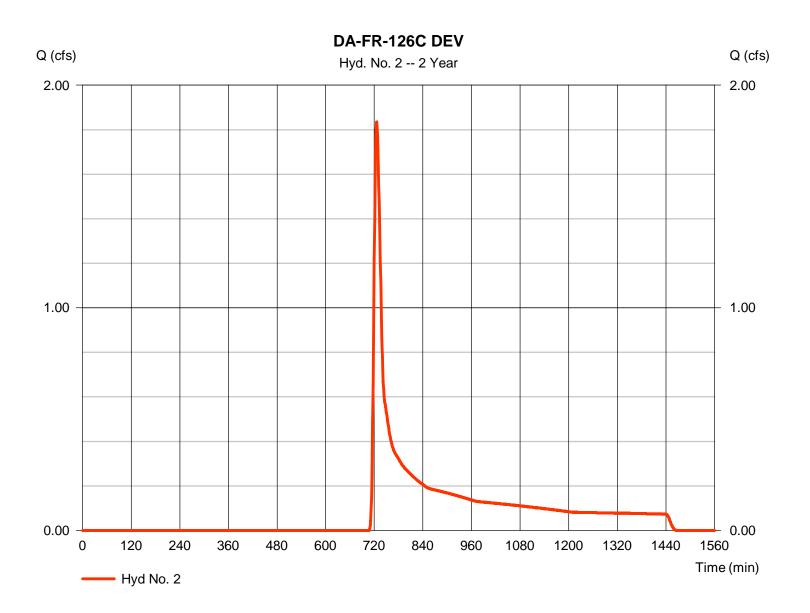
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-126C DEV

= SCS Runoff Hydrograph type Peak discharge = 1.836 cfsStorm frequency Time to peak = 726 min = 2 yrsTime interval = 1 minHyd. volume = 7,957 cuftDrainage area = 57* Curve number = 4.390 acBasin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.70 min Total precip. = 3.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.746 \times 48) + (0.204 \times 65) + (0.090 \times 82) + (3.259 \times 58) + (0.018 \times 100) + (0.073 \times 55)] / 4.390$



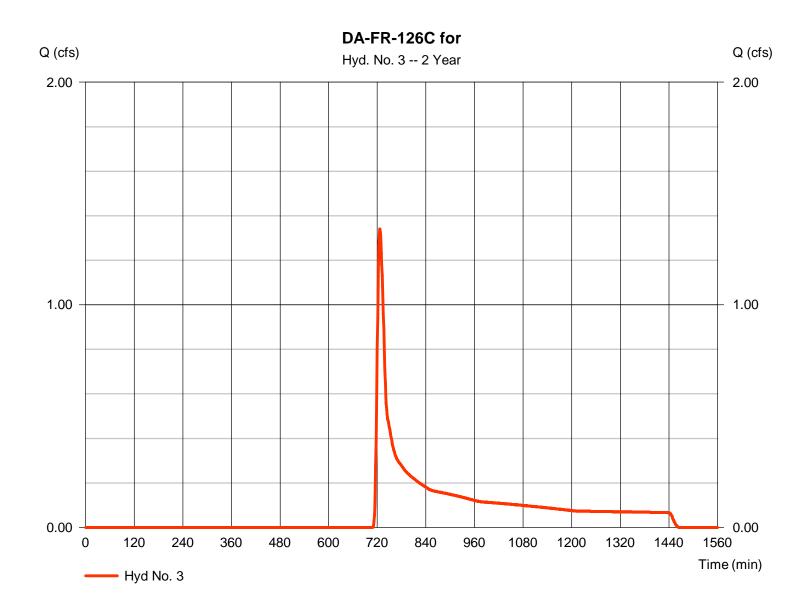
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-126C for

Hydrograph type = SCS Runoff = 1.341 cfsPeak discharge Storm frequency Time to peak = 726 min = 2 yrsTime interval = 1 min Hyd. volume = 6,705 cuftDrainage area = 4.390 acCurve number = 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 = 15.70 min Total precip. Distribution = 3.70 in= Type II Storm duration = 484 = 24 hrs Shape factor



Hydrograph Summary Report Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description		
1	SCS Runoff	8.265	1	724	25,378				DA-FR-126C PRE		
2	SCS Runoff	7.768	1	724	24,153				DA-FR-126C DEV		
DA-FR-126C_Hydraflow.gpw					Return F	Period: 10 Y	l ′ear	Monday, 08 / 21 / 2017			

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

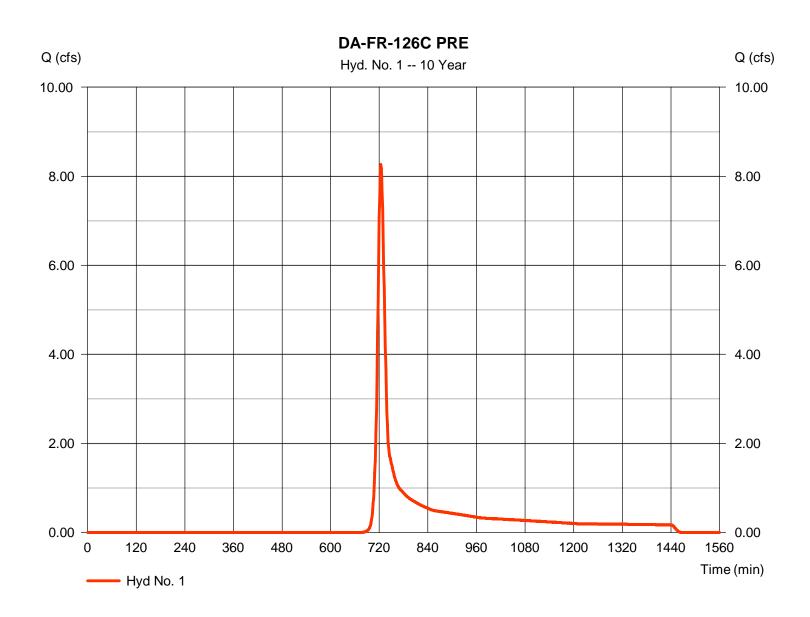
Monday, 08 / 21 / 2017

Hyd. No. 1

DA-FR-126C PRE

= SCS Runoff Hydrograph type Peak discharge = 8.265 cfs= 724 min Storm frequency Time to peak = 10 yrsTime interval = 1 min Hyd. volume = 25.378 cuft Drainage area Curve number = 4.390 ac= 58*Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.70 min Total precip. = 5.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.090 \times 82) + (2.590 \times 58) + (0.018 \times 100) + (1.488 \times 55) + (0.204 \times 70)] / 4.390$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

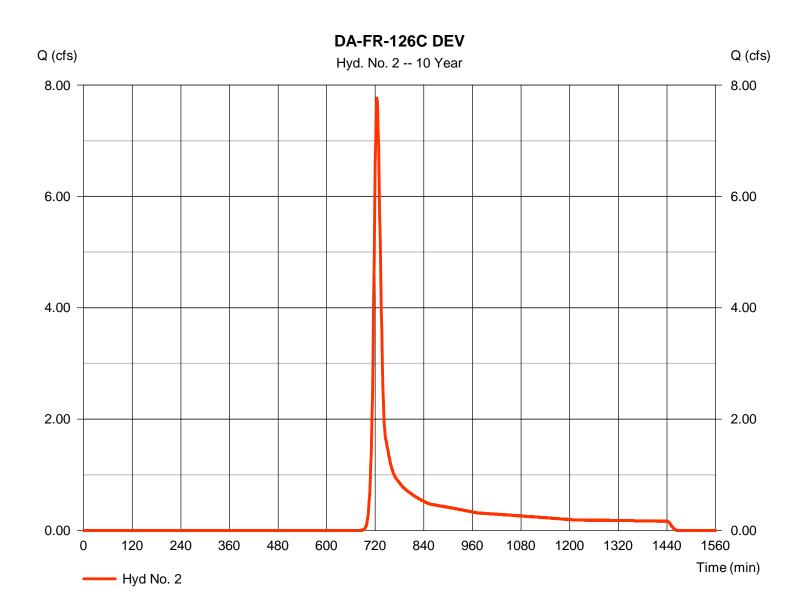
Monday, 08 / 21 / 2017

Hyd. No. 2

DA-FR-126C DEV

= SCS Runoff Hydrograph type Peak discharge = 7.768 cfsStorm frequency Time to peak = 724 min = 10 yrsTime interval = 1 min Hyd. volume = 24,153 cuftCurve number Drainage area = 4.390 ac= 57* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.70 min Total precip. = 5.70 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.746 \times 48) + (0.204 \times 65) + (0.090 \times 82) + (3.259 \times 58) + (0.018 \times 100) + (0.073 \times 55)] / 4.390$



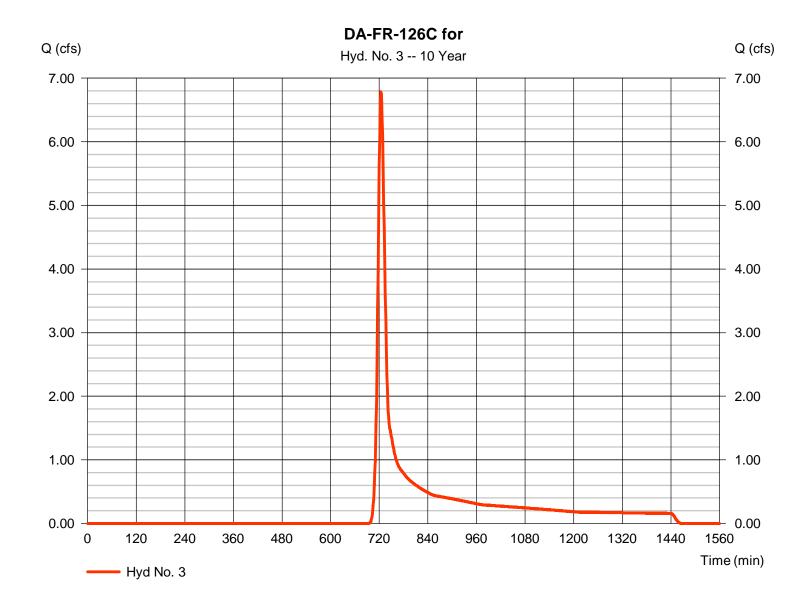
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Hyd. No. 3

DA-FR-126C for

= SCS Runoff Hydrograph type Peak discharge = 6.784 cfsStorm frequency Time to peak = 724 min = 10 yrsTime interval = 1 min Hyd. volume = 21,758 cuftDrainage area Curve number = 4.390 ac= 55 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) = 15.70 min Total precip. = 5.70 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2016 by Autodesk, Inc. v10.5

Monday, 08 / 21 / 2017

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)									
(Yrs)	В	D	E	(N/A)						
1	0.0000	0.0000	0.0000							
2	69.8703	13.1000	0.8658							
3	0.0000	0.0000	0.0000							
5	79.2597	14.6000	0.8369							
10	88.2351	15.5000	0.8279							
25	102.6072	16.5000	0.8217							
50	114.8193	17.2000	0.8199							
100	127.1596	17.8000	0.8186							

File name: SampleFHA.idf

Intensity = $B/(Tc + D)^E$

Return		Intensity Values (in/hr)											
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60	
1	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
2	5.69	4.61	3.89	3.38	2.99	2.69	2.44	2.24	2.07	1.93	1.81	1.70	
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5	6.57	5.43	4.65	4.08	3.65	3.30	3.02	2.79	2.59	2.42	2.27	2.15	
10	7.24	6.04	5.21	4.59	4.12	3.74	3.43	3.17	2.95	2.77	2.60	2.46	
25	8.25	6.95	6.03	5.34	4.80	4.38	4.02	3.73	3.48	3.26	3.07	2.91	
50	9.04	7.65	6.66	5.92	5.34	4.87	4.49	4.16	3.88	3.65	3.44	3.25	
100	9.83	8.36	7.30	6.50	5.87	5.36	4.94	4.59	4.29	4.03	3.80	3.60	

Tc = time in minutes. Values may exceed 60.

ng\Franklin County\Downloaded Files\Franklin County DA Batch 4 - 046 - 058\Precipitation\Hydraflow\FranklinCo.pc|

	Rainfall Precipitation Table (in)								
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	
SCS 24-hour	3.30	3.70	0.00	0.00	5.70	0.00	0.00	0.00	
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Huff-2nd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Huff-3rd	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Huff-4th	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Huff-Indy	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
Custom	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	